Chelan County Public Works Department

316 Washington Street, Suite 402
Wenatchee, Washington

January 16, 2026
Addendum No. 4

Attention: All bidders and planholders

RE: CRP 650, Totem Pole Road Phase I Road Improvement Project

You are hereby notified of the following changes to the Contract Documents, Special Provisions, Standard Specifications, Plans and other documents comprising the contract documents for the above referenced project.

General Note

1. **The bid opening date for Totem Pole Road Phase I has been extended one additional week to January 27, 2026.**

Contract

1. **Superseding Addendum 2, the first sentence of the Call For Bids shall be removed and replaced with the following:**

   Sealed bids will be received by the Board of Chelan County Commissioners at their office at 400 Douglas Street, Wenatchee, Washington 98801 until **9:30:00 A.M. on January 27, 2026.**

Contract Special Provisions

1. **On Pages SP-101 through SP-104, deleted the entire section titled "(January 2, 2018) Concrete Block Faced Structural Earth Wall Materials" and replace with the following:**

(September 2, 2025)
Concrete Block Faced Structural Earth Wall Materials

General Materials

Concrete Block

Acceptability of the blocks will be determined based on the following:

1. Visual inspection.
2. Compressive strength tests, conforming to Section 6-13.3(4).
3. Water absorption tests, conforming to Section 6-13.3(4).
4. Manufacturer's Certificate of Compliance in accordance with Section 1-06.3.
5. Freeze-thaw tests conducted on the lot of blocks produced for use in this project, as specified in Section 6-13.3(4).

6. Copies of results from tests conducted on the lot of blocks produced for this project by the concrete block fabricator in accordance with the quality control program required by the structural earth wall manufacturer.

The blocks shall be considered acceptable regardless of curing age when compressive test results indicate that the compressive strength conforms to the 28-day requirements, and when all other acceptability requirements specified above are met.

Testing and inspection of dry cast concrete blocks shall conform to ASTM C 140, and shall include block fabrication plant approval by WSDOT prior to the start of block production for this project.

Mortar

Mortar shall conform to ASTM C 270, Type S, with an integral water repellent admixture as accepted by the Engineer. The amount of admixture shall be as recommended by the admixture manufacturer. To ensure uniform color, texture, and quality, all mortar mix components shall be obtained from one manufacturer for each component, and from one source and producer for each aggregate.

Geosynthetic Soil Reinforcement

Geogrid reinforcement shall conform to Section 9-33.1, and shall be a product listed in Appendix D of the current WSDOT Qualified Products List (QPL). The values of T_{al} and T_{ult} as listed in the QPL for the products used shall meet or exceed the values required for the wall manufacturer's reinforcement design as specified in the structural earth wall design calculation and working drawing submittal.

The minimum ultimate tensile strength of the geogrid shall be a minimum average roll value (the average test results for any sampled roll in a lot shall meet or exceed the values shown in Appendix D of the current WSDOT QPL). The strength shall be determined in accordance with ASTM D 6637, for multi-rib specimens.

The ultraviolet (UV) radiation stability, in accordance with ASTM D 4355, shall be a minimum of 70 percent strength retained after 500 hours in the weatherometer.

The longitudinal (i.e., in the direction of loading) and transverse (i.e., parallel to the wall or slope face) ribs that make up the geogrid shall be perpendicular to one another. The maximum deviation of the cross-rib from being perpendicular to the longitudinal rib (skew) shall be no more than 1 inch in 5 feet of geogrid width. The maximum deviation of the cross-rib at any point from a line perpendicular to the longitudinal ribs located at the cross-rib (bow) shall be 0.5 inches.

The gap between the connector and the bearing surface of the connector tab cross-rib shall not exceed 0.5 inches. A maximum of 10 percent of connector tabs may have a gap between 0.3 inches and 0.5 inches. Gaps in the remaining connector tabs shall not exceed 0.3 inches.

The Engineer will take random samples of the geogrid materials at the job site. Acceptance of the geogrid materials will be based on testing of samples from each lot. A "lot" shall be defined as all geogrid rolls sent to the project site produced by the same manufacturer during a continuous period of production at the same manufacturing plant having the same product name. The Contracting Agency will

require 14 calendar days maximum for testing the samples after their arrival at the WSDOT Materials Laboratory in Tumwater, WA.

The geogrid samples will be tested for conformance to the specified material properties. If the test results indicate that the geogrid lot does not meet the specified properties, the roll or rolls which were sampled will be rejected. Two additional rolls for each roll tested which failed from the lot previously tested will then be selected at random by the Engineer for sampling and retesting. If the retesting shows that any of the additional rolls tested do not meet the specified properties, the entire lot will be rejected. If the test results from all the rolls retested meet the specified properties, the entire lot minus the roll(s) which failed will be accepted.

All geogrid materials which have defects, deterioration, or damage, as determined by the Engineer, will be rejected. All rejected geogrid materials shall be replaced at no expense to the Contracting Agency.

Except as otherwise noted, geogrid identification, storage and handling shall conform to the requirements specified in Section 3-09.2. The geogrid materials shall not be exposed to temperatures less than –20F and greater than 122F.

Drainage Geosynthetic Fabric
Drainage geosynthetic fabric shall be a non-woven geosynthetic conforming to the requirements in Section 9-33.1, for Construction Geotextile for Underground Drainage, Moderate Survivability, Class B.

Proprietary Materials

Allan Block Wall
Wall backfill material placed in the open cells of the precast concrete blocks and placed in the one to three foot zone immediately behind the precast concrete blocks shall be crushed granular material conforming to Section 9-03.9(3).

GEOWALL Structural Earth Retaining Wall System
Connection pins shall be fiberglass conforming to the requirements of Basalite Concrete Products, LLC.

KeyGrid Wall
KeyStone connection pins shall be fiberglass conforming to the requirements of Keystone Retaining Wall Systems, Inc.

Landmark Retaining Wall
Lock bars shall be made of a rigid polyvinyl chloride polymer conforming to the following requirements:

Property	Value	Specification
Specific Gravity	1.4 minimum	ASTM D 792
Tensile Strength at yield	2,700 psi minimum	ASTM D 638

Lock bars shall remain sealed in their shipping containers until placement into the wall. Lock bars exposed to direct sunlight for a period exceeding two months shall not be used for construction of the wall.

Mesa Wall
Block connectors for block courses with geogrid reinforcement shall be glass fiber reinforced high-density polypropylene conforming to the following minimum material specifications:

Property	Specification	Value
Polypropylene	ASTM D 4101	
	Group 1 Class 1 Grade 2	73 ± 2 percent
Fiberglass Content	ASTM D 2584	25 ± 3 percent
Carbon Black	ASTM D 4218	2 percent minimum
Specific Gravity	ASTM D 792	1.08 ± 0.04
Tensile Strength	ASTM D 638	
at yield		8,700 ± 1,450 psi
Melt Flow Rate	ASTM D 1238	0.37 ± 0.16 ounces/10 min.

Block connectors for block courses without geogrid reinforcement shall be glass fiber reinforced high-density polyethylene (HDPE) conforming to the following minimum material specifications:

Property	Specification	Value
HDPE	ASTM D 1248	
	Type III Class A Grade 5	68 ± 3 percent
Fiberglass Content	ASTM D 2584	30 ± 3 percent
Carbon Black	ASTM D 4218	2 percent minimum
Specific Gravity	ASTM D 792	1.16 ± 0.06
Tensile Strength	ASTM D 638	
at yield		8,700 ± 725 psi
Melt Flow Rate	ASTM D 1238	0.11 ± 0.07 ounces/10 min.

2. **In Section 6-13 of the Special Provisions, insert the following General Special Provisions for Lock and Load Retaining Wall System as another option for the Contractor to use for Wall #1.**

Materials
Section 6-13.2 is supplemented with the following:

(August 3, 2015)
Lock + Load Retaining Wall System
Stainless steel wire and wire rods shall conform to ASTM A 580.

Stainless steel bars, plates and shapes shall conform to ASTM A 276 Type 304.

The maximum particle size of the backfill material within 1'-6" of the back face of the precast concrete facing panel shall not exceed 3/4 inches.

Construction
Section 6-13.3 is supplemented with the following:

(August 3, 2015)
Lock + Load Retaining Wall System
Lock + Load is a registered trademark of Lock + Load Retaining Walls, Ltd.

Lock + Load Retaining Walls, Ltd.
1681 Chestnut Street Suite 400
Vancouver, BC V6J 4M6 Canada
(604) 732-9990
FAX: (604) 676-2705
www.lock-load.com

Precast Concrete Facing Panel and Concrete Block Fabrication
Section 6-13.3(4) is supplemented with the following:

(August 3, 2015)
Lock + Load Retaining Wall System
Concrete for precast concrete panels and counterfort members shall conform to ASTM C 1116 Type III, with cement and aggregate gradation as recommended by Lock + Load Retaining Walls, Ltd, slump and air content as specified in this Section, and a minimum compressive strength at 28 days of 5,500 psi. The fiber reinforcement shall be mixed in the concrete at a minimum reinforcement ratio of 3.0 pounds per cubic yard and as specified by Lock + Load Retaining Walls, Ltd.

Full size precast concrete facing panels for Lock + Load retaining walls shall be 2'-8" wide and 1'-4" tall.

Precast concrete counterfort members shall be fabricated, handled, stored, and shipped in accordance with the requirements specified in this Section for precast concrete facing panels.

Backfill
Section 6-13.3(7) is supplemented with the following:
(August 3, 2015)
Specific Backfill Requirements for Precast Concrete Panel Faced Structural Earth Walls

Lock + Load Retaining Wall System
The Contractor shall begin placement and compaction of backfill above the tail of the counterfort member first, then towards the back face of the precast concrete facing panel, followed by placement and compaction of the remainder of the backfill layer. The zone for compaction by plate compactor equipment only, with no soil density testing requirement, shall be within 1'-4" of the back face of the precast concrete facing panel.

Contract Plans

1. **Remove the following plan sheets and replace in their entirety with attached Addendum No. 3 Plan Sheets:**

   WP1 (Supersedes note change from Addendum 1), WD1, MD1

Questions and Answers

1. Question: Multiple Contractors and Suppliers asked for clarification regarding retaining wall type, retaining wall cap, and mounting the pedestrian railing to Wall #1. Additionally, Contractors requested an alignment for Wall #1.

Answer: See included changes to Special Provisions and attached, revised, Plan Sheets for clarifications.

All bidders will be required to furnish the Board of Chelan County Commissioners with evidence of the receipt of this addendum by noting on the proposal sheet.

The new bid date for this project is January 27, 2026.

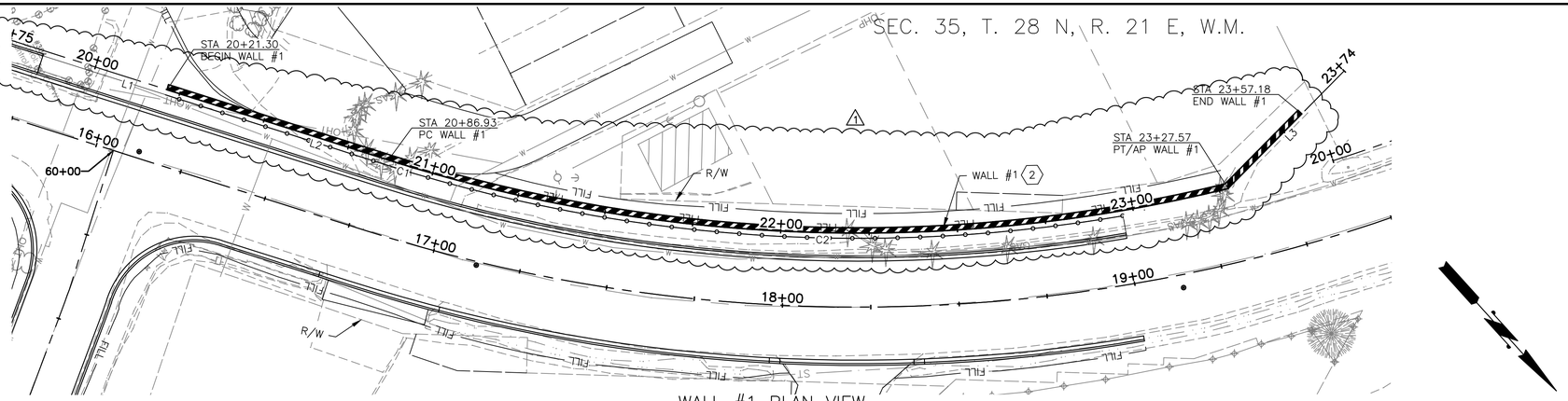
Addendum Number 4 is hereby incorporated in and made part of the contract documents, and its terms and conditions are fully binding on the plan holder and contractor when awarded and when formally executed.

Chelan County Public Works

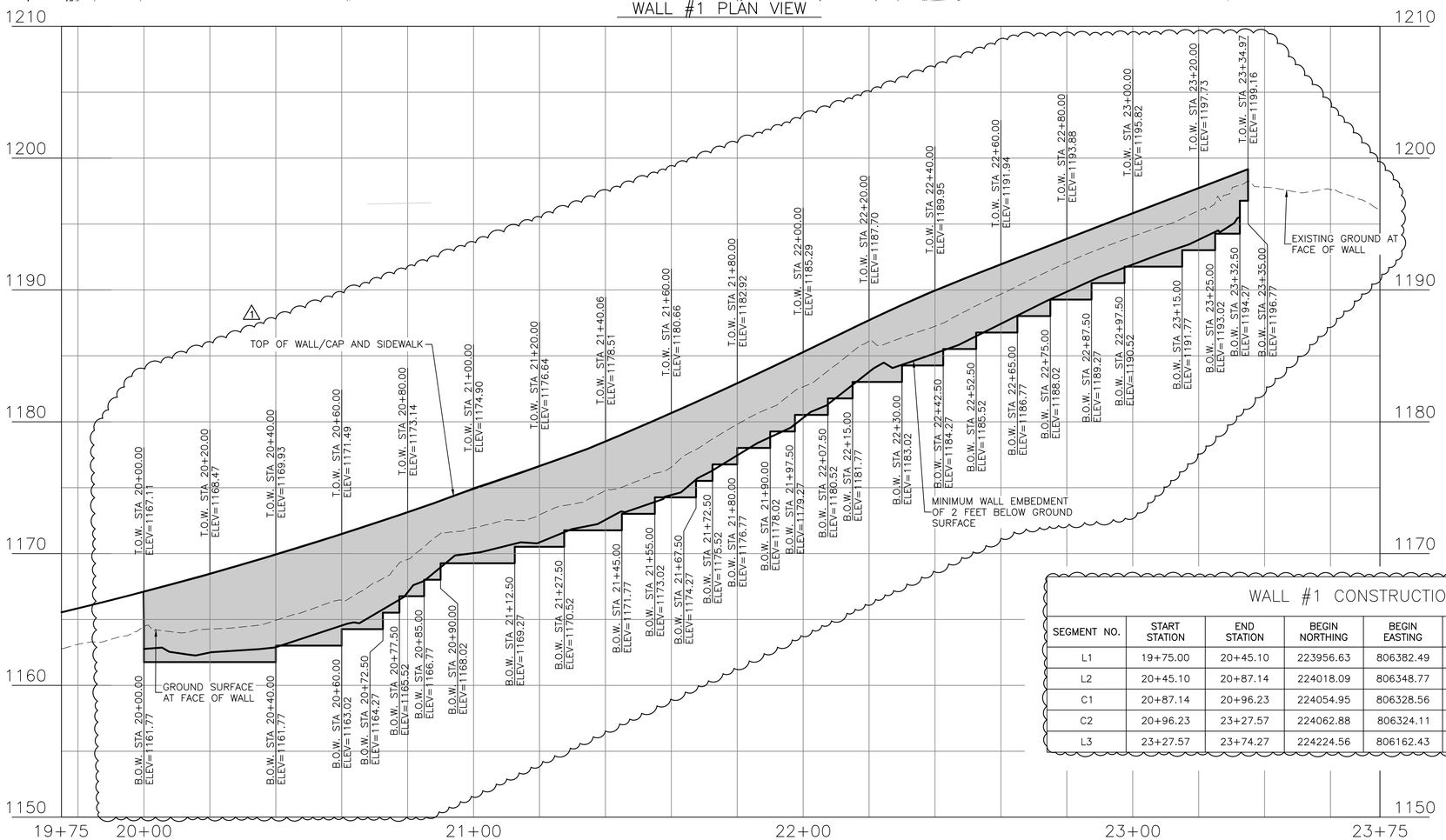
Josh Patrick, P.E.
Assistant County Director

Attachment: Plan Sheets WP1, WD1, MD1

FEDERAL AID NO. STPUS-7006(007)



WALL #1 PLAN VIEW



WALL #1 PROFILE

TOTEM POLE RD STA 16+14 LT TO STA 19+60 LT

CONSTRUCTION NOTES:

1. CONSTRUCT MSE WALL PER DETAILS ON DWG. NO. WD1.

LEGEND

- RETAINING WALL
- PEDESTRIAN HANDRAIL
- WALL CONCRETE FACING ELEMENTS OR BLOCK UNITS WITH CAP

GENERAL NOTES:

1. CONTRACTOR SHALL COORDINATE WITH WALL MANUFACTURERS TO DESIGN RETAINING WALL SYSTEMS ACCORDING TO WALL MANUFACTURER SPECIFICATIONS AND REQUIREMENTS AND DETAILS.
2. ALL STATIONS AND OFFSETS ARE MEASURED TO THE FACE OF THE WALL.
3. THE CONTRACTOR SHALL FIELD VERIFY AND LOCATE ALL UTILITY CROSSINGS THROUGH, UNDER, AND ADJACENT TO WALLS.

DATUM

NAVD 88

HORIZONTAL SCALE

20 0 20 40

FEET

VERTICAL SCALE

5 0 5 10

FEET

WALL #1 CONSTRUCTION CENTERLINE ALIGNMENT CONTROL

SEGMENT NO.	START STATION	END STATION	BEGIN NORTHING	BEGIN EASTING	END NORTHING	END EASTING	BEARING	DELTA	RADIUS	TANGENT	LENGTH
L1	19+75.00	20+45.10	223956.63	806382.49	224018.09	806348.77	N28°45'15"W				70.098'
L2	20+45.10	20+87.14	224018.09	806348.77	224054.95	806328.56	N28°43'32"W				42.037'
C1	20+87.14	20+96.23	224054.95	806328.56	224062.88	806324.11		1°11'12"	439.02'	4.55'	9.093'
C2	20+96.23	23+27.57	224062.88	806324.11	224224.56	806162.43		30°17'23"	437.60'	118.44'	231.338'
L3	23+27.57	23+74.27	224224.56	806162.43	224223.18	806115.74	S88°18'50"W				46.702'

No.	Date	Revision	By	Appr.
1	01/15/26	ADDENDA #4	KC	JS

PERTEET
123 OHME GARDEN ROAD, SUITE 8
WENATCHEE, WA 98801
800.615.9900

PUBLIC WORKS CHELAN COUNTY

811 Know what's below. Call before you dig.

NICHOLAS CRAIG EATON STATE OF WASHINGTON 24822 REGISTERED PROFESSIONAL ENGINEER

Drawn By N. EATON	Date 11/2025
Designed By J. LEE	11/2025
Checked By L. SAUGEN	11/2025
Approved By N. SZOT	11/2025

CRP #650

Project Number 20220011

CHELAN COUNTY

TOTEM POLE ROAD PHASE 1

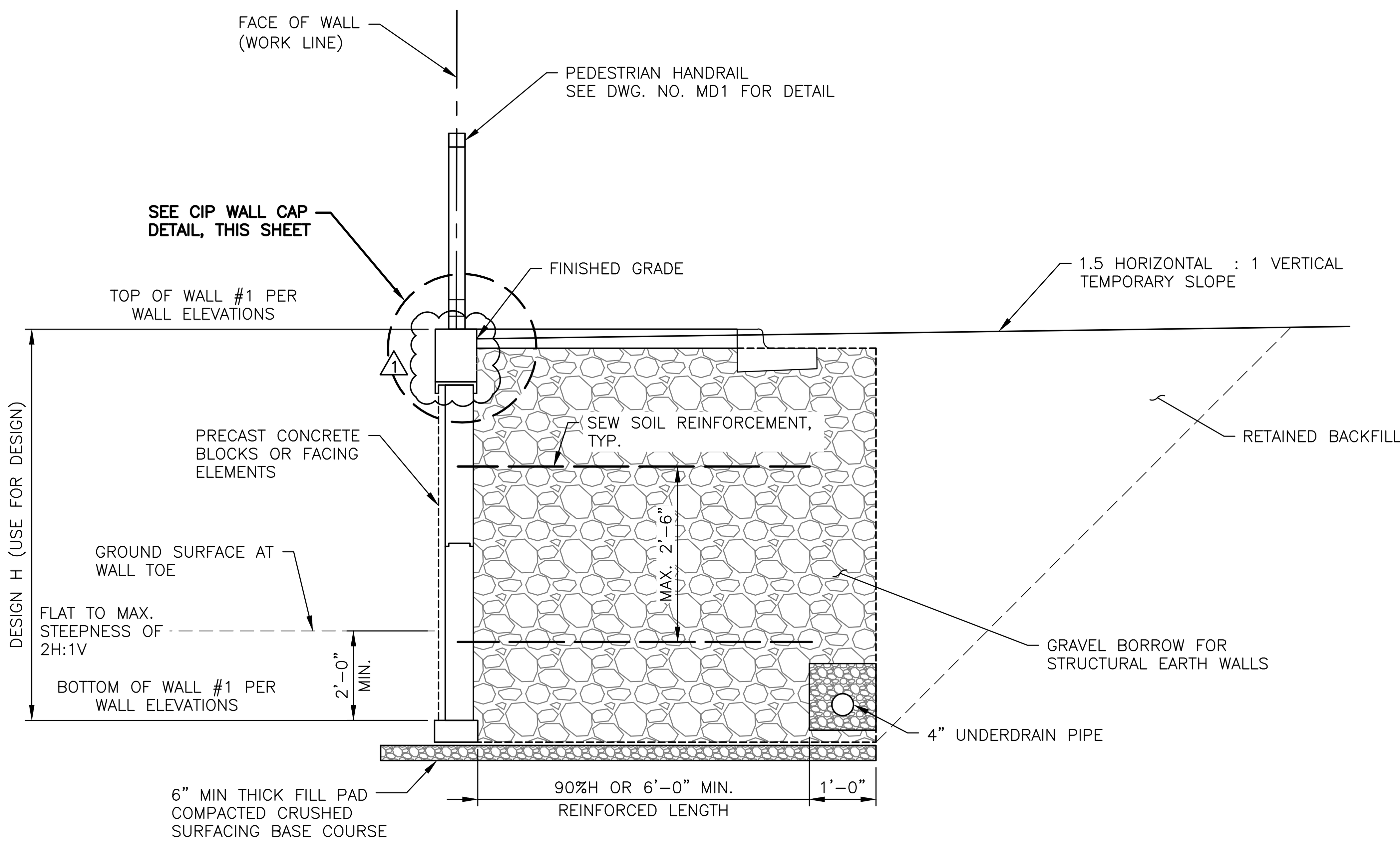
RETAINING WALL PLAN AND PROFILE

Drawing No. WP1

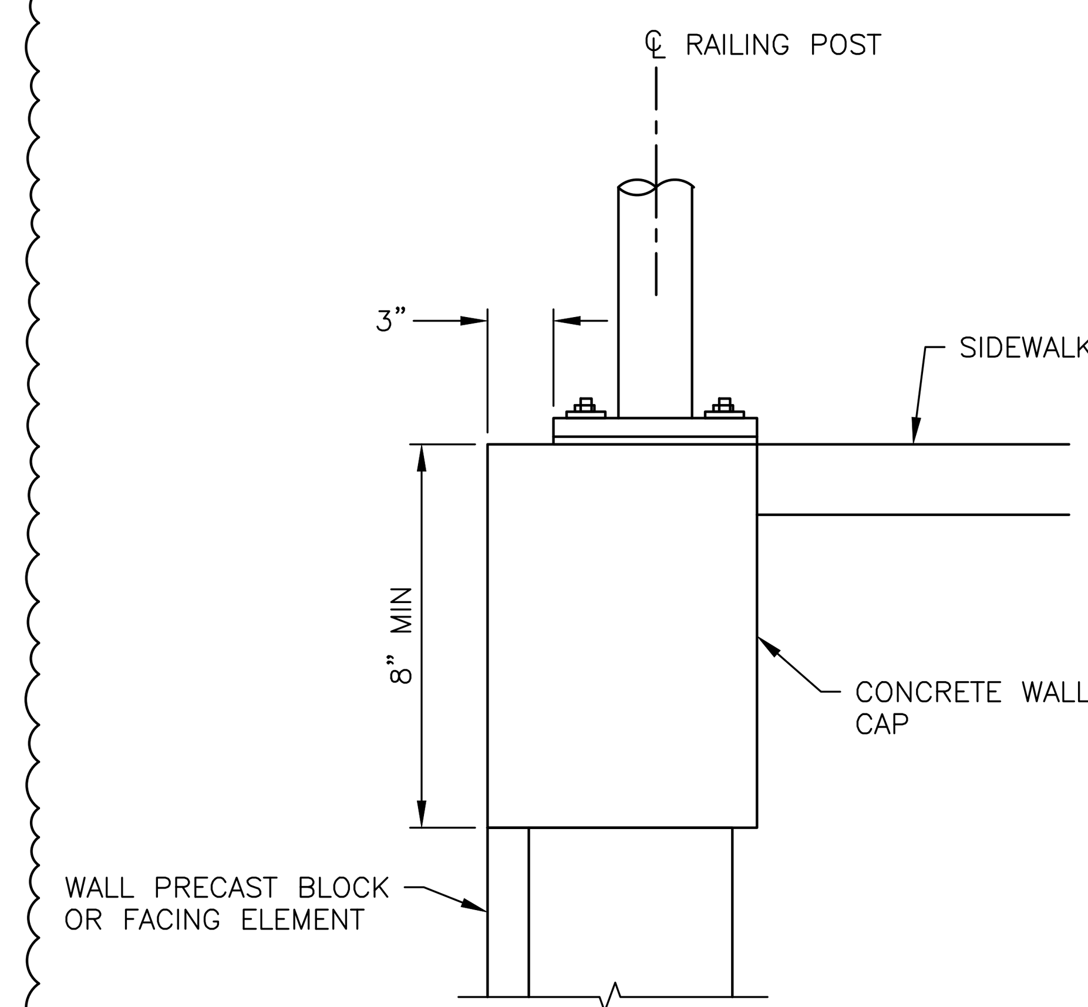
Sheet No. 30 of Total 50

WALL NOTES:

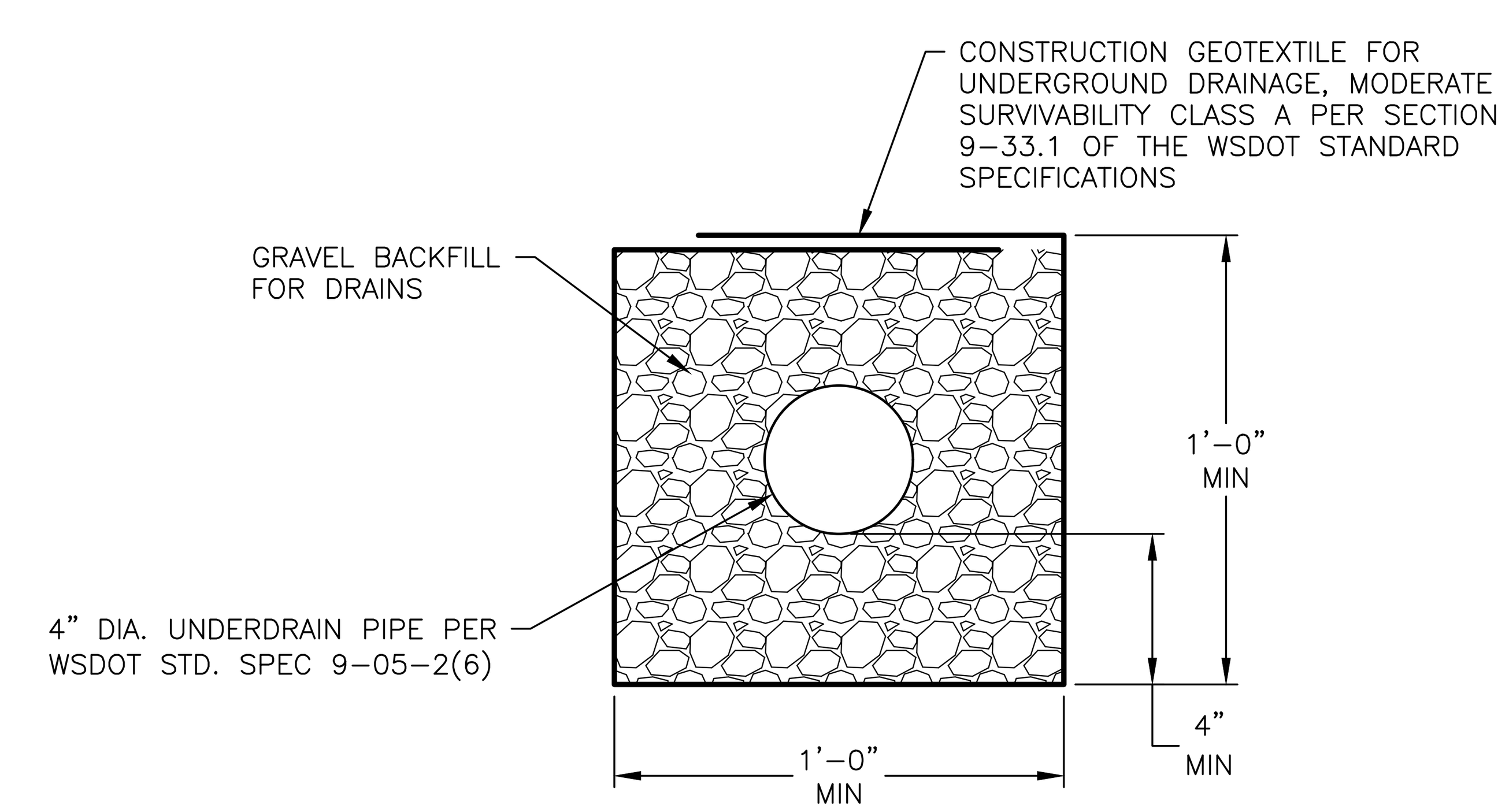
1. ALL CONCRETE SHALL BE CLASS 4000.
2. WSDOT SPECIFIED GRAVEL BORROW FOR SEWS IN ACCORDANCE WITH WSDOT STANDARD SPEC 9-03.14.(4) (WSDOT, 2025).
3. WSDOT SPECIFIED CRUSHED SURFACING BASE COURSE FOR FILL PAD IN ACCORDANCE WITH WSDOT STANDARD SPEC 9-03.9.(3) (WSDOT, 2025).
4. WALL SURFACES SHALL BE GRAY PRECAST CONCRETE BLOCKS OR FACING ELEMENTS.
5. SEW REINFORCEMENT, SOIL STRENGTH, UNITS WEIGHT, SEISMIC PARAMETERS, AND WALL SETTLEMENT PERFORMANCE CRITERIA SHALL BE AS SPECIFIED BY SPECIFIED BY 6-13.3(2)A AND SPECIAL PROVISIONS.
6. WALL CAP PAID FOR UNDER BID ITEM STRUCTURAL EARTH WALL.
7. ~~TOP OF WALL CONDITION VARIES PER GRADING PLANS. CONCRETE CAP AND EDGE TO BE REVIEWED IN SHOP DRAWING.~~
8. TOP OF WALL CONDITION VARIES PER GRADING PLANS. CONCRETE CAP AND EDGE TO BE REVIEWED IN SHOP DRAWING AND SUBMITTAL. RAILING POST CONNECTION ON CONCRETE CAP AND EDGE; OR THICKENED EDGE OF CONCRETE SIDEWALK DESIGNED AS A STRUCTURAL MOMENT SLAB BY CONTRACTOR. (1)



SECTION – WALL #1
N.T.S.



CONCEPTUAL CIP WALL CAP DETAIL
N.T.S.



TYPICAL UNDERDRAIN DETAIL
N.T.S.

No.	Date	Revision	By	Appr.
1	01/15/26	ADDENDA #4	KC	JS

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800.615.9900

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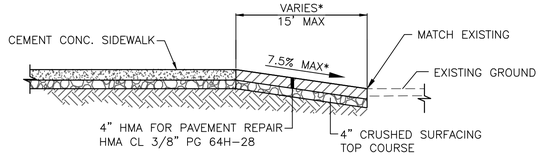
NICHOLAS CRAIG SZOT, STATE OF WASHINGTON, 48871, REGISTERED PROFESSIONAL ENGINEER, 1-15-26

		Date
Drawn By	N. EATON	11/2025
Designed By	J. LEE	11/2025
Checked By	J. SAUGEN	11/2025
Approved By	N. SZOT	11/2025

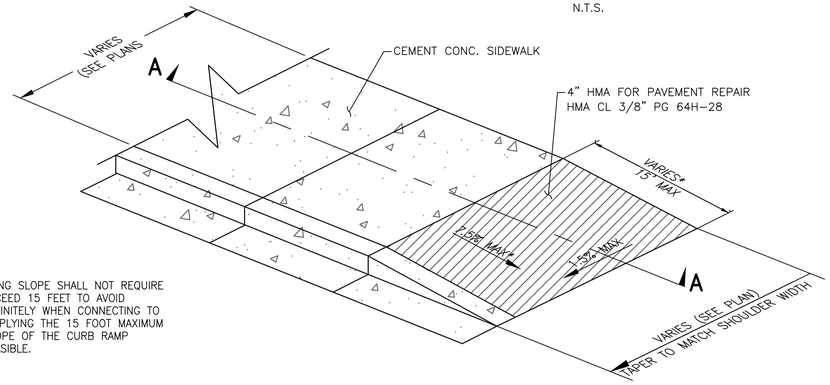
CRP #650
Project Number 20220011

CHELAN COUNTY
TOTEM POLE ROAD PHASE 1
WALL DETAILS

Drawing No. WD1
Sheet No. 31 of 50 Total

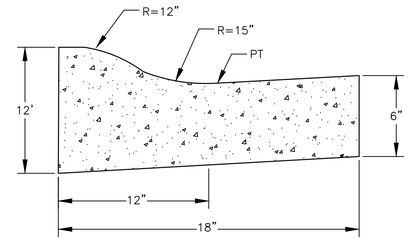


SECTION A-A
N.T.S.



* THE RAMP MAXIMUM RUNNING SLOPE SHALL NOT REQUIRE THE RAMP LENGTH TO EXCEED 15 FEET TO AVOID CHASING THE SLOPE INDEFINITELY WHEN CONNECTING TO STEEP GRADES. WHEN APPLYING THE 15 FOOT MAXIMUM LENGTH, THE RUNNING SLOPE OF THE CURB RAMP SHALL BE AS FLAT AS FEASIBLE.

ASPHALT SIDEWALK TRANSITION RAMP DETAIL
N.T.S



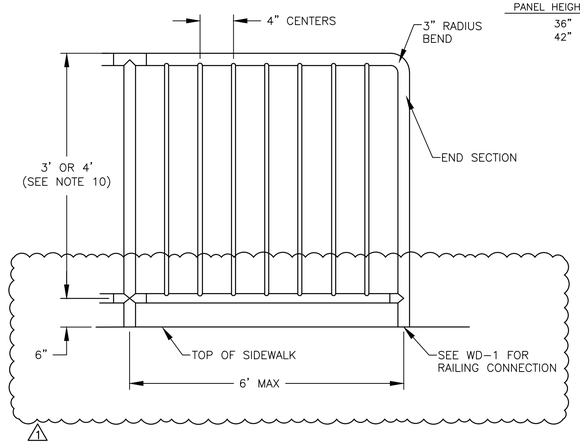
CEMENT CONC. ROLLED CURB & GUTTER
N.T.S

CONSTRUCTION NOTES:

1. HMA CL. 3/8 IN. PG 64H-28
2. CRUSHED SURFACING BASE COURSE (MAXIMUM LIFT 6 INCHES)

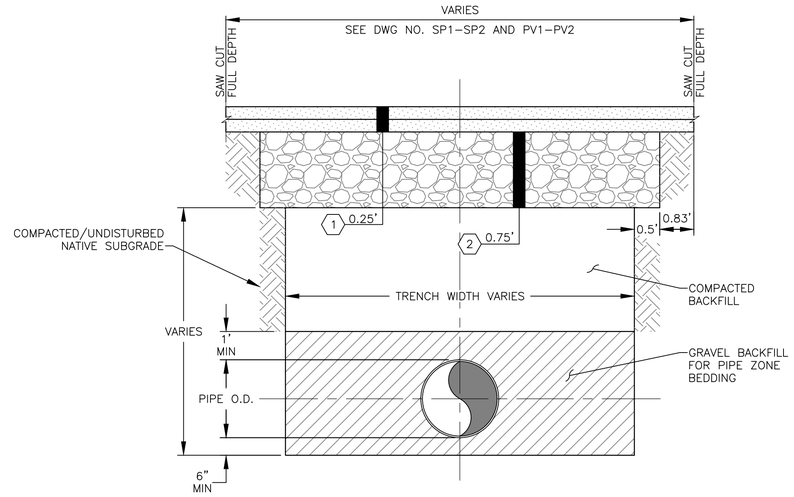
PIPE SCHEDULE
(ALL DIMENSIONS O.D.)

PANEL HEIGHT	TOP RAIL/POST	BOTTOM RAIL	BALUSTER
36"	1.90"	1.90"	.840"
42"	2.875"	2.375"	.840"



NOTES:
1. RAILING SHALL BE ALUMINUM PIPE RAIL OR APPROVED EQUIVALENT. INSTALLATION PER MANUFACTURER'S RECOMMENDATIONS.
2. SHOP DRAWINGS OF RAILING SHALL BE SUBMITTED FOR APPROVAL SHOWING COMPLETE DIMENSIONS AND DETAILS OF FABRICATION AND INCLUDING AN ERECTION DIAGRAM. MATERIALS BEING USED SHALL BE SPECIFIED IN THE SHOP DRAWINGS.
3. ALL ALUMINUM PARTS SHALL BE GIVEN A CLEAR ANODIC COATING AT LEAST 0.0006 INCHES THICK AND BE HOT WATER SEALED AND SHALL HAVE UNIFORM FINISH.
4. PIPE RAILING AND PIPE RAILING SPLICES MAY BE HEATED TO NO MORE THAN 400'F FOR A PERIOD TO EXCEED 30 MINUTES TO FACILITATE FORMING OR BENDING.
5. CUTTING SHALL BE DONE BY SAWING OR MILLING AND ALL CUTS SHALL BE TRUE AND SMOOTH. FLAME CUTTING WILL NOT BE PERMITTED.
6. PIPE RAILING, PIPE BALUSTERS AND PIPE RAILING SPLICES SHALL BE ADEQUATELY WRAPPED TO ENSURE SURFACE PROTECTION DURING HANDLING AND TRANSPORTATION TO THE JOB SITE.
7. WELDING OF ALUMINUM SHALL BE IN ACCORDANCE WITH THE LATEST AASHTO STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS.
8. ALLOW FOR EXPANSION AT APPROXIMATELY EVERY FOURTH POST.
9. RAILS, POSTS AND FORMED ELBOWS SHALL BE A.S.T.M. B-241 OR B-429 ALLOY, 6063-T6 SCHEDULE 40 (STD. PIPE). BRACKETS, ENDCAPS AND OTHER FITTINGS SHALL BE A.S.T.M. 6063-T5. SPLICES AND REINFORCING SLEEVES SHALL BE DRAWN ALUMINUM TUBING 6063-T832. SLEEVE I.D. SHALL BE 1" GREATER POST O.D.
10. PANEL HEIGHT: 36 INCHES FOR PEDESTRIAN USES
    42 INCHES FOR BIKE PATHS.

PEDESTRIAN HANDRAIL IN SIDEWALK
N.T.S.



TRENCH REPAIR DETAIL
N.T.S.

No.	Date	Revision	By	Appr.
1	01/15/26	ADDENDA #4	KC	JS

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JENNIFER D. SAUGEN — STATE OF WASHINGTON — 47014 — REGISTERED PROFESSIONAL ENGINEER
01-16-26

		Date
Drawn By	N. EATON	11/2025
Designed By	M. WARNER	11/2025
Checked By	J. LEE	11/2025
Approved By	J. SAUGEN	11/2025

CRP #650

Project Number 20220011

CHELAN COUNTY
TOTEM POLE ROAD PHASE 1
MISCELLANEOUS DETAILS

Drawing No. MD1
Sheet No. 38 of Total 50