

Underwater Inspection Report West Cashmere Bridge Chelan County, WA

Bridge No. 401 Structure No. 08430900



Prepared For:

Chelan County 316 Washington Street, Suite 402 Wenatchee, WA 98801

ATTN: Ms. Paula Cox, PE Project Manager Tel: 509 / 667.6415 Prepared By:

Echelon Engineering, Inc. 21027 61st Avenue West Lynnwood, WA 98036

ATTN: Ms. Shelley Sommerfeld, PE President Tel: 425 / 672.8924

Inspection Date: August 21, 2012 12-2419B



Civil / Marine Consulting Engineers

October 12, 2012

Chelan County 316 Washington Street, Suite 402 Wenatchee, Washington 98801

ATTN: Ms. Paula Cox, PE Project Manager

RE: Underwater Inspection of West Cashmere Bridge, Chelan County, Washington Bridge No. 401 / Structure ID 08430900

Dear Ms. Cox:

This report is submitted to document the findings of our recent underwater inspection of the West Cashmere Bridge in Cashmere, Washington. The structure is an eight span steel deck truss structure supported by reinforced concrete piers and approach bents. Pier 5 was located in the river channel at the time of the inspection. Pier 5 is a cast in place reinforced concrete pier with a spread footing and foundation piles. The project was conducted by Echelon Engineering on August 21, 2012.

1. EXECUTIVE SUMMARY

Based on the observed condition, all inspected substructure components appear sound. No evidence of any cracking, spalling or other significant deterioration of the concrete was noted. However, one area on the south side of the web wall was noted to have an area of form void and scale that appears to have resulted from poorly consolidated concrete during the original construction. Monitoring of this defect is recommended. Additionally, exposure of the footing was noted at the upstream end of the pier (i.e. max. vertical height of 6 inches). No exposure of the foundation piles was found. Moderate debris build-up was evident at the upstream end of the pier in the area where the footing is exposed. Although localized scour was found in the area of the debris build-up, no significant general or localized scour patterns were identified. Conditions appear similar to that reported in the 2007 underwater inspection report.

2. INSPECTION FINDINGS

The West Cashmere Bridge located on Goodwin Road in Cashmere, Washington is an eight span steel deck truss bridge supported by reinforced concrete piers and abutments. Numbering of the structure has been designated south to north, consistent with the previous inspection report. Pier 5 was the only pier located within the channel during this inspection. Construction drawings for the bridge were not available at the time of this inspection, however, the report covering the previous underwater inspection, conducted by others in 2007, was available.

Investigation of the concrete pier showed it to be in generally good condition. The concrete was found to be sound. No significant damage was noted on Pier 5. However, one area on the south side of the web wall was noted to have an area of form void and scale which appears to have resulted from poorly

consolidated concrete during the original construction. The affected area is \sim 3 ft. x 1 ft. and ranges from $\frac{1}{2}$ inch to a maximum of 4 inches deep. No exposed reinforcing or rust bleeding was noted to be associated with this area, however, access was limited due to the presence of a large rock. As this defect does not appear to have changed significantly since the previous inspection, no repair appears warranted at this time and we recommend monitoring the area during future inspections.

Minor velocity abrasion of the concrete surfaces exposed to flow was also evident on the pier (i.e. ³/₄ to 1 inch deep). The area of greatest abrasion was found to be on the upstream end of the pier extending from the high water mark to the mudline.

The previous inspection report noted partial exposure of the Pier 5 footing at the upstream end. Although access was limited due to the presence of timber debris at the nose of the pier, this investigation found similar exposure of the footing with a maximum vertical exposure of 6 inches noted. No evidence of undermining of the footing or exposure of the foundation piling was found. A moderate build-up of timber debris was noted on the upstream end of the pier which consists of several large logs along with numerous twigs and sticks. This condition resulted in limited access of this area to inspection.

The water level experienced during the inspection was El. 794.3'. All elevations have been determined utilizing the top of the concrete at Pier 5 of El. 817.0' as provided in the previous inspection report.

Although localized scour was evident at the upstream end of Pier 5, the scour is associated with the debris build-up and at this time is minor. No other localized scour or general scour patterns were identified. The channel bottom was found to be primarily 3-12 inch rock with cobbles and some areas of sand. A number of large rocks ranging from 1-3 ft. in diameter were present in the vicinity of the pier, but based on the information in the 2007 inspection report, these large rocks appear to be moving down river during periods of heavy flow. Probing of the channel bottom in the vicinity of the piers, using a #3 rebar allowed 3 – 9 inches of penetration primarily through the cobbles. Although minor shifting of the channel bottom was encountered, no significant change was noted when compared to the data provided in the 2007 inspection report.

It has been a pleasure to have worked with you on this project. Should you have any questions concerning this report, or if we can assist you further, please do not hesitate to contact our office.

Sincerely, Echelon Engineering, Inc.

Shelley D. Sommerfeld, P.E. President



SDS:jds Enclosures

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Underwater Inspection Report

Bridge Number		Route Goodwin Boad		Agency/Owner Chelan County	Date August 21, 2012	
401			Goodwin Hodd	Official County		
West Cashmere Bridge					Wenatchee River	
Inspector				Identification No.	Hours on Site	
Shelley	y Sommerfeld,	P.E.		WA Br. Insp. No. G9912	1.5	
Dive Cor Echelo	ntractor on Engineering,	Inc.				
Diver Na	me			Diver Name		
Erling	Vegsund					
Structure Reinfor	Structure Type Reinforced Concrete Pier & Steel Deck Truss Bridge			Substructure Type Reinforced concrete pier		
Foundation Type Concrete spread footing w/ foundation			oundation piles	Number of Spans 8	Number of Piers in Water 1	
Interior Bents (1) 1 Concrete Piers/Col			Concrete Piers/Column	1S -		
 Abu	ıt/Pier Wall (2)	2	- Inspection found the submerged portions of Pier 5 to be in overall good			
Web Wall (3)			condition with no evidence of any cracking or spalling.			
Columns (4)		- Moderate velocity abrasion was noted on the concrete surfaces of Pier 5				
Shaft (5)			within the elevation of water fluctuation (i.e. \sim 3/4 to 1 in. deep).			
Piles (6)		- One area located on the south side of the web wall was noted to have an				
Bracing (7)			area of form void and scale which appears to have resulted during			
Foundation (8)			construction. The area is \sim 3 ft x 1 ft with a depth of $\frac{1}{2}$ - 4 inches resulting from			
Footing (9)			poor consolidation. No reinforcing or rust bleeding was visible. However,			
Sea	ıl (10)		there was limited assess due to a large boulder.			
Pile	s (11)	8	Foundation			
🛛 Scou	ur (12)	9,11	- Inspection of the mudline found exposure of the footing on the upstream end			
Sco	our Mitigat. (13)		of the pier with the maximum vertical height of exposure measured at 6 inches.			
🔀 Char	nnel (14)	12	- Pier 5 was noted to have an area of localized scour at the upstream end of the			
Stre	eambed(15)		pier where timber debris has become trapped. No significant scour condition			
Drift	t(16)		exists at this time. Although exposure of the footing was noted, no exposure			
Flov	w (17)		of the foundation piles was evident.			
14						
14	- The channel battom in the vicinity of the piers was found to consist primarily of 3,12 inch rock and					
10		annel bottom in the vicinity of the piers was found to consist primarily of 3-12 inch rock and				
	of the pier	cobbies with some areas of sand. A number of large diameter rocks (1-3 ft) are located in the vicinity				
	of the pier. Based on the previous 2007 report, it appears that these are moving down river during					
16	- Moderate or	periods of neavy flow.				
			ts from the water surface to	the channel bottom and cons	sists of several large	
		vith num	nerous twice and sticks	and ongrinor bottom and cone	siste of bovorul lurgo	
17	Elow does not appear to be restricted through the channel					
		or uppe				



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Inspector Shelley Sommerfeld, P.E	. (WA Bridge Inspector No	o. G9912)		Date August 21, 2012		
Bridge Number 401	Bridge Name West Cashmere Bridg	ge				
Bridge Type Reinforced Concrete Pie	r & Steel Deck Truss Bridge	e Waterw	Waterway Name Wenatchee River			
Dive Objective Inspect submerged bridg	ge members and obtain hy	drographic da	ta.			
Dive Operation						
Type of Operation \square	SCUBA Snorkel	Other _				
Equipment	Suit Dry Suit					
Air S	upply Aluminum 80 / Air					
Site A	ccess North shoreline					
Inspection	Tools Nikonos camera pro	bing rod ham	ner scraper U/W light	misc inspection tools		
Benair	Tools Not Applicable	bing roa, nami				
Ropair Mat	roois <u>Not Applicable</u>					
Conditions						
Water	Salt Fresh	Brackish	Temperature <u>63</u>	°F Visibility <u>10 ft.</u>		
Surface	Calm Choppy	Rough				
Surf	Small Medium	Large				
Lide	☐ Flign ☐ Low					
Weather			\square Overcast	≚ Air Temperature 75 °F		
Thermocline	Temperature <u>N/A</u> °F	Depth <u>N</u>	/A	nii Tomporataro <u>- To -</u> T		
Dive Checks	Eirot Aid Equipment on (Site	M Physical Condition	on of Divor(a) Chaokad		
			\boxtimes Communications for Diver(s) Checked			
	Dive Gear Inspected		Team Briefed and Understand Dive Plan			
	Air Source Checked		Special Site Hazards Noted			
	Π					
Dive Plan and Dive Team	Procedures					
General – Verify drawings	and investigate submerged e	elements.				
1. Determine access loc	ation – Access for inspection	obtained on the	e north shore. All inspe	ection activities conducted f		
2. Hold pre-dive safety	neeting to discuss planned di	ive, roles, respo	onsibilities, review emer	gency procedures and		
andition of divora	Set up dive equipment and co	nduct function	and safety checks	geney precedence and		

4. Take underwater photos. Once photos complete, conduct probing of the mudline and obtain hydrographic information.



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Dive Schedule							
Dive No.	Entry Time	Exit Time	Total Time in Water	Maximum Depth	Remarks		
1	5:50 PM	6:25 PM	0:35 min.	7 ffw	EBV		
	•	1	1	1			

Dive Narrative

The bridge was accessed from the north shore. Investigation of Pier 5 was conducted by diving. Data was relayed to the support personnel (in a small skiff) using voice communications.

All underwater elements appear sound. The pier was found to be in overall good condition. Investigation of the concrete surfaces found them to be sound with moderate velocity abrasion of the areas exposed to flow (approx. ³/₄ - 1 inch deep). The area of greatest abrasion was noted on the upstream end of the pier extending from the high water mark to the channel bottom.

One area on the south side of the web wall was noted to have an area of form void and scale which appears to have resulted from poorly consolidated concrete during the original construction. The affected area is \sim 3 ft. x 1 ft. and ranges from ½ inch to a maximum of 4 inches deep. No exposed reinforcing or rust bleeding was noted to be associated with this area, however, access was limited due to the presence of a large rock.

Investigation of the pier foundation noted the footing to be exposed at the upstream end for a maximum height of 6 inches. Moderate amounts of timber debris were encountered at the upstream end of the pier. This condition did limit accessibility to the pier at the upstream end.

Probing of the mudline in the area around the piers found the bottom to be primarily 3-12 inch rock and cobbles with a number of large diameter rocks and some areas of sand. Probing of the channel bottom in the vicinity of the pier using a #3 rebar achieved 3-9 inches of penetration primarily through the cobbles. Although the pier exhibits minor localized scour at the upstream end no significant scour patterns or any undermining of the foundations was identified.

Dive Team Members	Shelley Sommerfeld, PE (Print Name)	Team Leader / Notes (Role)
	Erling Vegsund (Print Name)	Dive Inspector (Role)
	Travis Seigal (Print Name)	Topside Support (Role)
	Ryan Jenson (Print Name)	Topside Support (Role)
	(Print Name)	(Role)



PHOTO No. 1: West Cashmere Bridge, Looking Northwest - Note the build-up of timber debris on the upstream nose of Pier 5 and the minor abrasive scale evident from the high water mark down.

PHOTO No. 2: Pier 5, Looking East -Inspection of the pier found the concrete to be sound and in good condition. Note the embedded steel plate and the timber debris on the nose of the pier.



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PHOTO No. 3: Pier 5, Looking North - Inspection of the leading edge of the pier found the steel plate to be well secured and evidence of minor abrasive scale on all concrete surfaces exposed to flow.



PHOTO No. 4: Pier 5, Upstream End - Note the good condition of the concrete and the embedded steel nose plate. Also note the large diameter timber debris that is trapped at the nose of the pier.





PHOTO No. 5: Pier 5, Upstream End, Close-up - Note the minor abrasive scale at the waterline. This area was noted to have sustained the most significant abrasive scale.



PHOTO No. 6: Pier 5, North Side - Note the good condition of the concrete in this area where the upstream column meets the web wall.





PHOTO No. 7: Pier 5, Upstream End - Note the embedded steel nose plate behind the build-up of timber debris at the upstream end of the pier. Also note the good condition of the submerged concrete and the rock and gravel bottom evident beneath the debris.



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PHOTO No. 8: Pier 5, Downstream End - Note the good condition of the submerged concrete and the rock, sand and silt bottom composition.



PHOTO No. 9: West Cashmere Bridge, Looking West - Note the well vegetated shoreline on both banks downstream of the bridge.



PHOTO No. 10: West Cashmere Bridge, Looking East - Note the well vegetated shoreline on both banks upstream of the bridge.



URB TO CIRE 20'



NOTES:

- I. Reference elevation: Pier 5, Top of Concrete Pier El. +817.0'.
- 2. Reference previous underwater inspection report by WSDOT Dive Team, dated August 27, 2007.
- 3. Reference As-built plans, Cashmere West Bridge over the Wenatchee River, Chelan Co. WA. Built in 1929.

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	CHELAN COUNTY			
	West Cashmere Bridge			
	Br. No. 401; Underwater Inspection			
	PLAN & PROFILE			
	DATE: Aug. 2012 ECHELON ENGINEERING, INC.			
	PROJECT: 12-2419B Civil/Marine Consulting Engineers			
	SHEET: I of 2 Lynnwood, Washington			
	DRAWN: SDS / JDS			





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- AREA OF EXPOSED FOOTING

- 786.6

WENATCHEE RIVER $\triangleleft \frown$ FLOW

		GRAPHIC SCALE			
	0	5'	10'	15'	
CHELAN	COUN				
West Cae Br. No. 40	shmere Ol; Und	e Brida Ierwate	ge er Insp	ection	
PIER 5 - Plan & Elevation					
DATE: Aug. 2 PROJECT: 12-2	2012 419B		ON ENGINE arine Consi	ERING, INC. Ilting Engineers	
SHEET: 20 DRAWN: SDS /	2 [JDS	Lyn Tel	nwood, Was : (425) 672-	hington -8924	