

PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE PROJECT

Prepared for

Washington State Department of Ecology Office of the Columbia River 303 S. Mission Street, Suite 200 Wenatchee, WA 98801

Chelan County Natural Resources Department 316 Washington Street, Suite 401 Wenatchee, WA 98801

Prepared by

Anchor QEA, LLC 720 Olive Way, Suite 1900 Seattle, Washington 98101

December 2012

APPRAISAL STUDY PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE PROJECT

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LIST OF ACRONYMS AND ABBREVIATIONS

CCNRD	Chelan County Natural Resources Department
cfs	cubic feet per second
Chelan PUD	Public Utility District No. 1 of Chelan County
Corps	U.S. Army Corps of Engineers
DAHP	Washington Department of Archaeology and Historic Preservation
Ecology	Washington State Department of Ecology
EFH	Essential Fish Habitat
ESA	Endangered Species Act
fps	feet per second
HDPE	high density polyethylene
IID	Icicle Irrigation District
NAVD	North American Vertical Datum
NEPA	National Environmental Policy Act
NHPA	National Historic Preservation Act
NMFS	National Marine Fisheries Service
NPDES	National Pollutant Discharge Elimination System
O&M	operations and maintenance
OHWM	ordinary high water mark
PHS	Priority Habitat and Species
PID	Peshastin Irrigation District
PVC	polyvinyl chloride
RM	River Mile
SEPA	State Environmental Policy Act
TDH	total dynamic head
USEPA	U.S. Environmental Protection Agency
USFWS	U.S. Fish and Wildlife Service
USGS	U.S. Geological Survey
VFD	variable frequency drive
WDFW	Washington Department of Fish and Wildlife
WRIA	Water Resource Inventory Area
WSDOT	Washington State Department of Transportation

EXECUTIVE SUMMARY

Peshastin Irrigation District (PID) provides water for irrigation of approximately 3,700 acres in the Wenatchee River Valley. The primary water supply for PID is a diversion on Peshastin Creek approximately 2.4 miles upstream of the Wenatchee River. Additional flow is needed in Peshastin Creek downstream of the PID diversion to improve passage and habitat conditions for bull trout, Chinook salmon, and steelhead. The proposed PID pump exchange project would divert water to the PID Canal through a pump station on the Wenatchee River, allowing for reduced diversions and increased flows in lower Peshastin Creek. The increased flows would improve late summer fish passage, spawning, and rearing conditions in lower Peshastin Creek. The intent of this Appraisal Study is to provide a preliminary evaluation of project alternatives, assess whether the alternatives are viable, select one or two preferred project alternatives, and recommend steps for additional study and implementation of the project.

PID diverts up to 50 cubic feet per second (cfs) from Peshastin Creek during the late summer for irrigation. Due to diversions and natural fluctuations in flow, late summer flows in Peshastin Creek downstream of the PID diversion often fall below 10 cfs. An instream flow analysis completed by Hydrology Northwest (2012) indicates that 41 cfs is needed to provide adequate fish passage conditions for Chinook salmon through the entire length of lower Peshastin Creek. With channel improvements at the worst cross sections, the flow rate needed to maintain adequate fish passage conditions could potentially be reduced to 28 cfs. During a typical year, reducing late summer diversions by pumping approximately 20 cfs from the Wenatchee River for irrigation would allow for maintenance of a minimum 28-cfs flow in Lower Peshastin Creek, when that flow rate is available upstream of the PID diversion.

The instream flow analysis also indicated that increased flow would improve habitat for steelhead rearing, Chinook salmon spawning, and Chinook salmon rearing. An analysis of the relationship of flow versus weighted usable area (WUA), a relative measure of habitat abundance, indicates that the WUA for steelhead rearing would be 4 times greater at a flow of 22 cfs than at a flow of 5 cfs, which is typical of the historic, late summer low flow downstream of the PID diversion during a dry year. The WUA for Chinook salmon

spawning would be 20 times greater at a flow of 20 cfs than at a flow of 4 cfs. Additional discussion between regulatory agencies and local stakeholders is needed to determine how the proposed project, which is intended to improve late summer stream flows in lower Peshastin Creek, can be implemented in conjunction with other improvements to the stream channel and floodplain as part of a more comprehensive restoration approach for lower Peshastin Creek.

The following alternatives were evaluated as part of this study (See Figure 4-1):

- Alternative 1 would include a pump station on the Wenatchee River upstream of the easternmost U.S. Highway 2 bridge in the study area, 1,240 feet of pipeline, and delivery to the PID Canal 19,560 feet downstream of the PID Diversion.
- Alternative 2 would include a pump station on the Wenatchee River downstream of the easternmost U.S. Highway 2 bridge in the study area, 1,790 feet of pipeline, and delivery to the PID Canal 19,560 feet downstream of the PID Diversion.
- Alternative 3 would include a pump station on the Wenatchee River downstream of the westernmost U.S. Highway 2 bridges in the study area, 1,490 feet of pipeline, and delivery to the PID Canal 14,720 feet downstream of the PID Diversion.
- Alternative 4 would include a pump station on the Wenatchee River downstream of Dryden Dam, 3,240 feet of pipeline, and delivery to the PID Canal 14,240 feet downstream of the PID Diversion.
- Alternative 5 would include a pump station on the Wenatchee River upstream of Dryden Dam, 4,910 feet of pipeline, and delivery to the PID Canal 12,860 feet downstream of the PID Diversion.

The alternatives were evaluated by completing preliminary hydraulic analyses, geomorphic evaluations of proposed pump station sites, geologic reviews of pipeline alignments, reviews of property impacts, environmental and permitting reviews, and opinions of probable cost. The alternatives evaluation revealed the following:

• Alternative 1 would have the lowest cost (\$1.88 to \$3.90 million depending on pump capacity). The pump station site is more suitable for diverting flows under low flow conditions than the other alternatives, except for Alternative 5. However, pump station access would be a challenge, a private orchard would be impacted, and the pipeline would be steep and difficult to construct.

- Alternative 2 would have the third lowest cost (\$2.13 to \$4.43 million depending on pump capacity). The pump station site would be more accessible than Alternative 1. However, channel modifications would likely be required for diversion from the river, the pipeline would cross U.S. Highway 2 and a private orchard, and the pipeline would be steep and difficult to construct.
- Alternative 3 would have roughly the same cost as Alternative 1 (\$1.86 to \$3.97 million depending on pump capacity). This alternative would have little impact to private property. However, the Dryden Transfer Station would be impacted, channel modifications would likely be required for diversion from the river, the pipeline would be steep and difficult to construct, and slope stability risk would be high.
- Alternative 4 would have the second highest cost (\$2.76 to \$5.36 million depending on pump capacity). The pipeline alignment is more favorable from a construction standpoint. However, channel modifications would likely be required for diversion from the river, pump station access could be challenging, a private orchard would be impacted, the pipeline would have to cross U.S. Highway 2, and the overall cost would be relatively high.
- Alternative 5 offers the most favorable pumping conditions, best pump station access, lowest power requirements, and least impact to private property. However this alternative would have the highest cost (\$3.17 to \$6.15 million depending on pump capacity), would require coordination with other existing and proposed uses of the pump station site, the delivery pipeline would need to cross U.S. Highway 2, and pipeline construction would impact a newly paved roadway.

Following the evaluation of the proposed alternatives, the evaluation was presented to PID, Chelan PUD, Chelan County, and Ecology. Based on the review of project alternatives with PID and others, Alternative 1 was selected as the preferred project alternative because of more favorable hydraulic conditions at the proposed diversion location, a lower projected project cost, and the potential for improving the reliability of the PID system by providing an alternate source of supply downstream of the most vulnerable part of the system.

PID also recommended that Alternative 5 be studied further as a backup to the preferred alternative. Alternative 5 would not provide the same benefit to the PID system's reliability as Alternative 1 and would have the highest implementation costs of the project alternatives

evaluated. However, Alternative 5 would provide the most favorable hydraulic conditions at the proposed diversion location and would likely have the least impact on private property.

Improvement of fish habitat and passage conditions in lower Peshastin Creek will need to consider and evaluate stream channel and floodplain functions in more detail. It is anticipated that as the technical feasibility of the pump exchange project is further evaluated, channel and floodplain improvements will also be evaluated as part of a more comprehensive approach to improving conditions in Lower Peshastin Creek.

As part of future study of the pump exchange project, stakeholders may want to give further consideration to the potential for using the project to also reduce late summer diversions from Icicle Creek to the Icicle Irrigation District (IID) Canal. The diversions could be reduced by replacing water delivered to the PID Canal through a bifurcation at the downstream end of the IID Division 2 Canal with water pumped from the Wenatchee River. The proposed PID pump exchange facilities could also be expanded to deliver water directly to IDD Division 3A Canal. A cursory evaluation was completed of facilities that would be needed to extend deliveries to the IID Canal, which is approximately 160 to 170 feet higher than the PID Canal. The additional facilities would likely include a booster pump station at the PID Canal, additional discharge pipeline, and a delivery facility at the IID Canal. Depending on the pump capacity, the additional facilities could add \$1.65 to \$3.33 million to the project implementation costs for Alternative 1 and \$1.78 to \$4.30 million to project implementation costs for Alternative 5.

Development of a feasibility-level study, providing a more detailed design analysis of the preferred alternatives is recommended. A detailed evaluation of pump station operations would be completed as part of the feasibility study, and would include a determination of whether facilities will also be designed to reduce flows in Icicle Creek; property owner coordination; geotechnical exploration; a more detailed environmental and permitting review; a more detailed engineering analysis; and a refined cost analysis. The feasibility study would also include feasibility-level drawings for the preferred alternative, pump station operational recommendations, a detailed summary of property owner concerns and issues, geotechnical engineering requirements and environmental issues.

1 INTRODUCTION

This report presents an Appraisal Study of the proposed Peshastin Irrigation District (PID) pump exchange project. The project would provide an alternate supply of water for PID by pumping water to the PID Canal from the Wenatchee River. PID provides water for irrigation to nearly 3,700 acres west of the town of Cashmere, in Chelan County, Washington. PID typically diverts up to 50 cubic feet per second (cfs) from Peshastin Creek, approximately 2.4 miles upstream of its confluence with the Wenatchee River. The Chelan County Natural Resources Department (CCNRD) has been working jointly with the Washington State Department of Ecology (Ecology), other local and federal agencies, and local water users to implement projects in the Wenatchee River basin aimed at improving the management of water resources to better meet the needs of water users and improve instream flow conditions. This Appraisal Study was funded under a grant (Grant G1100240) from the Columbia River Basin Water Supply Development Account administered by Ecology's Office of the Columbia River.

The proposed pump exchange project would deliver water to PID through a pump station constructed on the right bank (looking downstream) of the Wenatchee River. The pump station would operate primarily during the late summer to deliver water from the Wenatchee River through a transmission pipeline to the PID Canal, located on the hillside south of the river. Pump station deliveries would allow for a reduction in diversions from Peshastin Creek at the existing PID Diversion, which would increase flow in Peshastin Creek downstream of the PID Diversion and in the Wenatchee River upstream of the pump station. The increased flows would improve late summer fish passage conditions.

1.1 Previous Studies

This study is intended to build on previous work done to evaluate the concept of pumping from the Wenatchee River as an alternative for improving flows in lower Peshastin Creek. The following subsections provide a brief review of previous studies that have evaluated pumping from the Wenatchee River to the PID Canal.

1.1.1 Peshastin Subbasin Needs and Alternatives Study

The proposed pump exchange project was initially identified as part of the *Peshastin Subbasin Needs and Alternatives Study* (Anchor Environmental 2007). That study evaluated the primary summertime water needs within the Peshastin Creek subbasin, which include diversions for irrigation and instream flow for passage of bull trout (*Salvelinus confluentus*) and Chinook salmon (*Oncorhynchus tshawytscha*). Several alternatives were identified for improving water management in the subbasin to better meet both instream and out-of-stream water needs. Pumping from the Wenatchee River was identified as a potential alternative and more detailed study was recommended.

1.1.2 Campbell Creek Reservoir Feasibility Study

Another alternative that was identified as part of the *Peshastin Subbasin Needs and Alternatives Study* was an off-channel storage reservoir in Campbell Creek Canyon. The storage reservoir would allow for capture and storage of Peshastin Creek flows during the high flow season and release of water in the late summer to offset irrigation diversions and improve flows in lower Peshastin Creek. A detailed feasibility study of the reservoir began in 2009. However, the site investigations and design analysis intended to be part of the study could not be completed due to property owner issues that prevented access to the site. The *Campbell Creek Reservoir Feasibility Study* (Anchor QEA 2010) provides a partial evaluation and summary of the Campbell Creek Reservoir alternative. It also includes a summary and evaluation of other alternatives for improving water use in the Peshastin Creek subbasin, including a preliminary evaluation of the proposed pump station on the Wenatchee River.

The evaluation provided in the *Campbell Creek Reservoir Feasibility Study* of the proposed pump exchange project included development of preliminary pumping, fish screening, pipeline, and delivery concepts. Preliminary design criteria were identified for the facilities. Five conceptual alternatives were identified, each with a different pump station location, pipeline alignment, and delivery location. A preliminary evaluation and comparison of the alternatives was provided and preliminary opinions of the probable construction and operation costs were developed for each alternative. The alternatives were evaluated for three different pump station design flow rates; 10 cfs, 20 cfs, and 40 cfs.

1.2 Appraisal Study Description

This Appraisal Study builds on the work done in the *Peshastin Subbasin Needs and Alternatives Study* and the *Campbell Creek Reservoir Feasibility Study*. The following subsections summarize the scope of work and purpose of this Appraisal Study.

1.2.1 Scope of Work

The scope of work for this Appraisal Study approved by CCNRD and Ecology included the following tasks:

- 1. **Frame the Proposal** This task included preparation of a summary of the project background, review of water supply needs, and a definition of requirements and design criteria for the pump exchange project.
- 2. Instream Flow Benefit Analysis This task, which was completed by Hydrology Northwest, included an update of the instream flow needs analysis that was completed for prior studies and development of a more detailed analysis of instream benefits that would result from pumping alternatives evaluated as part of this study.
- 3. **Appraisal Study** This task included further evaluation and development of the concepts presented in prior studies and development of this report to provide stakeholders with the information needed to make decisions regarding future implementation of the project. The following was completed as part of this task:
 - Collection and review of background information
 - A site visit to review and document existing site conditions
 - A geomorphic analysis of the Wenatchee River within the reach evaluated for a proposed pump station in this Appraisal Study
 - Geologic review and field inspection of proposed pipeline alignments
 - An initial property ownership and right-of-way investigation
 - Preliminary environmental review and permitting fatal flaw analysis
 - A preliminary evaluation of probable capital and long-term operating costs
 - A workshop and review with stakeholders to select preferred alternative(s)
 - Preparation of this Appraisal Study
- 4. **Project Management** This task included coordination with CCNRD, project updates, additional meetings, and management of the Appraisal Study.

1.2.2 Purpose

The intent of this Appraisal Study is to evaluate the project concept and alternatives in enough detail to provide PID and other stakeholders with the information needed to narrow down the alternatives to one or two preferred alternatives, identify apparent "fatal flaws" and challenges, assess whether the project is viable, and determine whether to proceed with a more detailed evaluation of the preferred alternative(s) as part of the feasibility study phase of the project.

1.3 Report Organization

This report includes the following sections:

- **Project Background** This section describes existing conditions at the proposed project site and summarizes the design criteria and requirements for the project.
- Water Supply Needs This section summarizes both instream and out-of-stream water supply needs, with reference to the detailed instream flow analysis completed by Hydrology Northwest. It also includes a summary of instream flow benefits that would result from the pumping alternatives identified in this report.
- **Description of Preliminary Alternatives** This section provides a brief description of the preliminary alternatives that were evaluated as part of this study.
- Evaluation of Preliminary Alternatives This section describes the design analyses, field investigations, and evaluations completed as part of this study, including hydraulic analysis, geomorphic analysis of the Wenatchee River, geologic review of pipeline alignments, property ownership and right-of-way investigations, and preliminary environmental review and permitting fatal flaw evaluation.
- Opinions of Probable Costs This section includes a review and update of the preliminary opinions of the probable project costs developed as part of the *Campbell Creek Reservoir Feasibility Study*, including capital costs and long-term operating costs. This section also identifies cost considerations that will need to be evaluated and refined as part of the feasibility study phase of the project.
- Summary of Alternatives and Fatal Flaw Evaluation This section summarizes the challenges and benefits of each alternative, provides a comparison of alternatives, and identifies potential "fatal flaws" or other significant challenges with each alternative presented in this study.

- **Description of Preferred Alternative(s)** This section provides a more detailed description of the preferred alternative(s) selected for further analysis and potential implementation by PID and other stakeholders.
- Additional Considerations This section outlines additional items that may warrant further consideration as part of future study and an overall approach to improving conditions in lower Peshastin Creek.
- Summary and Recommendations This section provides an overall summary of the Appraisal Study and recommendations for future study and implementation.

Tables and figures are included throughout the report. Appendices follow the main body of the report and include supplemental information and documents prepared as part of this study. These documents include photographs, the instream flow benefit analysis by Hydrology Northwest, hydraulic calculations, fish screening criteria and calculations, geologic review data, property ownership data, environmental review data, opinions of probable cost, and preliminary pump information from a pump manufacturer.

2 PROJECT BACKGROUND

This section includes a brief description of the project site, existing conditions, and design criteria and requirements for the project. The background information provided in this section is based on information from prior studies, Chelan County GIS data, field observations and measurements, photographs taken at the site (Appendix A), aerial photography, existing geologic mapping, input from PID and other stakeholders, and other publicly available sources.

2.1 Existing Conditions

2.1.1 Location

The proposed pump exchange project would be located in the lower Wenatchee River basin, near the town of Dryden, Washington. The study area is adjacent to the Wenatchee River in Section 27, of Township 24 N, Range 18 E. The Wenatchee River forms a narrow valley as it flows through the Cascade foothills to the Columbia River. Peshastin Creek joins the Wenatchee River approximately 1.5 miles upstream of the town of Dryden, at River Mile (RM) 17.9. PID diverts water from Peshastin Creek approximately 2.4 miles upstream of its confluence with the Wenatchee River for delivery to irrigators along the south side of the Wenatchee River Valley from Peshastin Creek to the town of Cashmere.

The study area is shown in Figure 2-1. The project would include a pump station located on the right bank (looking downstream) of the Wenatchee River, at a suitable location downstream of Peshastin Creek. Pump station location alternatives identified in this study are generally within a reach extending from approximately 700 feet downstream of Peshastin Creek to 7,800 feet downstream of Peshastin Creek. Each alternative evaluated also includes a transmission pipeline that would extend through county road or state highway right-of-way and, where necessary, private property from the pump station to the PID Canal. Each alternative would also include a delivery structure or piped transition at the PID Canal to dissipate energy and convey flows to the canal.

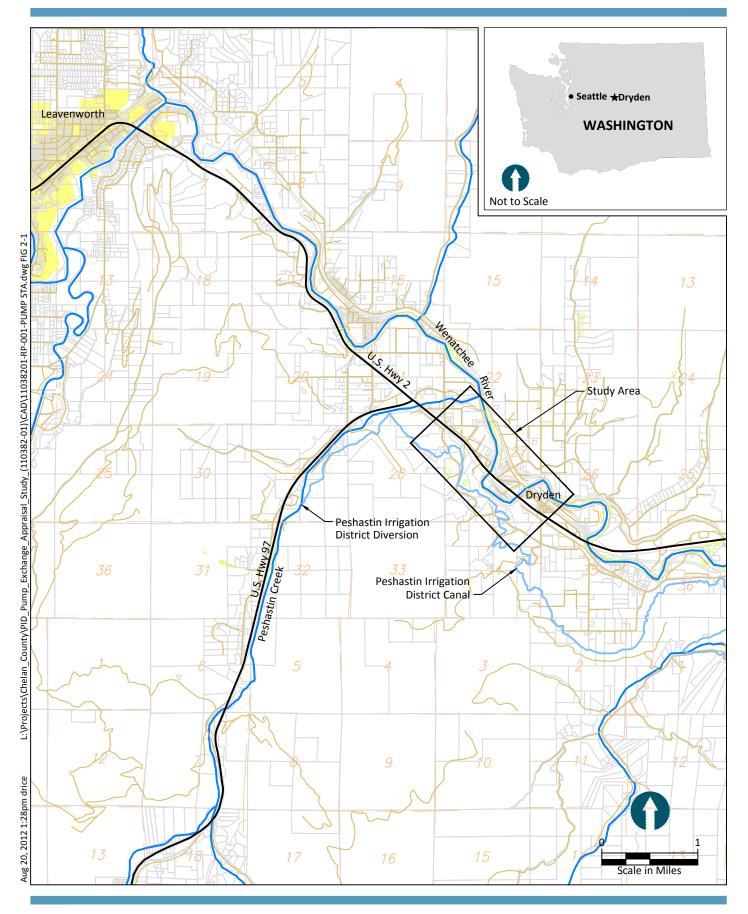




Figure 2-1 Location Map Peshastin Irrigation District Pump Exchange Appraisal Study Chelan County Natural Resources Department

2.1.2 Existing Facilities and Site Features

Existing irrigation facilities near the study area include the PID Canal and Dryden Dam. The PID Canal, which is located on a hillside south of the Wenatchee River, is more than 13 miles long and is comprised of open concrete-lined canal, unlined canal, and pipe. Within the study area, the PID Canal is mostly welded-steel pipe and open, concrete-lined canal. Canal invert elevations range from approximately 1,142 feet to 1,158 feet (North American Vertical Datum [NAVD] 88) in the study area. Elevations at the Wenatchee River range from approximately 920 feet to 972 feet (NAVD 88) in the study area.

Dryden Dam is owned and operated by the Public Utility District No. 1 of Chelan County (Chelan PUD). The Wenatchee Reclamation District diverts up to 200 cfs from the left bank of the Wenatchee River at Dryden Dam for irrigation of properties mostly in and around the City of Wenatchee, Washington. Wenatchee Reclamation District facilities include a fixed plate fish screen located in a diversion channel approximately 500 feet downstream of the dam and fish bypass facilities designed to return fish from the fish screen to the Wenatchee River. Chelan PUD also operates fish trapping facilities located on the right bank of the Wenatchee River at the Dryden Dam. The facility is used to collect adult broodstock for hatchery programs in the Wenatchee River Watershed.

Other notable features within the study area include U.S. Highway 2, local roads, bridges, homes, buildings, and the Dryden Landfill. U.S. Highway 2, which is operated by the Washington State Department of Transportation (WSDOT), is a divided 4-lane highway within the study area and is the major transportation route between Wenatchee and cities to the west, including Seattle. U.S. Highway 2 is generally aligned along the valley floor and crosses a bend in the Wenatchee River at two locations in the study area. Local roadways include paved county roads in and near the town of Dryden and local gravel roads and driveways. Primary land uses in and near the study area include rural residential and agriculture (primarily orchards). Some limited commercial and industrial uses exist along U.S. Highway 2 on a hill adjacent to the Wenatchee River. The landfill has been covered and closed. Chelan County currently operates a solid waste transfer station at the entrance to the old landfill.

2.1.3 Hydrology

The Wenatchee River is one of the larger tributaries to the Columbia River, with a watershed totaling 1,371 square miles. The watershed comprises most of Ecology's Water Resource Inventory Area (WRIA) 45. Wenatchee River hydrology is similar to other Columbia River tributaries that drain the east slopes of the Cascade Mountains, characterized by high flows in the late spring and low flows in the late summer and early fall. Annual precipitation in the watershed varies from less than 10 inches, at the Columbia River, to more than 100 inches, at the crest of the Cascade Mountains. Precipitation in the upper watershed falls mostly in the form of winter snow, so the hydrology is largely driven by snow melt in the late spring and early summer.

PID is one of several irrigation districts in WRIA 45 that rely on surface water diversions for irrigation. The pump exchange project would divert water from the Wenatchee River near Dryden, somewhere between RM 16.4 and RM 17.9. The U.S. Geological Survey (USGS) operates flow monitoring stations at Peshastin (near RM 21.4) and at Monitor (near RM 6.9). Flow statistics for the Wenatchee River at Peshastin (USGS Gage No. 12459000) and at Monitor (USGS Gage No. 12462500) are listed in Table 2-1. Exceedance hydrographs are provided in Figures 2-2 and 2-3. The USGS station at Peshastin is upstream of Peshastin Creek. The flows at Monitor reflect inflows from Peshastin Creek and Mission Creek.

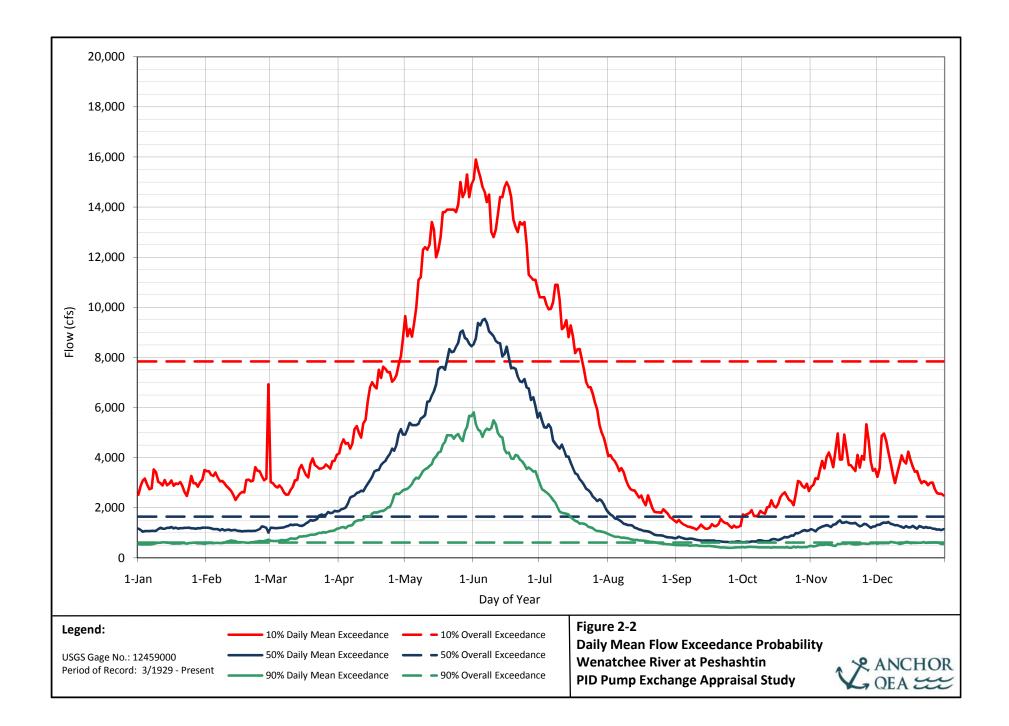
Flow Parameter (cfs)	Wenatchee River at Peshastin (USGS No. 12459000) ¹	Wenatchee River at Monitor (USGS No. 12462500) ²
Mean Daily Flow Rate	3,071	3,230
10 Percent Exceedance Daily Flow Rate	7,840	8,010
50 Percent Exceedance Daily Flow Rate	1,650	1,800
90 Percent Exceedance Daily Flow Rate	616	644
Peak Mean Daily Flow Rate	38,900	45,200
Low Mean Daily Flow Rate	210	221

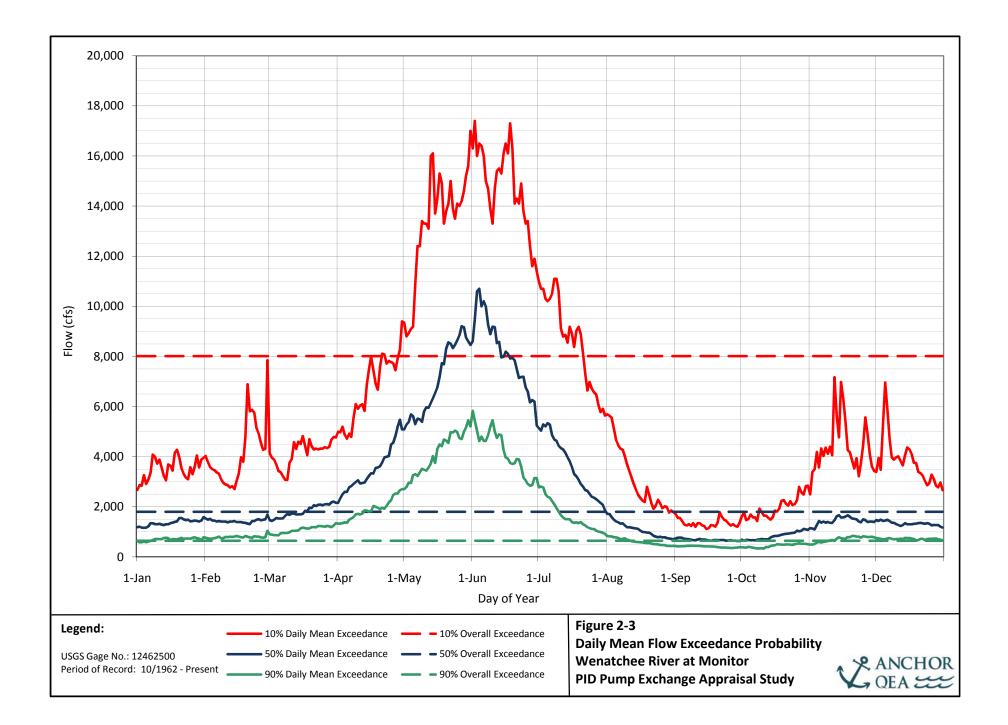
Table 2-1 Wenatchee River Daily Flow Statistics

Notes:

1. Period of Record for USGS No, 12459000 is March 1929 to present.

2. Period of Record for USGS No. 12462500 is October 1962 to present.





2.1.4 Site Geomorphology

The Wenatchee River channel through the study area is confined by geologic features and constructed infrastructure and little channel migration is evident in the historical aerial record. Upstream of the site the channel is confined for approximately 8 miles and little to no movement of the channel or gravel bar deposition is evident in the aerial record. Much of the bed and banks of the river are composed of large rock that is not typically moved by the river during high flow periods. Just downstream of the Peshastin Creek confluence is Dryden Dam and no channel movement occurs in this location. The river flows beneath U.S. Highway 2 at two locations within the study area. Bedrock is located along the right bank between the two crossing locations. In summary, the river channel through the study area has not changed in the recent past likely because of the resistant geologic features along the channel margins and significant infrastructure that stabilizes banks along the river. Therefore, change in the river channel through the study area likely only occurs during very large, infrequent flood events; and during those events, change is likely minor.

2.1.5 Geology

Review of geologic maps, soil maps, and subsurface information obtained from water well logs indicate the majority of the study area is underlain at depth by the Chumstick formation consisting of thick deposits of interbedded sandstone, shale, conglomerate, and some tuff. This formation was deposited in lacustrine and fluvial environments during the late middle to late Eocene period (55 to 34 million years ago). It is bounded on the east by the Entiat fault and on the west by the Leavenworth fault. The moderate hillside reliefs running along the east and west sides of the Wenatchee River valley are comprised mainly of Chumstick formation sandstone. Within the Wenatchee River valley, quaternary period deposits overlay the Chumstick formation, and depending on the proximity to the river channel, consist of alluvial deposits (gravels, sands, and silts), glacial outwash deposits, and landslide deposits derived from upslope weathered sandstone conglomerate. For the purposes of this study, the subsurface geology can be classified into the following main units:

• *Fine Grained Alluvium* - This unit consists of fine grained bedded silts, clays, and sands deposited during flood events within the Wenatchee river valley. The unit is typically encountered at the surface and may vary in thickness from 1 to 20 feet. This unit may also be encountered in varying thickness at higher elevations along the river

valley as it was deposited during a period of differential uplift and depression of the land surface in reference to sea level.

- *Coarse Grained Alluvium* This unit is comprised of coarse gravel and cobbles with occasional boulders and includes both alluvial deposits and glacial outwash deposits. This unit varies in thickness and may underlie the Fine Grained Alluvium unit, or is present at the surface depending on proximity to the river channel.
- *Landslide* This unit is comprised of historical landslide deposits consisting of angular sediments and fragments of pre-existing sedimentary rock deposits (Chumstick formation) derived from upslope areas. This unit is present at the surface and varies in thickness.
- Sandstone This unit includes the Chumstick formation consisting of interbedded sandstone, conglomerate, shale, and very minor tuff. Sedimentary structures and plant remains indicate deposition in environments ranging from alluvial fan and fluvial to lacustrine. Within the study area, the sandstone unit may consist of predominately shaly sandstone and shale and is susceptible to landslides along the steeper reliefs. The contact interface between the Sandstone unit and overlying Fine Grained Alluvium or Landslide units can create potential slip planes for slope stability failures.

2.2 Design Criteria

In general, the pump exchange project is intended to provide facilities that will deliver water from the Wenatchee River to PID in the late summer to allow for reduced diversions from Peshastin Creek so that more flow is available in Peshastin Creek to meet instream flow needs for fish passage. The project will also need to provide a reliable, cost effective, alternative source of supply for PID. In order to accomplish these goals, design criteria and requirements have been developed in consultation with PID and other stakeholders for analysis of the proposed project. These requirements and criteria are summarized below.

2.2.1 Design Flow Rates

A range of design flow rates were selected for analysis based on instream and irrigation water needs and the potential for meeting those needs with other sources of supply. A detailed evaluation of instream flow needs is outlined in Section 3. The evaluation included a review of the benefit to fish passage conditions that would result from increasing late summer flows in lower Peshastin Creek. PID has historically diverted up to 50 cfs during the peak irrigation season from Peshastin Creek. Increased flow in lower Peshastin Creek would result from reductions in PID's late summer diversions, which would need to be offset by pumping from the Wenatchee River or through a combination of pumping and delivery through PID's connection to a bifurcation at the downstream end of the IID Division 2 Canal. In order to evaluate a range of potential pumping and delivery conditions, the facilities were evaluated at design flows of 10, 20, and 40 cfs.

2.2.2 Pump Station

The pump station and associated transmission pipeline would be designed to deliver the required flow from the Wenatchee River to the PID Canal. Design criteria and requirements for the pump station would include the following:

- Pump Station Location The pump station would be built into the right bank (looking downstream) of the Wenatchee River somewhere between RM 16.4 and RM 17.9. The pump station site would be selected based on system hydraulics, geomorphology, accessibility, geology, environmental impacts, property impacts, and other factors that are evaluated as part of this Appraisal Study.
- Pump Station Configuration The pump station would include a screened intake (see Section 2.2.3) designed to fit into the side of the river channel and divert flow under a wide range of river conditions to reinforced concrete sumps for pumping.
- Type and Number of Pumps It is anticipated that the pumps would be vertical turbine pumps designed to draw water from reinforced concrete sumps. Two or more pumps would be included. The number of pumps would be selected based on the design flow rate and the size of pumps available.
- Pump Sizing Pumps would be sized to deliver the design flow rate from the Wenatchee River to the PID Canal through a transmission pipeline. The delivery elevation at the canal would be approximately 186 to 224 feet higher than the water surface elevation of the river. Pump and pipe sizing would be optimized by balancing capital costs with energy requirements and related long-term costs.
- Pump Station Structure The pump station structure would be reinforced concrete designed to support the pumps, screens, and related equipment. It is anticipated that

the pumps would sit on a reinforced concrete pad above the pump sumps and intake. A small metal or concrete masonry unit building, or some other type of weather tight enclosure, would be provided to house electrical and control equipment for the pumps and screen.

- Electrical and Controls Each pump would be equipped with a variable frequency drive (VFD) to optimize pump performance, provide flexibility in delivering a range of flow rates, and allow for soft pump starts and stops. The pump station would require extension of three-phase power service.
- Other Equipment Associated equipment would include discharge piping, valves, fittings, pressure transmitters and switches, a flow meter, and other appurtenances. Discharge and fittings would be steel or ductile iron with appropriate linings and coatings to reduce the risk of corrosion.

2.2.3 Intake and Fish Screening

Fish screening facilities would be designed to meet the most current requirements for screening of diversions from the Washington Department of Fish and Wildlife (WDFW) and the National Marine Fisheries Service (NMFS) *Anadromous Salmonid Passage Facility Design Guidelines* (NMFS 2008), developed by the NMFS Northwest Region. The criteria applicable to the sizing and design of a pump station intake constructed on the Wenatchee River would include the following:

- Maximum approach velocity = 0.4 feet per second (fps) for active screens, or 0.2 fps for passive screens
- Effective Screen Area = Maximum Screen Flow/Approach Velocity
- Sweeping Velocity = 0.8 to 3.0 fps
- Screen Material = Must be corrosion-resistant and sufficiently durable to maintain a smooth, uniform surface with long-term use
- Maximum Opening Size
 - 0.087 inch for Woven Wire Mesh (6-14 mesh)
 - 1.75 millimeters for Slotted Screens
 - 3/32 inch diameter for circular screen openings (includes perforated plate)
 - 3/32 inch on diagonal for square screen openings
- Minimum Open Area = 27 percent

2.2.4 Pipelines

The transmission pipeline would be designed to convey water from the pump station to the PID Canal. The following requirements and criteria would apply to the design of the transmission pipeline:

- Sizing The pipeline would be sized based on balancing pressure loss in the delivery pipeline with the corresponding increase in pumping power required. As a general guideline, pressure pipe would be sized to limit velocities to 5 fps.
- Material It is anticipated that the majority of the pipeline would be constructed with either pressure-rated steel, ductile iron (DI), high density polyethylene (HDPE), or polyvinyl chloride (PVC) pipe depending on size, pressure rating required, and other design factors.
- Pipe Roughness For completing preliminary hydraulic analyses of the proposed transmission pipeline, Hazen-Williams coefficients (C) were conservatively estimated to be 130 for plastic pipe and 110 for metal pipe.
- Trenching and Backfill The transmission pipeline would be buried with a minimum cover of at least 30 inches. Imported bedding and select backfill would be used to provide a solid foundation and protection for the pipeline.
- Pressure Rating Pipe and fittings would be rated to handle at least 150 percent of the projected maximum working pressure. Blocking and/or restraints would be used to ensure that the pipe does not move during pressure surges. No solvent welded joints on PVC pipe would be allowed.
- Appurtenances Air release and vacuum valves would be provided at key locations along the transmission pipeline.

2.2.5 Delivery to PID Canal

The type and configuration of the delivery structure will depend on whether the delivery is made to an open section of PID Canal or a section of the canal that has been piped. The following criteria would apply to the design of the delivery structure:

 Location – If possible, the structure would be located within the existing PID Canal easement. Other factors that would be considered as part of the selection of a location for the delivery to the Canal would include whether the Canal was open or piped, site topography, geology, accessibility, and environmental impacts.

- Extent and Reliability of Service The closer the delivery is located to the diversion, the more customers PID will be able to serve through the pump station. However, the most vulnerable section of the PID Canal runs through the slide area on the hillside south of the Dryden Transfer Station and the bend in the Wenatchee River. This reach includes a segment of steel pipe approximately 30 feet long that is suspended above a slide area. If the delivery to the canal is upstream of that location, the additional supply from the pump station would not provide as much benefit to the reliability of the system.
- Sizing and Configuration The structure would be designed to convey the design flow rate from the transmission pipeline to the PID Canal at velocities that would be low enough to prevent Canal erosion or damage to existing pipe, fittings, and appurtenances. The structure would also be configured to allow for accurate measurement of water levels for hydraulic control of the pump station.
- Material It is anticipated that the structure would be constructed with reinforced concrete or appropriate pipe fittings and would likely include a baffle, diffuser, or other device designed to dissipate energy.

3 WATER SUPPLY NEEDS

The proposed pump exchange project is being evaluated for its potential to improve water resource management in the Wenatchee River Watershed, and more specifically in the Peshastin Creek Subbasin, to better meet both instream and out-of-stream water needs. This section includes a brief description of the instream and out-of-stream water uses in the Peshastin Creek Subbasin, summarizes a detailed instream flow analysis prepared by Hydrology Northwest (Appendix B), and compares existing water supply to water needs.

3.1 Out-of-Stream Water Needs

The primary out-of-stream water need within the Wenatchee River Watershed is for irrigation. Additional out-of-stream water needs include municipal, light commercial and industrial, and domestic use. For surface water in the Peshastin Creek Subbasin, the primary out-of-stream water needs are irrigation supply for PID and the Tandy Ditch Company.

3.1.1 Irrigation Diversions

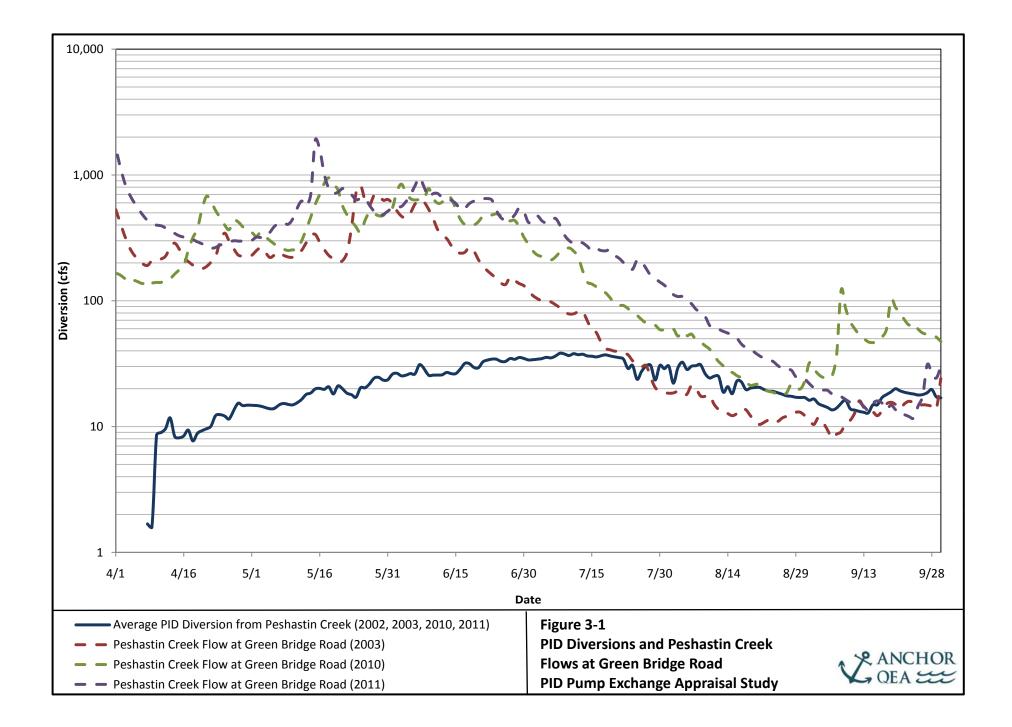
Both PID and the Tandy Ditch Company have irrigation diversions on Peshastin Creek. These are the primary surface water diversions within the Peshastin Creek Subbasin. PID diverts up to 50 cfs from Peshastin Creek, approximately 2.4 miles upstream of its confluence with the Wenatchee River. The diversions typically occur between early April and mid-October.

PID diversions from 2002, 2003, 2010, and 2011 were reviewed for this analysis. Data from 2002 and 2003 was obtained from Ecology from diversion records submitted by PID. Records from 2010 and 2011 were obtained directly from PID. Figure 3-1 includes a chart showing PID's average daily diversion from Peshastin Creek during 2002, 2003, 2010, and 2011. The chart also shows Peshastin Creek flows during 2003, 2010, and 2011, continuously measured at an Ecology flow monitoring station at Green Bridge Road, which is downstream of the PID diversion.

The data indicates that diversion rates are typically the greatest from early June through the middle of August, when irrigation demand is the highest. Diversions are reduced in late August when flows in Peshastin Creek cannot sustain peak diversion rates. Late summer

diversions are typically supplemented by flows from the bifurcation at the downstream end of the IID Division 2 Canal. During 2002 and 2003, the peak 2-week average diversion rate was just over 30 cfs. During 2010, the peak average 2-week diversion rate was nearly 40 cfs. During 2011, it was over 41 cfs. In 2010 and 2011, peak average daily diversion rates were in the range of 45 to 50 cfs. Diversion rates are affected by flow availability in Peshastin Creek, timing of the fruit harvest, and reductions in water use following the fruit harvest.

In addition to surface water from Peshastin Creek, PID relies on surface water diverted from Icicle Creek to the IID Canal during the late summer. PID has a surface water right on Icicle Creek and jointly operates the diversion and conveyance facilities, including the IID Division 1 and Division 2 Canals, with Icicle Irrigation District. The IID Division 1 and Division 2 Canals convey water to a bifurcation structure located near Peshastin Creek. The Peshastin Irrigation District Comprehensive Water Conservation Plan (Klohn Leonoff 1993) indicates that an agreement between IID and PID governs the operation of the bifurcation structure. IID has a right to the first 30 cfs. IID conveys flow from the bifurcation through a siphon under Peshastin Creek and U.S. Highway 97 to the IID Division 3A Canal. PID has the right to the next 30 cfs. Historically, 16 cfs of that amount was diverted to the Gibbs and Tandy Ditches. The remaining 14 cfs was available for conveyance through a 20-inch steel pipeline to the PID Canal. The bifurcation structure was refurbished in 1996. PID estimates the maximum flow capacity of the pipeline from the bifurcation at 30 cfs. No records have been provided documenting recent late summer deliveries to the PID Canal from the bifurcation, but the Peshastin Irrigation District Comprehensive Water Conservation Plan (Klohn Leonoff 1993) indicated that historical flows were between 7 and 14 cfs. Flows from the bifurcation are used to supplement diversions from Peshastin Creek, particularly during the late summer, when irrigation demand is still high and flows in Peshastin Creek are low.



3.1.2 Water Rights

Table 3-1 summarizes PID's surface water rights, as listed in Ecology's water rights database. PID has water right claims for diversion of up to 57.5 cfs from Peshastin Creek. PID also has water right certificates that allow for diversion of up to 34.4 cfs from Icicle Creek at the IID Canal diversion and up to 2.4 cfs from the Wenatchee River downstream of the Dryden Dam. The information contained in Table 3-1 is presented for information only and is not to be construed as a representation of PID's legal water rights.

Source	Water Right Document	Flow Rate (cfs)	Volume (acre-feet)	Area of Use (acres)	Purpose
Peshastin Creek	S4-064984CL	50.0	15,000	2,258	Irrigation
	S4-113257CL	3.1	620	155	Irrigation
	S4-064985CL	4.4	550	110	Irrigation
Total		57.5	16,170	2,523	
Icicle Creek	S4-*00329CWRIS	34.4		2,063	Irrigation
Wenatchee River	S4-CV1P260	2.4		60	Irrigation

Table 3-1 PID Surface Water Diversion Rights

Notes:

cfs = cubic feet per second

3.2 Instream Water Needs

The primary instream flow needs in lower Peshastin Creek during summer are fish passage for ESA-listed Chinook salmon and bull trout, spawning for Chinook salmon, and rearing of ESA-listed Chinook salmon, bull trout, and steelhead. A fish passage and revised instream flow analysis was completed by Hydrology Northwest as part of the development of this Appraisal Study. A report summarizing the instream flow analysis, *Lower Peshastin Creek Fish Habitat and Passage Assessment 2011* (Hydrology Northwest 2012), is included in Appendix B. The following is a brief summary of the findings of the instream flow analysis.

As part of the study, Hydrology Northwest used the "Oregon Method" (Thompson 1972) to estimate stream flows for upstream migration of Chinook salmon and bull trout. That method assumes adequate flow when depth criteria are met on at least 25 percent of an individual transect width and on at least a 10 percent contiguous portion of the transect

width. Hydrology Northwest also evaluated the impact of stream flow rates on habitat functions, including spawning and rearing for Chinook salmon, and rearing for steelhead and bull trout by computing weighted usable area (WUA), which is a relative measure of habitat abundance. Relationships between the WUA for different habitat functions and flow rate were estimated for the lower 2.4 miles of Peshastin Creek, downstream of the PID diversion, by using the calibrated transects from the Peshastin Creek instream flow study portion of the *Final Technical Report Lower Wenatchee River PHABSIM Studies* (EES Consulting 2005).

3.2.1 Fish Passage

A stream survey was completed in August and September of 2011 to complete the fish passage analysis. Five transects were surveyed for the field study. The transects include both representative and critical bar sections along lower Peshastin Creek. Transects were surveyed over the course of three field days as flows were dropping during the late summer to capture "high," "medium," and "low" late summer flow conditions for passage modeling. In addition, Hydrology Northwest surveyed lower Peshastin Creek for habitat types.

The data collected were entered into the PHABSIM hydraulic model for lower Peshastin Creek to evaluate passage conditions for a range of flows from 5 to 50 cfs. Modeled depths were tallied to determine the stream flows needed to create adequate passage conditions at each transect according to the "Oregon Method." The results are summarized for bull trout and Chinook salmon in Tables 3-2 and 3-3 and are included in Appendix B.

Table 3-2

Discharges for Meeting Bull Trout Passage Conditions

Passage Criteria	Transect T-1 (cfs)	Transect T-2 (cfs)	Transect T-3 (cfs)	Transect T-4 (cfs)	Transect T-5 (cfs)
25 Percent of Total Width	6	5	10	16	18
10 Percent Contiguous Width	6	5	6	16	30
Both Criteria	6	5	10	16	30

Table 3-3

Discharges for Meeting Chinook Salmon Passage Conditions

Passage Criteria	Transect T-1 (cfs)	Transect T-2 (cfs)	Transect T-3 (cfs)	Transect T-4 (cfs)	Transect T-5 (cfs)
25 Percent of Total Width	15	17	28	16	34
10 Percent Contiguous Width	13	13	25	10	41
Both Criteria	15	17	28	16	41

For bull trout, both fish passage criteria are met for all transects surveyed for flows in excess of 30 cfs. Fish passage criteria are met for all but the worst transect surveyed for flows in excess of 16 cfs.

Flows required to meet passage conditions for Chinook salmon are greater because the minimum depth recommended for passage (0.8 foot) is greater than the minimum depth recommended for bull trout passage (0.6 foot). For Chinook salmon, both fish passage criteria are met at all transects surveyed for flows in excess of 41 cfs. Fish passage criteria are met at all but the worst transect surveyed for flows in excess of 28 cfs.

3.2.2 WUA Results

Estimates of the WUA (a relative measure of habitat abundance) in lower Peshastin Creek were evaluated, including steelhead juvenile rearing, Chinook salmon spawning and juvenile rearing, and bull trout juvenile and adult rearing. The results, shown in Appendix B, indicate that for all species except bull trout, the WUA increases relatively consistently as flow increases. For example, the WUA for steelhead rearing is nearly 4 times more abundant at a flow rate of 22 cfs than at a flow rate of 5 cfs. Chinook salmon spawning habitat is nearly 20 times more abundant at a flow rate of 20 cfs than at a flow rate of 5 cfs.

3.2.3 Summary

The results of the instream flow study indicate that wide gravel bars in lower Peshastin Creek likely pose a significant passage barrier under low flow conditions. Flows needed for adequate passage conditions are greater for Chinook salmon than for bull trout. The average flow required to meet passage conditions at the 5 transects measured was 23.4 cfs. Although a flow of 23.4 cfs would likely create adequate passage conditions through the majority of lower Peshastin Creek, higher flows would be needed to pass fish at the most critical transects surveyed (Transects T-3 and T-5), unless channel modifications can be made to improve passage flow conditions through the worst transects.

3.3 Comparison of Water Needs to Flows

Following development of the analysis of instream water needs by Hydrology Northwest, additional analysis was completed to compare Peshastin Creek flows with water needs. The following provides a more detailed summary of flows in Peshastin Creek and compares those flows to the flows needed to support fish passage and irrigation needs.

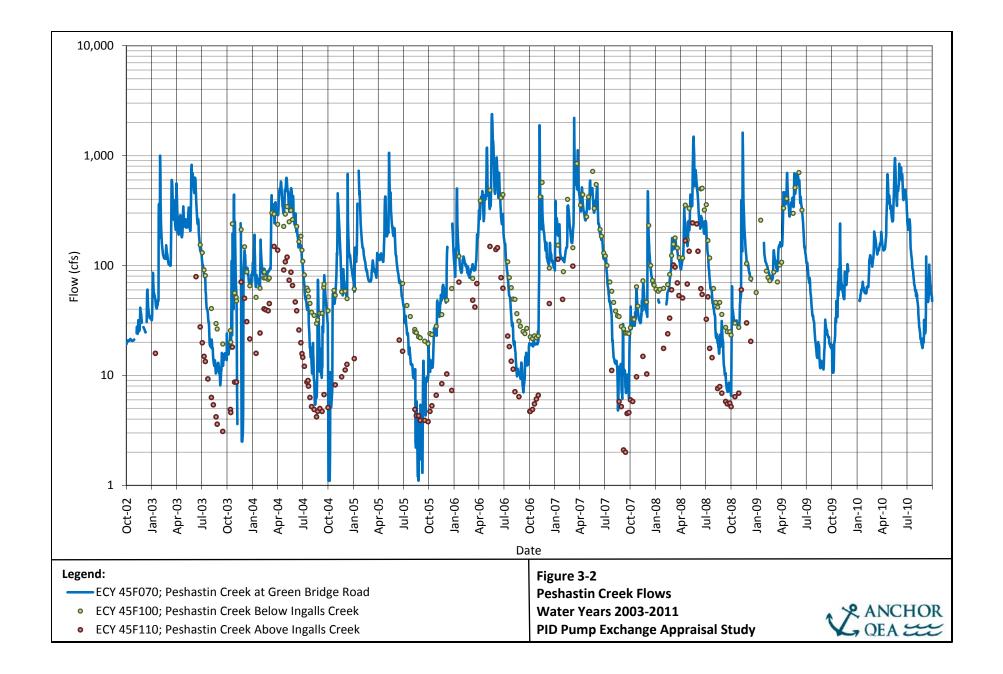
3.3.1 Peshastin Creek Flows

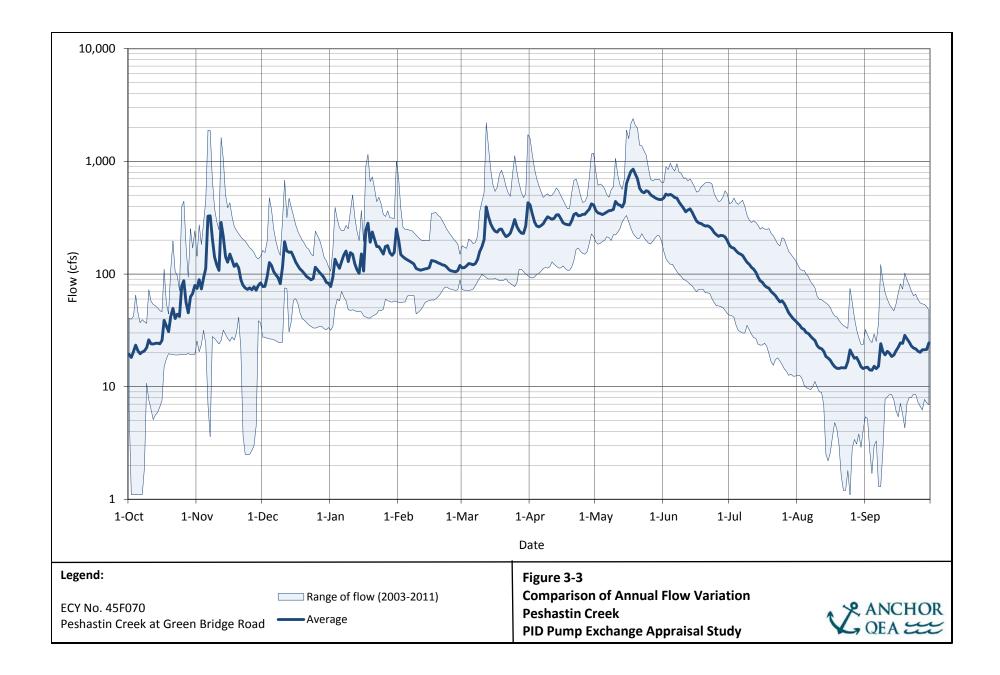
Historical flow data for Peshastin Creek was recorded at three flow monitoring stations operated by Ecology as part of the Washington State Flow Monitoring network. Those stations are:

- Ecology No. 45F110 This station is located on Peshastin Creek just upstream of its confluence with Ingalls Creek, which is at RM 9.4. The station consists of a manual stage height gage, which was read manually several times a year between 2003 and 2009. The gage has not been monitored since October 1, 2009.
- Ecology No. 45F100 This station is located on Peshastin Creek approximately 1 mile downstream of its confluence with Ingalls Creek, which is at RM 9.4. The station consists of a manual stage height gage, which was read manually several times a year between 2003 and 2009. The gage has not been monitored since October 1, 2009.
- Ecology No. 45F070 This station is located on Peshastin Creek at Green Bridge Road, approximately 1.5 miles upstream of the Wenatchee River. This is the only continuously operated station on Peshastin Creek and is the only station downstream of the irrigation diversions. The station has been in operation since October 2002.

Data from the three flow monitoring stations can be obtained from the Ecology website at https://fortress.wa.gov/ecy/wrx/wrx/flows/regions/state.asp?region=3. Figure 3-2 provides a log scale plot of historical flows measured at each of the monitoring stations. Figure 3-3 provides a comparison of the variation of annual flows measured at Green Bridge Road, plotted on a log scale, to illustrate the range of flows in Peshastin Creek at different times of the year.

Annual peak flows at Green Bridge Road typically exceed 500 cfs. Peak flows recorded in 2006 and 2007 were in excess of 2,000 cfs. Late summer flows typically drop below 10 cfs. Low recorded flows in 2004, 2005, and 2007 were less than 5 cfs. The flow rates at Green Bridge Road reflect surface water diversions to the Tandy Ditch and the PID Canal.





3.4 Comparison of Late Summer Flows to Fish Passage Flows

The flow monitoring records for Peshastin Creek at Green Bridge Road indicate that the flow in the creek is less than what is needed to establish fish passage in lower Peshastin Creek. As discussed in Section 3.2, approximately 41 cfs is needed downstream of the PID Diversion to maintain adequate fish passage conditions for Chinook salmon at all transects surveyed for the instream flow analysis. With appropriate channel modifications at key sections of the creek channel (similar to Transect T-5), the flow needed to maintain fish passage for Chinook salmon could be reduced to approximately 28 cfs.

The flows measured at Green Bridge Road and at the next upstream gage (Ecology No. 45F100) were plotted against the flow required for Chinook salmon passage for the period of 2003-2008 in Figures 3-4 through 3-8. The period of year plotted is June to November to capture the entire late summer/early fall low flow period for each year. The critical flow period for passage of Chinook salmon is August and September. The PID diversion operates until the end of September in most years. The critical flow period for passage for bull trout is from mid-August through mid-November. However, incubation and rearing of Chinook salmon and bull trout occur throughout the year. Note that the natural flow in Peshastin Creek (as estimated using Ecology Station 45F100 at RM 9.4) is less than the fish passage flow for much of the August-September time period.

Table 3-4 summarizes the flow volume that would be needed to provide the difference in flow between that measured at Green Bridge Road and the natural flow in Peshastin Creek. This volume would equal the volume of water diverted by the PID from Peshastin Creek. It is the maximum amount of water that could be provided by exchanging water pumped from the Wenatchee River with water diverted from Peshastin Creek. The volume was calculated by subtracting either the fish passage flow or the natural flow in Peshastin Creek from the flow at Green Bridge Road. Table 3-4 also summarizes the number of days in August and September that flows were less than those recommended for Chinook salmon passage, and the peak flow that would be needed to maintain either the required passage flow at Transects T-3 and T-5 or natural flow. On average, streamflows were less than the 28 cfs passage flow at Transect T-3 for an average of 56 days during August and September during the period of record. The average maximum additional flow that would have been needed to maintain a stream flow of 28 cfs downstream of Green Bridge Road during the period of record was 20

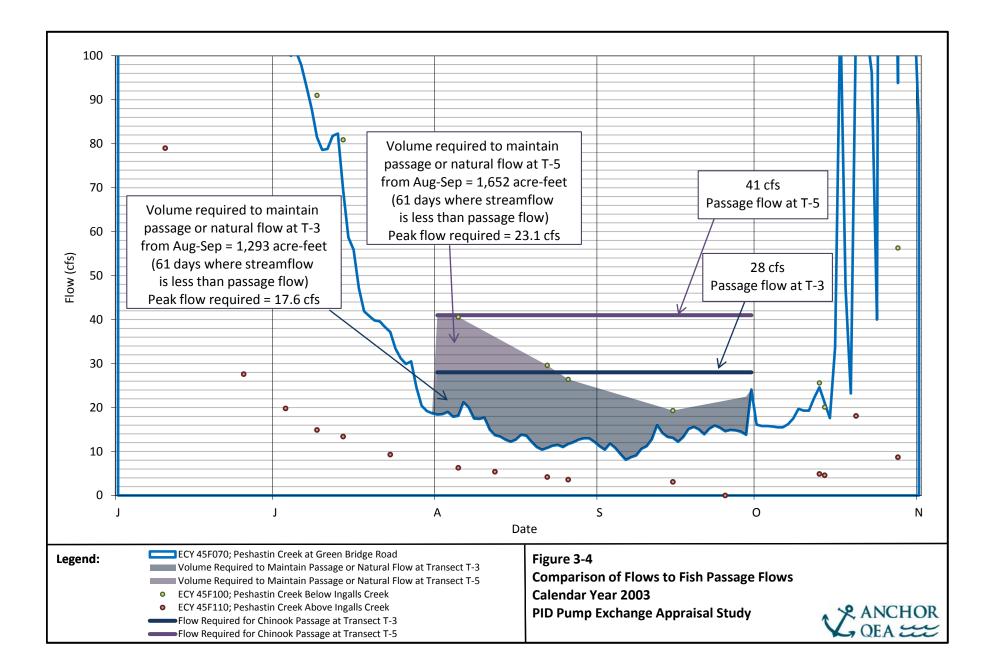
cfs. The average volume required would have been 1,538 acre-feet. Streamflows were less than the 41-cfs passage flow required at Transect T-5 for an average of 59 days during the late summer and early fall during the period of record. The average maximum additional flow that would have been needed to maintain a passage flow of 41 cfs or the natural flow downstream of Green Bridge Road during the period of record was 26 cfs. The average volume required would have been 1,995 acre-feet.

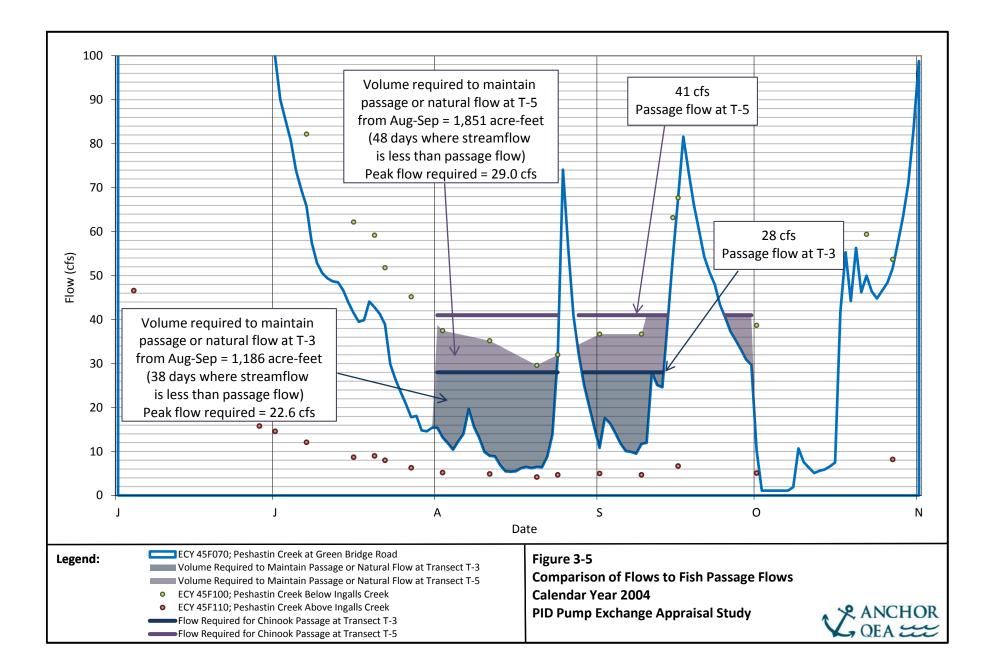
	Transect T-3 (Passage Flow=28 cfs)			Transect T-5 (Passage Flow=41 cfs)		
Year	Volume Needed (acre-feet)	Days Streamflow < Passage Flow	Maximum Additional Flow Needed (cfs)	Volume Needed (acre-feet)	Days Streamflow < Passage Flow	Maximum Additional Flow Needed (cfs)
2003	1,293	61	17.6	1,652	61	23.1
2004	1,186	38	22.6	1,851	48	29.0
2005	1,935	61	22.4	1,959	61	22.4
2006	1,633	59	18.4	2,125	61	28.1
2007	1,869	60	23.2	2,339	61	29.9
2008	1,313	56	18.1	2,041	61	25.2
AVERAGE	1,538	56	20.4	1,995	59	26.3

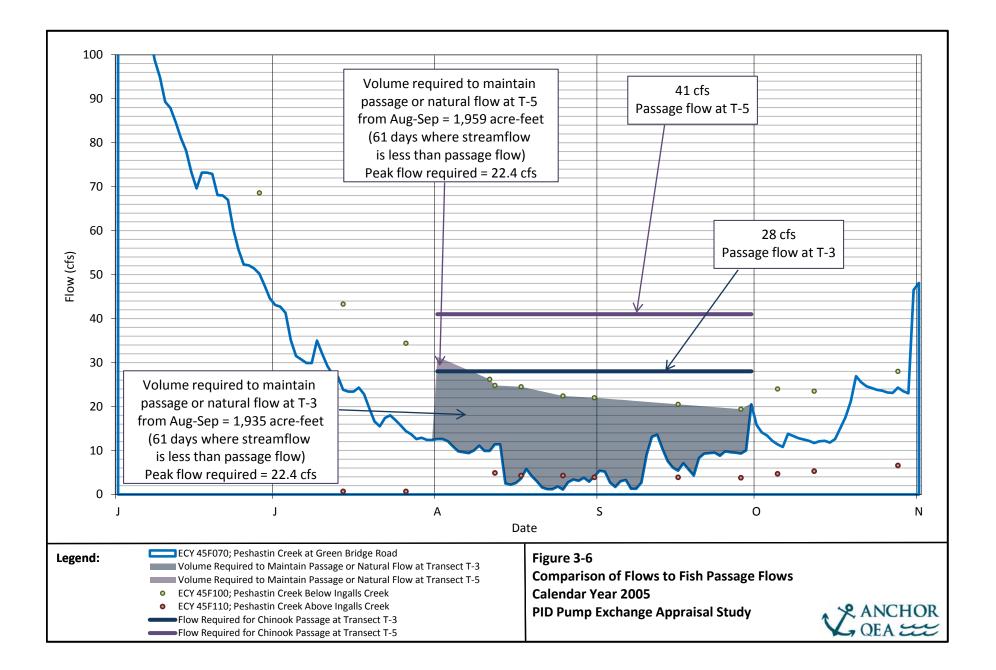
 Table 3-4

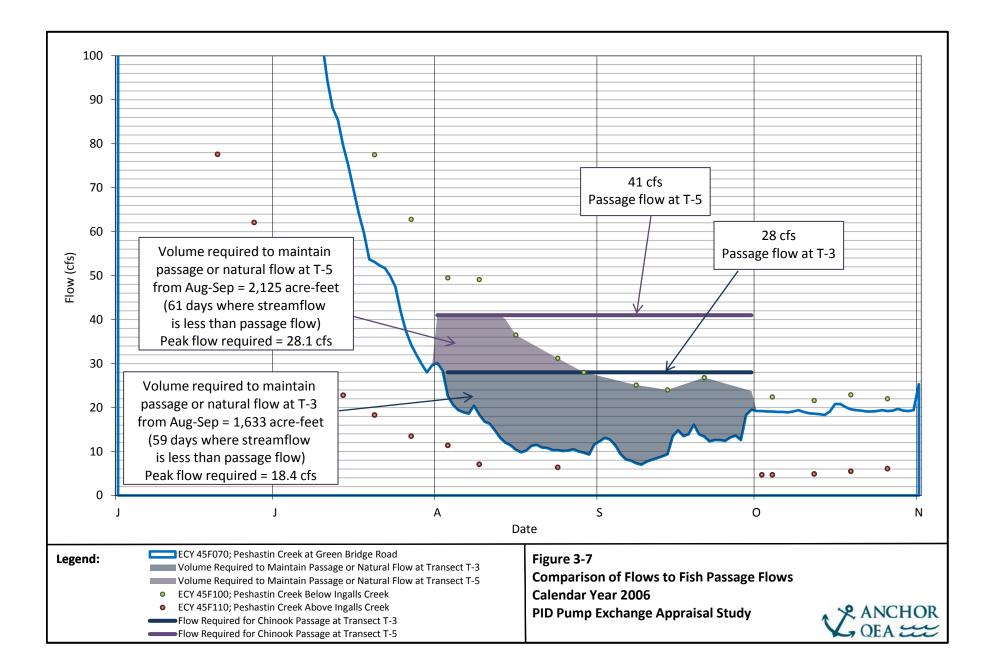
 Flows Needed to Maintain Chinook Salmon Passage Flow (August and September)

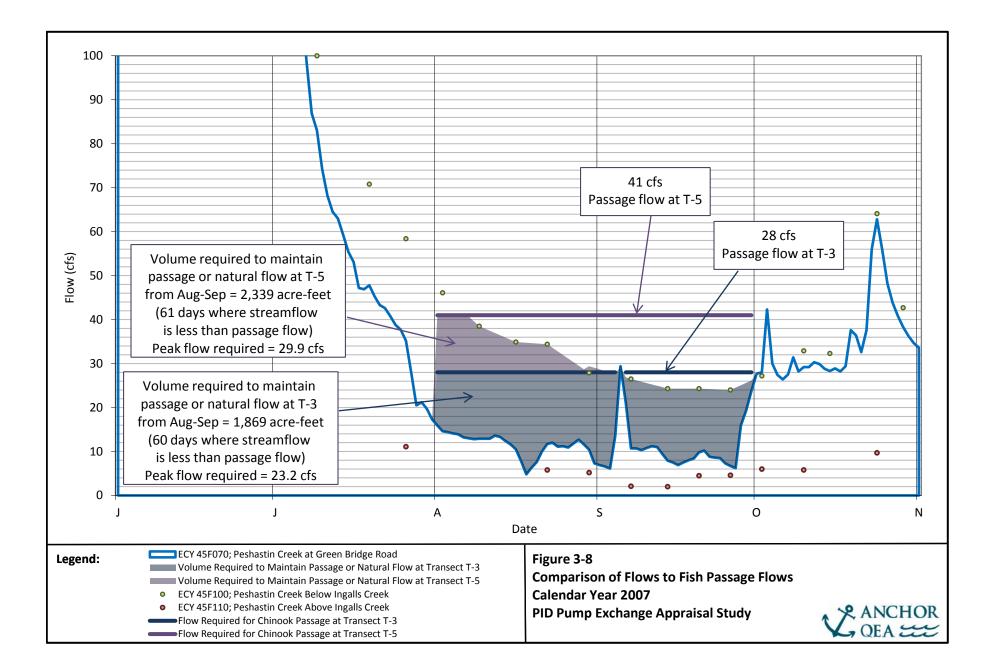
As a check, the diversion data illustrated in Figure 3-1 were compared to the volumes shown in Table 3-4. The average volume diverted by PID in August and September for the years data is available is approximately 1,700 acre-feet, which is consistent with the results shown in Table 3-4.

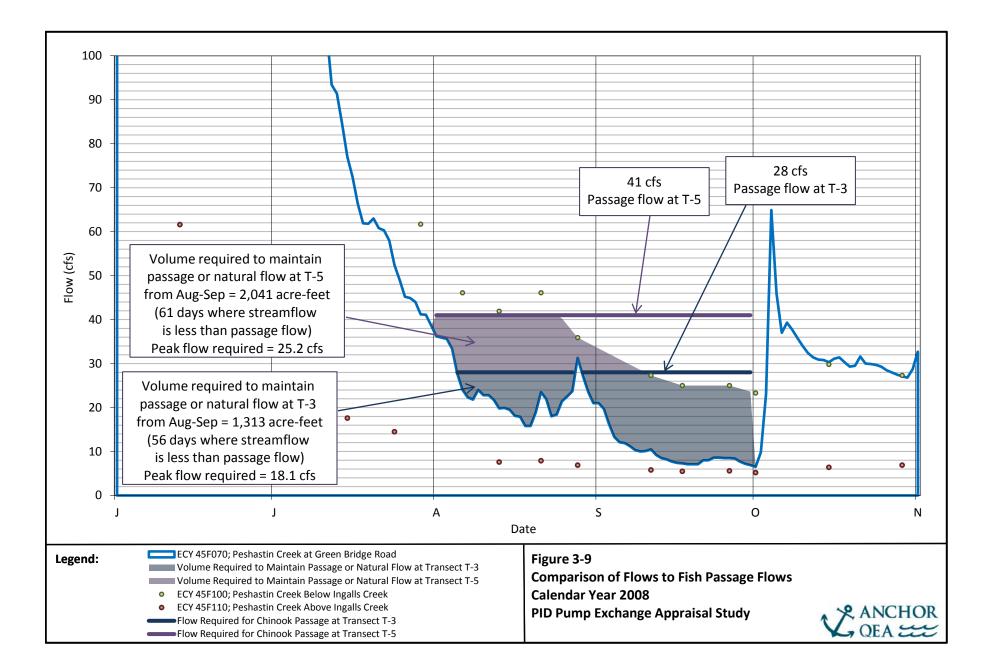












4 DESCRIPTION OF PRELIMINARY ALTERNATIVES

As part of an initial analysis of the pump exchange project completed as part of the *Campbell Creek Reservoir Feasibility Study* (Anchor QEA 2010), five alternatives were developed by identifying five potential pump station locations on the right bank of the Wenatchee River, with corresponding delivery pipeline alignments and delivery locations at the PID Canal. Those alternatives have been reviewed and refined as part of the work done for this study. This section provides a brief summary of each of the proposed alternatives and outlines potential alternate facility locations and alignments that may be considered. Photographs of the proposed pump station locations, pipeline alignments, and delivery locations associated with the alternatives are included in Appendix A. The major components of each alternative are shown on a map in Figure 4-1.

4.1 Alternative 1

Alternative 1 would include the following:

- A pump station located on the right bank of the Wenatchee River, just southwest (upstream) of U.S. Highway 2, approximately 7,250 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 16.5)
- A 1,240-foot delivery pipeline that would extend south and east through an existing orchard, and then south and west up a steep hillside to the PID Canal
- A delivery structure at the PID Canal approximately 19,560 feet downstream of the diversion at Peshastin Creek

The pump station would be located along a portion of the river with a relatively steep, high bank. The bank has been prone to erosion and movement, as evidenced by an existing wood structure that is propped up by wood supports over a portion of the bank that has eroded. The geomorphic analysis in Section 5 recommends locating the pump station adjacent to a portion of the river bank where bedrock is exposed. The pipeline alignment would cross private property and ascend a steep hill at an average slope of approximately 30 percent. The delivery structure would need to be designed to transition flow from the outlet of the pressurized delivery pipeline to an open, concrete-lined section of the PID Canal.

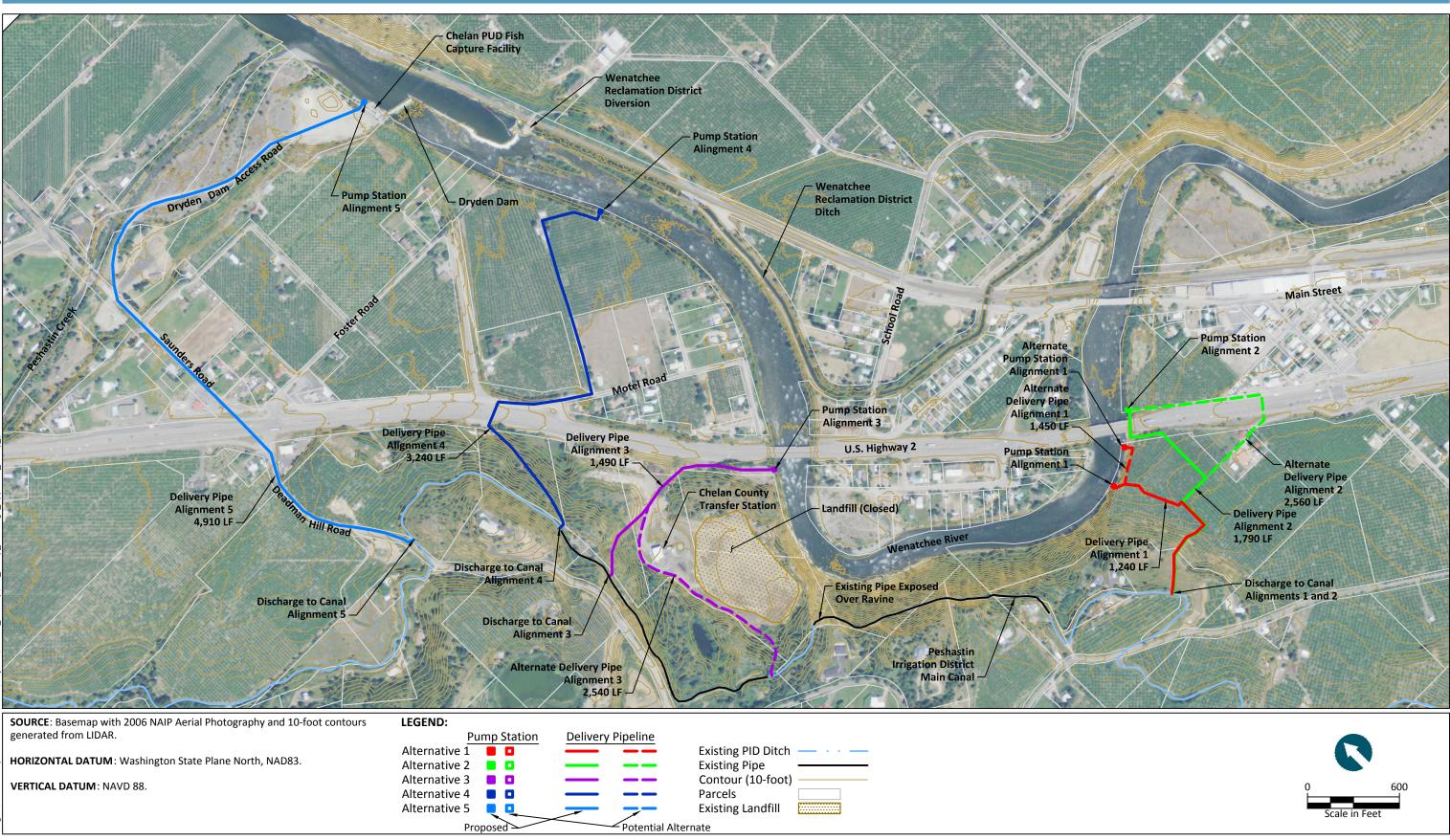




Figure 4-1

Preliminary Alternatives Peshastin Irrigation District Pump Exchange Appraisal Study Chelan County Natural Resources Department If the proposed pump station location shown will not work due to access or bank stability issues, an alternate pump station location could be considered closer to the U.S. Highway 2 bridge. A total of 1,450 feet of delivery pipe would be required if the pump station was located adjacent to the U.S. Highway 2 bridge.

4.2 Alternative 2

Alternative 2 would include the following:

- A pump station located on the right bank of the Wenatchee River, just north (downstream) of U.S. Highway 2, approximately 7,800 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 16.4)
- A 1,790-foot delivery pipeline that would extend south under the U.S. Highway 2 bridge, south and east through an existing orchard, and then south and west up a steep hillside to the PID Canal
- A delivery structure at the PID Canal approximately 19,560 feet downstream of the diversion at Peshastin Creek

The pump station would be located along a portion of the river with a relatively steep bank, adjacent to an existing orchard. The proposed pipeline alignment would cross WSDOT right-of-way adjacent to the existing U.S. Highway 2 bridge abutment, extend through private property, and ascend a steep hill at an average slope of approximately 30 percent. The delivery structure would need to be designed to transition flow from the outlet of the pressurized delivery pipeline to an open, concrete-lined section of the PID Canal.

If crossing U.S. Highway 2 under the existing bridge is not acceptable to WSDOT, an alternate alignment could be considered that would extend east from the pump station, along the edge of the U.S. Highway 2 right-of-way and cross the U.S. Highway 2 right-of-way via directional drilling, boring, or jacking at a location closer to the center of Dryden. This alternate alignment would require a total of 2,560 feet of delivery pipeline.

4.3 Alternative 3

Alternative 3 would include the following:

- A pump station located on the right bank of the Wenatchee River, just south (downstream) of U.S. Highway 2, approximately 4,650 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 17.0)
- A 1,490-foot delivery pipeline that would extend south and west, crossing the access road to the Dryden Transfer Station (former Dryden Landfill), and extending up a steep hill to the PID Canal
- A delivery structure at the PID Canal, which is in a welded-steel pipe through this area, approximately 14,720 feet downstream of the diversion at Peshastin Creek

The pump station would be located along a portion of the river with riparian foliage. The proposed pipeline alignment would extend west along the edge of the U.S. Highway 2 right-of-way, then up the hillside and across the access road to the Dryden Transfer Station. The alignment would ascend a steep hillside from the Dryden Transfer Station to the PID Canal at an average slope of approximately 24 percent. The delivery structure would need to be designed to transition flow from the outlet of the pressurized delivery pipeline to a gravity flow, welded-steel pipeline that comprises the PID Canal at the proposed delivery location.

An alternate alignment and delivery location could be considered for this alternative that would consist of extending the delivery pipeline around the west and south edge of the former Dryden Landfill, near the toe of the hillside above the landfill, and ascending a gentler slope to the Canal near the south end of the landfill. This alignment would require a total of 2,540 feet of delivery pipeline.

4.4 Alternative 4

Alternative 4 would include the following:

• A pump station located on the right bank of the Wenatchee River, adjacent to a private orchard, approximately 2,560 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 17.4)

- A 3,240-foot delivery pipeline that would extend south, through the private orchard to Motel Road, west on Motel Road to U.S. Highway 2, across U.S. Highway 2, and up a steep driveway to the PID Canal
- A delivery structure at the PID Canal, which transitions from an open, concrete-lined Canal to a welded-steel pipe through this area, approximately 14,240 feet downstream of the diversion at Peshastin Creek

The pump station would be located along a portion of the river with riparian foliage and a relatively steep, high bank. The proposed pipeline alignment would extend through a private orchard to public right-of-way; cross U.S. Highway 2 via directional drilling, boring, or jacking; and ascend a relatively steep driveway at an average slope of approximately 18 percent to the PID Canal. PID has indicated that it has an easement that allows access to the Canal using this driveway. The delivery structure could be designed to transition flow from the outlet of the pressurized delivery pipeline to an open, concrete-lined Canal or a gravity flow, welded-steel pipeline.

4.5 Alternative 5

Alternative 5 would include the following:

- A pump station located on the right bank of the Wenatchee River, just upstream of the Dryden Dam and Chelan PUD fish facility, approximately 700 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 17.8)
- A 4,910-foot delivery pipeline that would extend south and west, along the Dryden Dam access road, across U.S. Highway 2, and up Deadman Hill Road to the PID Canal
- A delivery structure or fittings at the PID Canal, which is an open concrete-lined Canal through this area, approximately 12,860 feet downstream of the diversion at Peshastin Creek

The pump station would draw water from the pool of water behind the Dryden Dam. The proposed pipeline alignment would extend along the Dryden Dam access road, cross U.S. Highway 2 via directional drilling, boring, or jacking; and extend up Deadman Hill Road to the PID Canal. The delivery structure could be designed to transition flow from the outlet of the pressurized delivery pipeline to an open, concrete-lined section of the PID Canal.

5 EVALUATION OF PRELIMINARY ALTERNATIVES

Each of the alternatives outlined in Section 4 was evaluated with the intent of identifying fatal flaws or challenges and providing enough background for stakeholders to narrow down the alternatives to one or two preferred alternatives. The evaluation included hydraulic analysis, site visits to observe and document existing conditions, a preliminary geomorphic analysis of the Wenatchee River channel, a geologic review of the delivery pipeline alignments, identification of property impacts, and environmental review and permitting fatal flaw analysis.

5.1 Hydraulic Analysis

Hydraulic analyses were completed to determine the preliminary size of pumps and pipe needed to deliver flows from the Wenatchee River to the PID Canal for each alternative. The analyses were completed for pump station flow rates of 10, 20, and 40 cfs, as outlined in Table 2-2. The results of the analyses are included in Appendix C. A spreadsheet analysis was used to estimate losses through the system, from the pump to the delivery at the Canal. A system curve was developed for each alternative that shows the estimated total dynamic head (TDH), or pumping head, required for pumping at different flow rates.

The Hazen-William formula was used to estimate friction losses through the system. The hydraulic analysis assumed that pump station manifold piping would be steel pipe with a Hazen-Williams coefficient of 110 and that the discharge piping would be plastic (HDPE or PVC) with a Hazen-Williams coefficient of 130. Minor losses were also estimated to account for losses through bends, valves, pipe entrances, pipe exits, and other fittings. The results of the hydraulic analysis are summarized in Table 5-1.

Design Parameter	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Pump Sump Elevation (feet)	923	922	935	953	972
Discharge Elevation (feet)	1,146	1,146	1,152	1,153	1,158
Elevation Gain (feet)	223	224	217	200	186
Delivery Pipe ¹ Length (feet)	1,240	1,790	1,490	3,240	4,910
Delivery Pipe ¹ Size (inches)	·				
At 10 cfs P.S. Capacity	20	20	20	20	20
At 20 cfs P.S. Capacity	30	30	30	30	30
At 40 cfs P.S. Capacity	36	36	36	36	36
Total Headloss ² (feet)					
At 10 cfs P.S. Capacity	21	23	22	28	33
At 20 cfs P.S. Capacity	17	18	18	21	24
At 40 cfs P.S. Capacity	27	28	27	32	36
Pumping Head, TDH (feet)					
At 10 cfs P.S. Capacity	244	247	239	228	219
At 20 cfs P.S. Capacity	240	242	235	221	210
At 40 cfs P.S. Capacity	250	252	244	232	222
Number of Pumps					
At 10 cfs P.S. Capacity	2	2	2	2	2
At 20 cfs P.S. Capacity	3	3	3	3	3
At 40 cfs P.S. Capacity	3	3	3	3	3
Flow Rate per Pump (cfs)					
At 10 cfs P.S. Capacity	5.00	5.00	5.00	5.00	5.00
At 20 cfs P.S. Capacity	6.67	6.67	6.67	6.67	6.67
At 40 cfs P.S. Capacity	13.33	13.33	13.33	13.33	13.33

Table 5-1Summary of Hydraulic Analysis Results

Notes:

1. Delivery Pipe includes transmission pipeline from the pump station to the delivery at the PID Canal.

2. Total headloss includes both friction and minor losses through the pump station and delivery pipeline.

cfs = cubic feet per second

TDH = Total Dynamic Head, or Pumping Head

The results indicate the following:

• For a design flow rate of 10 cfs, a 20-inch delivery pipeline would be required. Two pumps would be required, each with a capacity of 5.0 cfs. The pumps would be

required to deliver that flow rate against a TDH ranging from 219 feet (Alternative 5) to 247 feet (Alternative 2).

- For a design flow rate of 20 cfs, a 30-inch delivery pipeline would be required. Three pumps would be required, each with a capacity of 6.67 cfs. The pumps would be required to deliver that flow rate against a TDH ranging from 210 feet (Alternative 5) to 242 feet (Alternative 2).
- For a design flow rate of 40 cfs, a 36-inch delivery pipeline would be required. Three pumps would be required, each with a capacity of 13.33 cfs. The pumps would be required to deliver that flow rate against a TDH ranging from 222 feet (Alternative 5) to 252 feet (Alternative 2).

The pump station facility would include a fish screen consistent with NMFS and WDFW guidelines. Different types of screens are available for diversions and pumping that meet NMFS and WDFW guidelines, including fixed-plate screens, cylindrical end-of-pipe screens, rotating drum screens, vertical traveling screens, horizontal screens, and infiltration galleries. An inclined fixed-plate screen was identified as the most likely screening option for a pump station diverting water from the bank of the Wenatchee River. Table 5-2 provides a summary of the design parameters and sizes required for a fixed plate screen for the range of design flows that were evaluated as part of this analysis. Fish screen calculations are included in Appendix D.

Maximum Screen Flow (cfs)	Maximum Approach Velocity (fps)	Effective Screen Area (ft ²)	Screen Area with FOS (ft ²)	Proposed Screen Height (ft)	Minimum Required Screen Length (ft)
10	0.4	25.00	31.75	4.0	7.9
20	0.4	50.00	63.50	4.0	15.9
40	0.4	100.00	127.00	4.0	31.8

Table 5-2 Fish Screen Sizing – Inclined Flat-Plate Screen

Notes:

cfs = cubic feet per second FOS = Factor of Safety fps = feet per second

 ft^2 = square feet

An inclined fixed-plate screen would consist of a flat screen supported by the sump structure, inclined at an angle to generally match the slope of the riverbank. Other types of screens considered included a cylindrical end-of-pipe screen and a rotating drum screen. At the flow rates that were used for the analysis, a cylindrical end-of-pipe screen would have to be very long or have a large diameter and would likely not be practical because of the slope of the river, its shallow depth, and coarse substrate. A rotating drum screen could potentially work in this application if sited appropriately to provide the proper submergence and flow conditions.

In order to minimize maintenance, ensure proper performance, and secure approval by WDFW, the screen would need to be self-cleaning. Typical self-cleaning options for an inclined, fixed plate screen would include a mechanical air-burst system or mechanical brush. Another similar self-cleaning screen option would include an inclined traveling water screen that would rotate on a conveyor. Debris would be lifted out of the water and removed with internal water jets, a brush, or other self-cleaning mechanism.

5.2 Geomorphic Analysis of Wenatchee River Channel

A preliminary geomorphic analysis of the Wenatchee River channel through the study area was completed by visiting the study area, documenting conditions at each of the proposed pump station sites, and reviewing aerial photographs and other available information. A summary of the geomorphic analysis for each of the proposed diversion structure and pump station sites is summarized below. Photographs of the river channel at the proposed pump station sites are included in Appendix A.

5.2.1 Alternative 1

The diversion structure and pump station for Alternative 1 would be placed just upstream of the easternmost U.S. Highway 2 bridge in the study area. Although a pump station location was initially reviewed adjacent to the bridge, the recommended pump station location would be further upstream, just downstream of an exposed bedrock knob in the river. Bank materials at the proposed pump station location are composed of native small to large cobble with some large rock at approximately a 1:1 slope. A significant portion of the main flow is along the right bank and interacts with the bedrock creating and maintaining a large, deep

pool. The bedrock knob would help protect the diversion structure during high flows and the deep pool would provide assurance that diversions can occur during low flow time periods. Because of the location on the outside of the bend and the proximity to the bedrock forced pool, it is likely that future channel maintenance at this location would be minimal.

5.2.2 Alternative 2

The diversion and pump station for Alternative 2 would be located just downstream of the easternmost U.S. Highway 2 bridge in the study area. The right bank at this location is primarily composed of large rounded rock at an approximately 1:1 grade. These rocks may be derived from the channel or bank, or may have been imported to provide bank stability in associated with bridge construction or maintenance. The diversion structure would be sheltered from high flows by the bridge. The main flow of the channel is located away from the right bank and some manipulation to the channel bed leading to the diversion site would need to occur to enable water diversion during low flow conditions. During high flows, gravel bed materials would likely be in active transport adjacent to the diversion structure and this dynamic should be considered in design planning. It is likely that some maintenance within the mainstem channel would be necessary to maintain the ability to extract water.

5.2.3 Alternative 3

The diversion and pump station for Alternative 3 would be placed just downstream of the westernmost U.S. Highway 2 bridge in the study area. The right bank at this location is primarily composed of large rounded rock at an approximately 2:1 grade. These rocks are likely native bank materials. The diversion structure would be sheltered from high flows by the bridge. Because of the bank condition, the diversion structure would likely need to be constructed on a greater angle than a structure built at Sites 1 or 2. The main flow of the channel is located near the middle of the river and some manipulation to the channel bed leading to the diversion site would need to occur to enable water diversion during low flow conditions. Because this site is located along the inside of a bend, it is likely that gravel materials will deposit along this bank in the future. Therefore, it is expected that the river channel leading to the diversion would require regular maintenance to provide adequate flow to the diversion.

5.2.4 Alternative 4

The diversion and pump station for Alternative 4 would be located on the right bank of the river channel approximately 1,800 feet downstream of the Dryden Dam. The right bank at this location is primarily composed of large rounded rock at an approximately 2:1 grade. These rocks are likely native bank materials. The diversion structure could be constructed into the bank; however, shelter from high flows would be minimal. Because of the bank condition, the diversion structure would likely need to be constructed on a greater angle than a structure built at Sites 1 or 2. The main flow of the channel is located near the far bank of the river and some manipulation to the channel bed leading to the diversion site would need to occur to enable water diversion during low flow conditions. Because of the distance between the right bank and main flow in the channel at this location, it is likely that gravel materials will deposit along this bank in the future. Future maintenance at this site is uncertain. It is likely that during lower water years deposition would occur in the river adjacent to the diversion site. During high water years, the diversion structure could experience scour if not built carefully into the bank.

5.2.5 Alternative 5

The diversion and pump station for Alternative 5 would be located just upstream of Chelan PUD's fish rearing facilities on the right bank of the river channel upstream of the Dryden Dam. Geomorphic conditions at this site are largely muted because of the presence of the Dryden Dam. Bank materials are largely composed of fill. The river is pooled up behind the Dryden Dam allowing for a high certainty of diversion during low flow periods. It is not expected that manipulation of the river channel would be required to divert water at this location. Chelan PUD has indicated that they have had to remove material from behind the dam. However, future maintenance of the channel directly related to a diversion structure at this location would be unlikely.

5.2.6 Summary

Based on a preliminary review of geomorphic conditions in the study area, pump station locations proposed for Alternatives 1 and 5 appear to be the best suited for diversion placement of the sites evaluated. Both of these sites likely provide a high certainty of diversion during low flows without manipulating the river and future maintenance will likely be minimal compared to the other alternative sites evaluated.

The proposed pump station location for Alternative 2 is feasible, but would likely require greater channel maintenance efforts to maintain diversions during low flows. The proposed pump station locations for Alternatives 3 and 4 are not well suited for a diversion structure because of the likely level of instream manipulation that would be required to successfully extract water during low flow periods and the channel maintenance that would likely be required to allow diversion in the future.

5.3 Geologic Review of Pipeline Alignments

The primary geologic hazard associated with each alternative alignment is the potential for landslides. Once a preferred alignment is selected, a more in-depth site investigation including soil borings and/or test pits should be conducted along with a slope stability analysis. A brief discussion of geological units expected to be encountered, slope stability risk, and geotechnical constructability issues relating to each of the proposed alignments is presented below. This discussion is based on review of the available geologic and subsurface information, as well as a site visit conducted on July 29, 2011. Geology and soils maps for the study are included in Appendix E.

5.3.1 Alternative 1

The pipeline alignment for Alternative 1 would begin along the south side of U.S. Highway 2 adjacent to the existing concrete bridge structure crossing the Wenatchee River. The pump station for this alignment would be located on the outside of a 90-degree river bend and would be located at a steep portion of the river bank that shows signs of severe erosion resulting from high velocity flows and energy causing over-cutting of the bank soils. Excavation for the pump station would encounter the Coarse Grained Alluvium unit and large boulders and cobbles are expected to be encountered. The bank extends about 10 to 15 feet above the existing water surface and some shoring could be required to construct the pump station at this location.

From the pump station, the pipe alignment would extend along a moderately steep (5 to 15 percent) slope through an existing orchard field before turning at a 90 degree bend where it would continue directly up a steep slope (greater than 25 percent) to reach an open portion of the canal. The alignment through the moderately steep and steep portions would likely encounter the Fine Grained Alluvium and Sandstone units. A landslide area is mapped as being present along the west side of this alignment; however, no evidence of recent slide activity was observed along the hillside portion of this alignment and a large portion of the hillside appears to have been terraced in the past for agricultural purposes. A stand of timber along the south side of the hillside alignment showed no indication of tilting or distress.

Slope stability risks associated with this alignment are low to moderate. Construction of this alignment would be more conventional since a highway crossing is not required.

5.3.2 Alternative 2

The pipeline alignment for Alternative 2 would be similar to Alternative 1 but would begin along the north side of U.S. Highway 2 adjacent to the existing concrete bridge structure crossing the Wenatchee River. The pump station for this alignment would be located a few hundred feet downstream from the Alignment 1 pump station at a bank location less prone to erosion and more protected from direct stream flow energy by the bridge structure. Excavation for the pump station would encounter the Coarse Grained Alluvium unit and large boulders and cobbles are expected to be encountered. The bank extends about 10 to 15 feet above the existing water surface and some shoring could be required to construct the pump station and possibly to protect the bridge abutment fill slope.

From the pump station, the pipe alignment would either extend under the bridge and follow Alternative 1, or extend along the east side of U.S. Highway 2 for about 500 feet before crossing underneath the highway and across the orchard field where it would then match Alternative 1. Jacking and boring would be required to install the pipe beneath U.S. Highway 2. It is likely that the jack and bore section would encounter the Coarse Grained Alluvium unit resulting in a challenging installation. For this reason, it would likely be more feasible and cost effective to extend the pipe under the bridge, if allowed by WSDOT. This would require installation of the pipe in low overhead conditions within the abutment fill slope while working under the bridge deck structure. While challenging, this method would likely be feasible provided the bridge abutment foundations are not disturbed or undermined. WSDOT would need to be involved in the design and approval for the pipe section under the bridge or for the jack and bore section under U.S. Highway 2.

Slope stability risks associated with this alignment are low to moderate. Construction of a pipeline along this alignment would be moderately difficult due to the U.S. Highway 2 crossing.

5.3.3 Alternative 3

The pipeline alignment for Alternative 3 would begin along the south side of U.S. Highway 2 adjacent to the westernmost bridge crossing of the Wenatchee River in the study area. The pump station location is about 8 to 10 feet above the water along a steep section of the bank. Excavation for the pump station would encounter the Coarse Grained Alluvium unit and large boulders and cobbles are expected to be encountered.

The pipe alignment would run west along U.S. Highway 2 for about 500 feet. Excavation along this section will likely encounter the Coarse Grained Alluvium unit as well. The pipe alignment would then turn up a short steep section of slope (greater than 25 percent) where exposed surface soils appear to consist of a weathered component of the Sandstone unit. Evidence of recent or past slides was not observed along this shorter section of slope. The short slope crests along the north side of the closed Dryden Landfill and would cross the Dryden Dump Road near the existing Dryden Transfer Station. The ground surface surrounding the landfill is relatively flat and steep slopes border the west and south sides. These steep slopes show signs of recent slope instability. Slope instability is evidenced by undermining of the existing PID Canal pipe along a 20 feet section where it passes along the south side of the landfill. Significant movement was also observed along a section of Deadman Hill Road where it passes along the west side of the landfill. It appears that this section of Deadman Hill Road was constructed with a deep fill section that may be contributing to instability along this steep slope section. Slope stability risks associated with this alignment are potentially high. Careful routing of the pipeline alignment, along with a detailed slope stability analysis would be required.

5.3.4 Alternative 4

The pipeline alignment for Alternative 4 would begin along a section of the river running along an existing orchard located north of U.S. Highway 2. Excavation for the pump station would encounter the Coarse Grained Alluvium unit and large boulders and cobbles are expected to be encountered. The pipe alignment would follow an existing gravel access road across the orchard until reaching U.S. Highway 2. The alignment from the pump station to U.S. Highway 2 is expected to encounter the Fine Grained Alluvium unit.

Jacking and boring would be required to install the pipe beneath U.S. Highway 2. It is likely that the jack and bore section would encounter the Fine Grained Alluvium unit or fill material previously placed during construction of U.S. Highway 2. It is anticipated that jacking and boring at this location would be easier than the crossing that would be required for the alternative pipeline alignment for Alternative 2.

The pipeline would follow an existing un-paved driveway up a moderately steep (15 to 25 percent) portion of the hillside to a point where the proposed pipeline would tie into an existing piped section of the PID Canal. Evidence of slope instability was not observed along the section of driveway, but the slope is mapped as a Landslide unit.

Slope stability risks associated with this alignment are low to moderate. Construction of this alignment would be moderately difficult due to the U.S. Highway 2 crossing and the limited access along the steep driveway leading to the discharge point.

5.3.5 Alternative 5

The final proposed pipeline alignment, for Alternative 5, would begin just upstream of Dryden Dam and downstream of the mouth of Peshastin Creek. The river bank is near the water surface at this location and would allow for easier installation of the pump station. As with the other alignments, excavation for the pump station would encounter the Coarse Grained Alluvium unit and large boulders and cobbles are expected to be encountered.

The pipeline alignment would follow the existing gravel access to the Dryden Dam and public right-of-way along Saunders Road to U.S. Highway 2. The existing gravel access road between the river and Saunders Road follows the former Peshastin Creek channel and the Coarse Grained Alluvium unit would likely be encountered. The Saunders Road section of the alignment rises out of the former creek channel and the Fine Grained Alluvium unit would likely be encountered. Jacking and boring would be required to install the pipeline under U.S. Highway 2. It is likely that the jack and bore section would encounter the Fine Grained Alluvium unit or fill material previously placed during construction of U.S. Highway 2.

From U.S. Highway 2, the pipeline would follow Deadman Hill Road to an open section of the PID Canal. This section of the alignment is likely to encounter the Fine Grained Alluvium unit overlying the Sandstone unit. The thickness of the Fine Grained Alluvium is expected to vary between U.S. Highway 2 and the discharge point. The alignment does not extend through an area mapped as a Landslide unit, but an area to the south is mapped as being part of a Landslide unit.

Slope stability risks associated with this alignment are low. Construction of a pipeline along this alignment would be moderately difficult due to the U.S. Highway 2 crossing and the limited right-of-way width along Deadman Hill Road.

5.4 Property Ownership and Right-of-Way Investigation

A review of property ownership and rights-of-way in the study area was conducted to identify potential impacts to properties and state or Chelan County rights-of-way that would result from construction of pump station, pipeline, and delivery facilities. Parcels that could potentially be impacted by each alternative are highlighted on Figure 5-1. Aerial photography, photographs from site visits, and Chelan County parcel data were used to identify impacted properties and summarize the potential impacts.

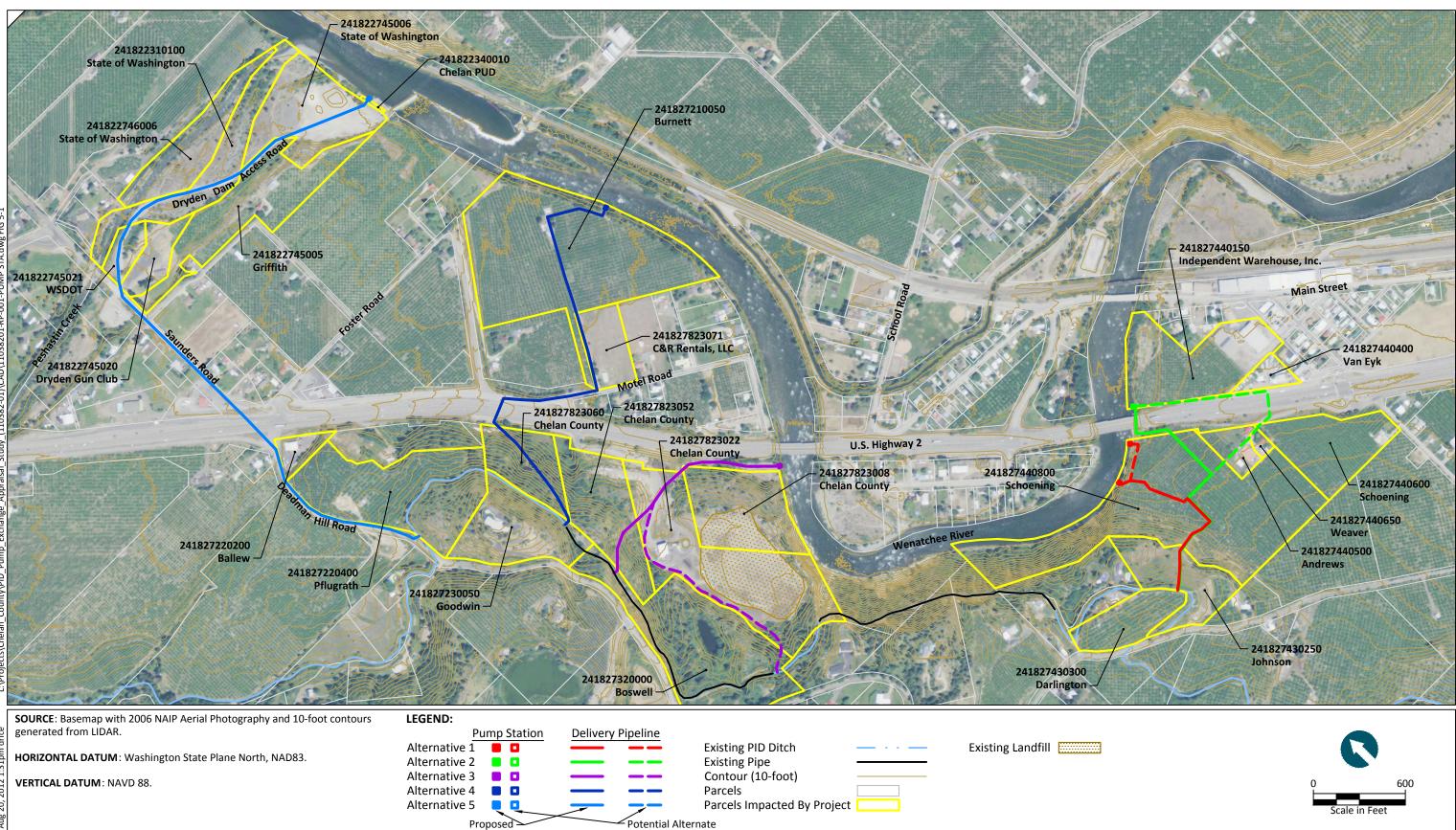




Figure 5-1

Parcel and Right-of-Way Impacts Peshastin Irrigation District Pump Exchange Appraisal Study Chelan County Natural Resources Department

5.4.1 Alternative 1

Construction of the facilities included as part of Alternative 1 would directly impact two private properties and could potentially impact a third property located adjacent to the PID Canal at the delivery location. Table 5-3 lists a summary of the parcels that would be impacted by or are adjacent to Alternative 1.

Parcel	Owner	Address	Impacted/Adjacent	
244027420250	Frederick Johnson	PO Box 97	Immonted	
241827430250		Dryden, WA 98821	Impacted	
244027440000	Don Schoening TRT	6223 Pine Flats Loop	Immonted	
241827440800		Cashmere, WA 98815	Impacted	
244027420200	Loren Darlington	PO Box 45	Adjacant	
241827430300		Dryden, WA 98821	Adjacent	

Table 5-3 Properties Impacted by or Adjacent to Alternative 1

The pump station would be constructed along the boundary of Parcel 241827440800. The proposed delivery pipeline alignment would cross an existing orchard through Parcel 241827440800. The pipeline would be designed to minimize impacts to the orchard as much as possible. However, construction of a large diameter pipeline would require a relatively wide trench and impacts to trees may not be avoidable. The proposed delivery pipeline would ascend a hillside through Parcel 241827430250 to the PID Canal. The alignment through this parcel appears to consist of open pasture. Construction may require access through the ditch easement along the boundary of Parcel 241827430300, which is privately owned, and construction of a delivery structure at the ditch could potentially have an impact on the parcel.

5.4.2 Alternative 2

Construction of the facilities included as part of Alternative 2 would directly impact three private properties and could potentially impact a fourth property located adjacent to the PID Canal at the delivery location. Additional parcels would be impacted if the alternate alignment shown in Figure 5-1 was selected. Table 5-4 lists a summary of the parcels that would be impacted by or are adjacent to Alternative 2.

Parcel	Owner	Address	Impacted/Adjacent	
244027420250	Frederick Johnson	PO Box 97	Impostod	
241827430250		Dryden, WA 98821	Impacted	
244027440450		PO Box 60	Imported	
241827440150	Independent Warehouse Inc.	Dryden, WA 98821	Impacted	
241027440500	Frank Andrews	PO Box 451	Impacted	
241827440500	Frank Andrews	Dryden, WA 98821	(Alternate Alignment Only)	
241027440000	Donald Schoening	6223 Pine Flats Loop	Impacted	
241827440600		Cashmere, WA 98815	(Alternate Alignment Only)	
244027440000	Don Schoening TRT	6223 Pine Flats Loop	Impacted	
241827440800		Cashmere, WA 98815	Impacted	
244027420200	Loron Darlington	PO Box 45	Adiacont	
241827430300	Loren Darlington	Dryden, WA 98821	Adjacent	
		900 Front St Ste D	Adiacont	
241827440400	Jeffrey Van Eyk et al.	Leavenworth, WA	Adjacent	
		98826	(Alternate Alignment Only)	
241027440650		136 N Chelan Ave	Adjacent	
241827440650	Andrew and Michelle Weaver	Wenatchee, WA 98801	(Alternate Alignment Only)	

Table 5-4Properties Impacted by or Adjacent to Alternative 2

The pump station would be constructed adjacent along the boundary of Parcel 241827440150. Construction of the pump station and access to the pump station could potentially impact the existing orchard on Parcel 241827440150. To the extent possible, the pump station and access road would be constructed along the edge of the orchard to minimize impact to existing trees. The proposed delivery pipeline alignment would then cross WSDOT right-of-way for U.S. Highway 2 under an existing bridge adjacent to the bridge abutment. WSDOT review and a right-of-way permit would be required. From the bridge to the PID Canal, property impacts would be similar to those listed for Alternative 1, including impacts to Parcels 241827440800, 241827430250, and 241827430300.

If the alternate alignment for the delivery pipe was selected, additional parcels would be impacted. Construction of the proposed delivery pipeline along the alternative alignment would extend along the boundary of Parcels 241827440150 and 241827440400 with U.S. Highway 2 right-of-way. The pipe would cross the WSDOT right-of-way at U.S. Highway 2 via directional drilling, jacking, or boring and would require a right-of-way permit. The

delivery pipe would also impact the driveway in Parcel 241827440500 and would extend across or near the boundaries of Parcels 241827440650 and 241827440600.

5.4.3 Alternative 3

Construction of the facilities included as part of Alternative 3 would directly impact three Chelan County properties and could potentially impact a private property located adjacent to the PID Canal at the delivery location. Table 5-5 lists a summary of the parcels that would be impacted by or are adjacent to Alternative 3.

Parcel	Owner	Address	Impacted/Adjacent	
241027220000	Bradford Boswell	4182 1/2 Mississippi St	Impacted	
241827320000		San Diego, CA 92104	Impacted	
241827823022	Chelan County	400 Douglas St	Impacted	
	Commissioner's Office	Wenatchee, WA 98801	Impacted	
241827823052	Chelan County	400 Douglas St	Impacted	
	Commissioner's Office	Wenatchee, WA 98801		
241827823008	Chelan County	400 Douglas St	Adjacont	
	Commissioner's Office	Wenatchee, WA 98801	Adjacent	

Table 5-5Properties Impacted by or Adjacent to Alternative 3

The pump station would be constructed near the northeast corner of Parcel 241827823008, owned by Chelan County. The proposed pipeline alignment would extend along the edge of the WSDOT right-of-way at U.S. Highway 2, and would require a right-of-way permit. The pipeline would also cross two additional parcels owned by Chelan County, which comprise the existing Dryden Transfer Station (former Dryden Landfill). The pipeline alignment would likely impact some forested slopes above the landfill and would cross the road to the existing transfer station. The delivery to the PID Canal would be adjacent to Parcel 241827320000, which is privately owned, and construction of the delivery structure could potentially impact that parcel.

5.4.4 Alternative 4

Construction of the facilities included as part of Alternative 4 would directly impact three private properties and could potentially impact a fourth private property located adjacent to

the PID Canal at the delivery location. Table 5-6 lists a summary of the parcels that would be impacted by or are adjacent to Alternative 4.

Parcel	Owner	Address	Impacted/Adjacent	
241827210050	David & Cindy Burnett	1511 Anton St	Impacted	
241827210050		Wenatchee, WA 98801	impacted	
241827823060	Chelan County	400 Douglas St	Imported	
	Commissioner's Office	Wenatchee, WA 98801	Impacted	
244027022074	C&R Rentals, LLC	PO Box 772	Impacted	
241827823071		Cashmere, WA 98815	Impacted	
241827230050	Alice Goodwin TRT	PO Box 74	Adjacent	
		Dryden, WA 98821	Adjacent	

Table 5-6Properties Impacted by or Adjacent to Alternative 4

The pump station would be constructed near the boundary of Parcel 241827210050. The proposed delivery pipeline alignment and access to the pump station would impact a private orchard and driveway in Parcel 241827210050. The delivery pipeline would extend along the private driveway adjacent to Parcel 241827823071. The delivery pipeline would extend west in Motel Road and would cross U.S. Highway 2 via directional drilling, jacking, or boring. County and WSDOT right-of-way permits would be required for construction in road rights-of-way. On the south side of U.S. Highway 2, the pipeline alignment would follow an existing gravel driveway up a steep hillside to the PID Canal. The driveway is located on Parcel 241827823060, which is owned by Chelan County. PID has indicated that they may have an existing easement for accessing the canal via this existing driveway. The delivery to the PID Canal would be adjacent to Parcel 241827230050, which is privately owned, and construction of the delivery structure could potentially impact that parcel.

5.4.5 Alternative 5

Construction of the facilities included as part of Alternative 5 would directly impact six properties and could potentially impact three other properties located adjacent to the delivery pipeline. Table 5-7 lists a summary of the parcels that would be impacted by or are adjacent to Alternative 5.

Parcel	Owner	Address	Impacted/Adjacent	
244022240400	Washington State	PO Box 47014	Impacted	
241822310100		Olympia, WA 98504	Impacted	
244022240040		PO Box 1231	Imposted	
241822340010	PUD No 1 of Chelan County	Wenatchee, WA 98807	Impacted	
244022745006	Washington State	PO Box 47014	Imported	
241822745006	Washington State	Olympia, WA 98504	Impacted	
244022745024	Washington State DOT	PO Box 98	Impacted	
241822745021		Wenatchee, WA 98807	Impacted	
244027220400	Robert Pflugrath	PO Box 434	Impacted	
241827220400		Peshastin, WA 98847		
241022746006	Washington State	PO Box 47014	Adjacont	
241822746006	Washington State	Olympia, WA 98504	Adjacent	
241022745005	Korry Criffith	410 S Division	Adjacont	
241822745005	Kerry Griffith	Cashmere, WA 98815	Adjacent	
241922745020	Dryden Gun Club	PO Box 85	Adjacent	
241822745020		Dryden, WA 98821	Aujacent	
241927220200	Stephen Ballew	PO Box 777	Adjacent	
241827220200		Leavenworth, WA 98826	Aujacent	

Table 5-7Properties Impacted by or Adjacent to Alternative 5

The pump station for Alternative 5 would be constructed adjacent to the Chelan PUD fish collection facility at the Dryden Dam. Access to the pump station and construction of the delivery pipeline would occur in the existing access road to the dam. The pump station and delivery pipeline alignment would impact one parcel owned by Chelan PUD (241822340010) and two parcels owned by the State of Washington (241822745006 and 241822745021).

Recent discussions with representatives from the Yakama Nation indicate that they are targeting this site for a new fish rearing facility. Their proposed project would include a year-round withdrawal of 7 cfs from a diversion facility located adjacent to the Chelan PUD fish collection facility. The hatchery facility would also require groundwater withdrawals to deal with potential icing issues. The Yakama Nation has indicated a willingness to consider a joint diversion facility, which could potentially reduce costs for both the fish rearing facility and a new PID pump station. A potential concern that may need to be addressed is that the combined diversion would create a false attraction to migrating adult fish. In addition, the Yakama Nation has studied the site in some detail and has indicated that there is lead contamination on the property owned by WSDOT (Parcel 241822745006). There would likely be some cleanup costs associated with a new project at this site.

The Yakama Nation also indicated that WDFW has expressed interest in purchasing property along the shoreline of the Wenatchee River, immediately upstream of the Chelan PUD facility, to improve recreational access for portaging of whitewater rafts. Establishing a diversion facility at the site would require additional consultation and coordination with both the Yakama Nation and WDFW.

The existing access road and proposed delivery pipeline alignment would also extend along the boundaries of Parcels 241822310100, 241822745005, and 241822745020. The proposed delivery pipeline alignment would extend south in Saunders Road and would cross U.S. Highway 2 via directional drilling, jacking, or boring. County and WSDOT right-of-way permits would be required for construction in road rights-of-way. On the south side of U.S. Highway 2, the pipeline alignment would follow Deadman Hill Road to the PID Canal. The delivery pipeline alignment proposed for Alternative 5 is almost entirely within gravel or paved roadways or access roads. Chelan County is in the process of paving Deadman Hill Road. Impacts to the newly paved roadway would need to be considered and reviewed with Chelan County.

5.5 Environmental Review and Permitting Fatal Flaw Analysis

Using existing information and professional knowledge of the study area, a preliminary review of environmental resources and permitting requirements was conducted to identify potential impacts or requirement that could be considered "fatal flaws" associated with the pump station alternatives. The analysis included consideration of regulatory requirements associated with constructing and operating a pump station and fish screen along the Wenatchee River at suitable locations, along with construction of water delivery pipelines from the pump station to the PID Canal. Five site alternatives were reviewed (Anchor QEA 2010) and analyzed with specific emphasis on any regulatory issues that would inhibit the project from obtaining needed permits and approvals.

5.5.1 Natural Resources

Natural resources are naturally occurring environmental features that may have material and/or aesthetic and recreational value. They are primarily regulated by local governments and state and federal agencies. Specific natural resources assessed in this evaluation include salmonids, wildlife species, fish and wildlife habitat conservation areas, riparian areas, and wetlands that exist within or, in proximity of, the five proposed alternative pump station sites and associated water delivery pipe alignments.

All five alternative pump station sites being proposed are within a reach of the Wenatchee River that has been designated as Critical Habitat under the Endangered Species Act (ESA) for ESA-listed Upper Columbia River spring-run Chinook salmon (Oncorhynchus tshawytscha), Upper Columbia River steelhead trout (O. mykiss), and bull trout (Salvelinus confluentus). Maps showing designated Critical Habitat for the ESA-listed species are in Appendix F (Figures 1 through 3). Sockeye salmon (*O. nerka*) and summer-run Chinook salmon (O. tshawytscha) also occur in this reach of the Wenatchee River, and although not protected under the ESA, these species provide important recreational fisheries. Priority Habitat and Species (PHS) managed by WDFW and known to occur in close proximity (within 1,000 feet) to the proposed pump station sites and water delivery alignments include a spotted owl (Strix occidentalis) breeding area¹ (Appendix F, Figure 4). PHS elements managed by WDFW also include habitat features such as snags and logs. The Wenatchee River (riverine) is the only area in close proximity to the proposed pump station sites or to the water delivery pipeline alignments that is mapped on the National Wetlands Inventory (NWI) mapping. Riparian habitat is dominated by cottonwood (*Populus balsamifera*) and willow (*Salix sp.*), although it is fairly limited due to the steep banks at the proposed sites, or as a result of previous disturbance at the proposed sites located at bridge crossings or at the Dryden site as a result of clearing.

Upland habitat above the Wenatchee River is generally representative of species associated with the Columbia Cascade province; vegetation along the banks and on the hillslopes are characterized by shrub-steppe habitat-associated species (*Artemisia sp.* and bunchgrasses) and mixed stands of ponderosa pine (*Pinus ponderosa*) and Douglas fir (*Pseudotsuga*

¹ Resolution – Township; Accuracy – 1:100,000 feet; Data source/Date – WDFW/April 9, 1991.

menziesii), with tree species composition and stand size a function of increasing elevation and available soil moisture (Franklin and Dryness 1973).

5.5.2 Impacts and Regulatory Requirements

Short-term impacts anticipated from the construction of a pump station, fish screen, and water delivery pipeline include clearing, grading, and excavation in the uplands and excavation and fill below the ordinary high water mark (OHWM). Long-term impacts include operation of the water diversion pump station and maintenance activities for the pump station and water delivery pipeline.

For the purpose of the fatal flaw analysis, a standard permitting process is assumed to be required for the project. This process includes applying for and obtaining all relevant applicable federal, state, and local permits. Table 5-8 lists standard permits that would likely need to be obtained for the project.

As was noted previously, the Yakama Nation has indicated that there is lead contamination on WSDOT property that they have identified for a fish rearing facility at the same location proposed for the pump station site in Alternative 5. Acquisition of the site and development of the site for a fish rearing facility or irrigation pump station will likely require remediation, which could potentially increase the cost of the project.

5.5.3 Mitigation Opportunities

Following best management practices during mobilization and construction will avoid and minimize impacts to the natural environment to the greatest extent practicable. Work within the stream channel will need to be conducted during the fisheries work window consistent with permit requirements. Pump stations will need to be fitted with fish screens that meet NMFS specifications, to avoid impacts to salmonid species. Unavoidable impacts to riparian and upland vegetation will need to be mitigated by reestablishing vegetation using native trees and shrubs.

5.5.4 Conclusions

This environmental resources and permitting requirements fatal flaws analysis is intended to assess the potential at any of the five PID pump station sites being considered, for issues that would inhibit the project from obtaining needed permits and approvals. Based on this preliminary assessment, there are no known existing conditions at any of the five potential sites that would inhibit obtaining permits for construction and operation of a pump station at any of the potential sites as long as appropriate avoidance, minimization, and mitigation measures were employed to offset potential natural resource impacts. A more detailed environmental review and evaluation of permit requirements is recommended as part of a more detailed feasibility study of the preferred alternative(s).

Table 5-8

Standard Federal, State, and Local Permits List

			Permits	Needed	
Permit	Agency	Apply with the JARPA (Y/N)	Pump Station Sites	Pipeline Corridors	Notes
Section 10/ Section 404 Permit	Corps	Y	√		Locating a structure or excavation in navigable waters, or discharging dredged or fill material into waters of the U.S.
ESA Section 7 Concurrence	NMFS	N	\checkmark		
ESA Section 7 Concurrence	USFWS	Ν	\checkmark		Will likely require a Biological Assessment given presence
EFH Concurrence	NMFS	Ν	√		of listed salmonids and designated critical habitat
EFH Concurrence	USFWS	N	\checkmark		
NHPA Section 106 concurrence	DAHP	N	\checkmark	✓	If federal funding, DAHP and tribes must be consulted.
Hydraulic Permit Approval	WDFW	Y	\checkmark		For work below the OHWM in waters of the state
Section 401 Water Quality Certification	Ecology	Y	~		For projects that require excavation in or discharge dredge or fill material into water or isolated wetlands
NPDES Construction General Permit	Ecology	N	~		Required if more than 1 acre is disturbed during construction
Shoreline Substantial Development Permit	County	Y	~	~	Per the County Shoreline Management Plan, possible exemption for construction of irrigation structures
SEPA Determination	County	Y	√	√	
Critical Areas Ordinance Compliance	County	Y	~	~	Per the County Shoreline Management Plan, possible exemption for construction of irrigation structures
Floodplain Development Permit	County	Y	\checkmark	√	If within the 100-year floodplain
Fill and Grade Permit	County		~	√	

6 OPINIONS OF PROBABLE COST

As part of an initial analysis of the pump exchange project completed as part of the *Campbell Creek Reservoir Feasibility Study* (Anchor QEA 2010), preliminary opinions of probable project implementation costs and long-term operational costs were developed for each alternative. Those opinions of cost have been reviewed and updated as part of this Appraisal Study. An analysis was also completed as part of this Appraisal Study of the cost to replace the facility at the end of the design life cycle.

6.1 Assumptions

Opinions of project implementation costs were developed for each of the alternatives based on the following assumptions:

- A 10 percent allowance was included for mobilization/demobilization.
- A 30 percent contingency was included.
- A 20 percent allowance was included for engineering, permitting, and construction administration.
- The total project cost includes 8.0 percent sales tax.
- An allowance of \$50,000 was included for land acquisition for pump station and pipeline easements.

Opinions of long-term annual operating costs were also developed for each alternative (in 2011 dollars) based on the following assumptions:

- Annual operations and maintenance (O&M) would include the following:
 - Salaries Assumed for 1/12 full-time equivalent (FTE) to help operate and maintain the new facilities, or one person for about 8 hours per week during the irrigation season.
 - Benefits Estimated at 40 percent of the salary amount.
 - Transportation Costs An allowance for trips to and from the pump station.
 - Maintenance and Small Repairs Estimated as 0.3 percent of the total project implementation cost.
 - Administration, Insurance, Accounting An allowance for added administrative work and insurance required for the new facilities.

- Pumping power costs are based on Chelan PUD Rate Schedule 5, for irrigation service.
- Power costs are also based on the estimated horsepower required for pumping under each alternative.
- The energy charge portion of the power cost was estimated for pumping durations of 2, 4, 6, and 8 weeks.
- The power costs assume that the monthly basic charge and demand charge would only apply during the irrigation season. It is assumed that the power service would be shut down during the off season.

An analysis of replacement costs was also developed to determine the annual deposit that would be required in an account to fund replacement of the facilities at the end of the design life cycle of the project. The life cycle replacement cost analysis was based on the following assumptions:

- A 50-year design life cycle would apply to all of the facilities.
- The replacement cost would be equal to the current opinion of the implantation cost of the project plus an assumed removal cost inflated through the life of the project.
- No loan would be required to fund the initial project, and no loan repayment costs would be incurred during the life cycle of the project.
- The long-term inflation rate was assumed to be 3 percent.
- The interest rate on a replacement fund account was assumed to be 3 percent.

Unit costs and quantities should be considered approximate, in accordance with the preliminary nature of this Appraisal Study. Overall, the costs should be considered as orderof-magnitude costs.

6.2 Opinions of Probable Project Implementation Costs

Tables 6-1, 6-2, and 6-3 summarize the opinions of probable project implementation costs for facilities designed to pump from the Wenatchee River to the PID Canal. These include construction costs, engineering and administration costs, a contingency, taxes, and an allowance for land acquisition. The total opinion of costs for the pump exchange project ranges from \$1.86 million (Alternative 3) to \$3.17 million (Alternative 5) for a 10-cfs design

flow rate; \$2.68 million (Alternative 3) to \$4.40 million (Alternative 5) for a 20-cfs design flow rate; and \$3.90 million (Alternative 1) to \$6.15 million (Alternative 5) for a 40-cfs design flow rate. More detailed cost information, including a list of major items, estimated quantities, and unit costs used to develop the opinions of cost is included in Appendix G.

ltem	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Miscellaneous Site Work	\$80,989	\$63,668	\$71,053	\$70,248	\$57,592
Earthwork	\$35,197	\$46,714	\$40,432	\$77,079	\$112,050
Pump Station	\$724,600	\$664,600	\$684,600	\$664,600	\$639,600
Pipeline	\$180,830	\$388,910	\$215,020	\$701,180	\$933,880
Outlet Structure	\$5,000	\$5,000	\$5,000	\$5,000	\$5,000
Construction Subtotal	\$1,027,000	\$1,169,000	\$1,016,000	\$1,518,000	\$1,748,000
Mobilization/Demobilization (10%)	\$102,700	\$116,900	\$101,600	\$151,800	\$174,800
Contingency (30%)	\$338,910	\$385,770	\$335,280	\$500,940	\$576,840
Engineering and Administration (20%)	\$225,940	\$257,180	\$223,520	\$333,960	\$384,560
Tax (8%)	\$135,564	\$154,308	\$134,112	\$200,376	\$230,736
Land Acquisition	\$50,000	\$50,000	\$50,000	\$50,000	\$50,000
Total Project Costs	\$1,880,000	\$2,133,000	\$1,861,000	\$2,755,000	\$3,165,000

Table 6-1 Opinion of Probable Costs (10-cfs)

ltem	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Miscellaneous Site Work	\$83,989	\$66,668	\$74,053	\$73,248	\$60,592
Earthwork	\$49,799	\$66,115	\$57,216	\$109,128	\$158,668
Pump Station	\$1,094,200	\$1,024,200	\$1,044,200	\$1,024,200	\$994,200
Pipeline	\$246,207	\$521,439	\$292,758	\$906,222	\$1,223,052
Outlet Structure	\$6,000	\$6,000	\$6,000	\$6,000	\$6,000
Construction Subtotal	\$1,480,000	\$1,684,000	\$1,474,000	\$2,119,000	\$2,443,000
Mobilization/Demobilization (10%)	\$148,000	\$168,400	\$147,400	\$211,900	\$244,300
Contingency (30%)	\$488,400	\$555,720	\$486,420	\$699,270	\$806,190
Engineering and Administration (20%)	\$325,600	\$370,480	\$324,280	\$466,180	\$537,460
Tax (8%)	\$195,360	\$222,288	\$194,568	\$279,708	\$322,476
Land Acquisition	\$50,000	\$50,000	\$50,000	\$50,000	\$50,000
Total Project Costs	\$2,687,000	\$3,051,000	\$2,677,000	\$3,826,000	\$4,403,000

Table 6-2Opinion of Probable Costs (20-cfs)

Table 6-3Opinion of Probable Costs (40-cfs)

ltem	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Miscellaneous Site Work	\$62,989	\$70,668	\$63,053	\$78,248	\$76,592
Earthwork	\$64,629	\$83,464	\$72,808	\$134,965	\$194,280
Pump Station	\$1,698,400	\$1,618,400	\$1,668,400	\$1,618,400	\$1,588,400
Pipeline	\$324,103	\$677,431	\$385,382	\$1,139,038	\$1,556,108
Outlet Structure	\$10,000	\$10,000	\$10,000	\$10,000	\$10,000
Construction Subtotal	\$2,160,000	\$2,460,000	\$2,200,000	\$2,981,000	\$3,425,000
Mobilization/Demobilization (10%)	\$216,000	\$246,000	\$220,000	\$298,100	\$342,500
Contingency (30%)	\$712,800	\$811,800	\$726,000	\$983,730	\$1,130,250
Engineering and Administration (20%)	\$475,200	\$541,200	\$484,000	\$655,820	\$753,500
Tax (8%)	\$285,120	\$324,720	\$290,400	\$393,492	\$452,100
Land Acquisition	\$50,000	\$50,000	\$50,000	\$50,000	\$50,000
Total Project Costs	\$3,899,000	\$4,434,000	\$3,970,000	\$5,362,000	\$6,153,000

6.3 Opinions of Probable Operating Costs

Tables 6-4, 6-5, and 6-6 summarize the opinions of probable annual operating costs for facilities designed to pump from the Wenatchee River to the PID Canal. The costs include O&M and an opinion of probable power costs. The ranges of projected annual operational costs are as follows:

- 10-cfs pump exchange project
 - \$20,400 (Alternative 3) to \$23,900 (Alternative 5) for 2 weeks of pumping
 - \$25,200 (Alternative 3) to \$28,300 (Alternative 5) for 8 weeks of pumping
- 20-cfs pump exchange project
 - \$30,700 (Alternative 3) to \$34,200 (Alternative 5) for 2 weeks of pumping
 - \$40,200 (Alternative 3) to \$42,600 (Alternative 5) for 8 weeks of pumping
- 40-cfs pump exchange project
 - \$52,500 (Alternative 3) to \$55,900 (Alternative 5) for 2 weeks of pumping
 - \$72,100 (Alternative 3) to \$75,200 (Alternative 2) for 8 weeks of pumping

Item	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5			
Annual O&M Costs ¹	\$11,920	\$12,820	\$11,920	\$14,720	\$16,120			
Pumping Power Costs ²	Pumping Power Costs ²							
2-Week Annual Pumping Duration	\$8,658	\$8,767	\$8,484	\$8,093	\$7,789			
4-Week Annual Pumping Duration	\$10,292	\$10,421	\$10,085	\$9,619	\$9,257			
6-Week Annual Pumping Duration	\$11,925	\$12,075	\$11,685	\$11,145	\$10,725			
8-Week Annual Pumping Duration	\$13,559	\$13,730	\$13,286	\$12,671	\$12,193			
Total Annual Operating Costs ³								
2-Week Annual Pumping Duration	\$20,600	\$21,600	\$20,400	\$22,800	\$23,900			
4-Week Annual Pumping Duration	\$22,200	\$23,200	\$22,000	\$24,300	\$25,400			
6-Week Annual Pumping Duration	\$23,800	\$24,900	\$23,600	\$25,900	\$26,800			
8-Week Annual Pumping Duration	\$25,500	\$26,500	\$25,200	\$27,400	\$28,300			

Table 6-4Opinion of Probable Operating Costs (10-cfs)

Notes:

1. Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance, and accounting in 2012 dollars.

2. Pumping costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service).

3. Total Annual Operating Costs are rounded to the nearest \$100.

ltem	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Annual O&M Costs ¹	\$14,220	\$15,320	\$14,120	\$17,620	\$19,320
Pumping Power Costs ²					
2-Week Annual Pumping Duration	\$17,026	\$17,157	\$16,613	\$15,635	\$14,853
4-Week Annual Pumping Duration	\$20,252	\$20,408	\$19,761	\$18,597	\$17,665
6-Week Annual Pumping Duration	\$23,478	\$23,658	\$22,908	\$21,558	\$20,478
8-Week Annual Pumping Duration	\$26,704	\$26,909	\$26,056	\$24,519	\$23,290
Total Annual Operating Costs ³					
2-Week Annual Pumping Duration	\$31,200	\$32,500	\$30,700	\$33,300	\$34,200
4-Week Annual Pumping Duration	\$34,500	\$35,700	\$33,900	\$36,200	\$37,000
6-Week Annual Pumping Duration	\$37,700	\$39,000	\$37,000	\$39,200	\$39,800
8-Week Annual Pumping Duration	\$40,900	\$42,200	\$40,200	\$42,100	\$42,600

Opinion of Probable Operating Costs (20-cfs)

Notes:

See notes for Table 6-4.

Table 6-6

Opinion of Probable Operating Costs (40-cfs)

Item	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5
Annual O&M Costs ¹	\$17,820	\$19,420	\$18,020	\$22,220	\$24,620
Pumping Power Costs ²					
2-Week Annual Pumping Duration	\$35,219	\$35,567	\$34,480	\$32,698	\$31,307
4-Week Annual Pumping Duration	\$41,907	\$42,321	\$41,027	\$38,906	\$37,250
6-Week Annual Pumping Duration	\$48,595	\$49,075	\$47,574	\$45,114	\$43,193
8-Week Annual Pumping Duration	\$55,282	\$55,829	\$54,121	\$51,322	\$49,136
Total Annual Operating Costs ³					
2-Week Annual Pumping Duration	\$53,000	\$55,000	\$52,500	\$54,900	\$55,900
4-Week Annual Pumping Duration	\$59,700	\$61,700	\$59,000	\$61,100	\$61,900
6-Week Annual Pumping Duration	\$66,400	\$68,500	\$65,600	\$67,300	\$67,800
8-Week Annual Pumping Duration	\$73,100	\$75,200	\$72,100	\$73,500	\$73 <i>,</i> 800

Notes:

See notes for Table 6-4.

6.4 Life Cycle Replacement Cost Analysis

Replacement costs were evaluated to determine the annual deposit that would need to be made to an account to fund replacement of the facilities at the end of the assumed life cycle for the project. It's likely that components of the project will have longer or shorter design life cycles; however, to simplify the analysis, an overall design life cycle of 50 years was assumed. It is also unlikely that the all of the facilities would need to be completely replaced at the end of the assumed life cycle. For this reason, the analysis was performed for three levels of replacement: 25, 50, and 100 percent. The life cycle replacement cost analysis is also included in Appendix G.

Two methods of annual deposit to a replacement fund were evaluated. The first would be a constant annual deposit through the life of the project. The second would be an increasing annual deposit, escalated at the assumed annual inflation rate of 3 percent. Tables 6-7, 6-8, and 6-9 summarize the estimated annual replacement fund costs at project years 1, 25, and 50.

	Level of								
Year	Replacement	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5			
With Co	With Constant Annual Deposit:								
	25%	\$18,733	\$21,269	\$18,549	\$27,468	\$31,559			
1-50	50%	\$37,466	\$42,538	\$37,097	\$54,936	\$63,118			
	100%	\$74,933	\$85,077	\$74,194	\$109,873	\$126,235			
With In	creasing Annual Depo	osit:							
	25%	\$9,929	\$11,273	\$9,831	\$14,559	\$16,727			
1	50%	\$19,858	\$22,547	\$19,663	\$29,118	\$33,454			
	100%	\$39,717	\$45,093	\$39,325	\$58,236	\$66,909			
	25%	\$20,184	\$22,916	\$19,985	\$29,596	\$34,003			
25	50%	\$40,368	\$45,833	\$39,970	\$59,191	\$68,006			
	100%	\$80,736	\$91,666	\$79,940	\$118,382	\$136,012			
	25%	\$42,261	\$47,982	\$41,844	\$61,967	\$71,195			
50	50%	\$84,522	\$95,964	\$83,689	\$123,933	\$142,389			
	100%	\$169,043	\$191,927	\$167,378	\$247,866	\$284,779			

Table 6-7Estimated Annual Replacement Fund Costs – 10-cfs Pump Station

	Level of								
Year	Replacement	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5			
With C	Vith Constant Annual Deposit:								
	25%	\$26,759	\$30,383	\$26,662	\$38,108	\$43,850			
1-50	50%	\$53,518	\$60,766	\$53,323	\$76,215	\$87,700			
	100%	\$107,036	\$121,532	\$106,647	\$152,430	\$175,400			
With In	creasing Annual Depo	osit:							
	25%	\$14,183	\$16,104	\$14,132	\$20,198	\$23,242			
1	50%	\$28,366	\$32,208	\$28,263	\$40,397	\$46,484			
	100%	\$56,732	\$64,416	\$56,526	\$80,793	\$92,968			
	25%	\$28,831	\$32,736	\$28,727	\$41,059	\$47,246			
25	50%	\$57,663	\$65,472	\$57,453	\$82,118	\$94,492			
	100%	\$115,325	\$130,945	\$114,907	\$164,236	\$188,984			
	25%	\$60,366	\$68,542	\$60,147	\$85,968	\$98,923			
50	50%	\$120,733	\$137,085	\$120,294	\$171,937	\$197,846			
	100%	\$241,466	\$274,169	\$240,589	\$343,874	\$395,691			

Estimated Annual Replacement Fund Costs – 20-cfs Pump Station

	Level of							
Year	Replacement	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5		
With Co	With Constant Annual Deposit:							
	25%	\$38,836	\$44,161	\$39,536	\$53,401	\$61,281		
1-50	50%	\$77,673	\$88,322	\$79,072	\$106,802	\$122,562		
	100%	\$155,345	\$176,644	\$158,144	\$213,605	\$245,125		
With In	creasing Annual Depo	osit:						
	25%	\$20,585	\$23,407	\$20,955	\$28,304	\$32,481		
1	50%	\$41,169	\$46,813	\$41,911	\$56,609	\$64,962		
	100%	\$82,338	\$93,627	\$83,821	\$113,218	\$129,924		
	25%	\$41,844	\$47,581	\$42,598	\$57,537	\$66,027		
25	50%	\$83,688	\$95,162	\$85,196	\$115,074	\$132,055		
	100%	\$167,377	\$190,324	\$170,392	\$230,148	\$264,109		
	25%	\$87,612	\$99,624	\$89,191	\$120,470	\$138,246		
50	50%	\$175,225	\$199,249	\$178,381	\$240,939	\$276,493		
	100%	\$350,449	\$398,497	\$356,762	\$481,879	\$552,986		

Estimated Annual Replacement Fund Costs – 40-cfs Pump Station

6.5 Funding for Total Project Costs

The purpose of developing opinions of project costs as part of this Appraisal Study is to help stakeholders identify potential funding opportunities and costs constraints. PID has indicated that funding for both project implementation and long-term power and O&M costs would need to be secured to make the project viable. Total project costs that would need funding, including the project implementation cost and the present value of estimated annual long-term operating costs through the assumed 50-year life cycle of the project, are summarized in Tables 6-10, 6-11 and 6-12. The ranges of the totals of the present values of these projected costs are as follows:

- 10-cfs pump exchange project
 - \$2.9 million (Alternative 3) to \$4.4 million (Alternative 5) for 2 weeks of pumping
 - \$3.2 million (Alternative 3) to \$4.6 million (Alternative 5) for 8 weeks of pumping

- 20-cfs pump exchange project
 - \$4.2 million (Alternative 3) to \$6.1 million (Alternative 5) for 2 weeks of pumping
 - \$4.7 million (Alternative 3) to \$6.5 million (Alternative 5) for 8 weeks of pumping
- 40-cfs pump exchange project
 - \$6.6 million (Alternative 1) to \$8.9 million (Alternative 5) for 2 weeks of pumping
 - \$7.6 million (Alternative 1) to \$9.8 million (Alternative 5) for 8 weeks of pumping

Alternative	Annual Pumping Duration	Total Project Implementation Cost ¹	Present Value - O&M Costs ²	Present Value - Power Costs ³	Present Value - Total Project Costs ⁴
1	2 Weeks	\$1,880,000	\$596,000	\$432,908	\$2,908,908
	4 Weeks	\$1,880,000	\$596,000	\$514,590	\$2,990,590
	6 Weeks	\$1,880,000	\$596,000	\$596,273	\$3,072,273
	8 Weeks	\$1,880,000	\$596,000	\$677,955	\$3,153,955
2	2 Weeks	\$2,133,000	\$641,000	\$438,341	\$3,212,341
	4 Weeks	\$2,133,000	\$641,000	\$521,058	\$3,295,058
	6 Weeks	\$2,133,000	\$641,000	\$603,774	\$3,377,774
	8 Weeks	\$2,133,000	\$641,000	\$686,491	\$3,460,491
3	2 Weeks	\$1,861,000	\$596,000	\$424,213	\$2,881,213
	4 Weeks	\$1,861,000	\$596,000	\$504,241	\$2,961,241
	6 Weeks	\$1,861,000	\$596,000	\$584,270	\$3,041,270
	8 Weeks	\$1,861,000	\$596,000	\$664,298	\$3,121,298
4	2 Weeks	\$2,755,000	\$736,000	\$404,651	\$3,895,651
	4 Weeks	\$2,755,000	\$736,000	\$480,957	\$3,971,957
	6 Weeks	\$2,755,000	\$736,000	\$557,263	\$4,048,263
	8 Weeks	\$2,755,000	\$736,000	\$633,569	\$4,124,569
5	2 Weeks	\$3,165,000	\$806,000	\$389,436	\$4,360,436
	4 Weeks	\$3,165,000	\$806,000	\$462,847	\$4,433,847
	6 Weeks	\$3,165,000	\$806,000	\$536,258	\$4,507,258
	8 Weeks	\$3,165,000	\$806,000	\$609,669	\$4,580,669

Total Costs Requiring Funding – 10-cfs Pump Station

Notes:

1. Total project implementation cost includes construction, engineering, administration, contingency, and taxes.

2. Present value of annual O&M costs through assumed 50-year project life cycle.

- 3. Present value of annual power costs through assumed 50-year project life cycle.
- 4. Sum of total project implementation cost, present value of O&M costs, and present value of power costs.

Alternative	Annual Pumping Duration	Total Project Implementation Cost ¹	Present Value - O&M Costs ²	Present Value - Power Costs ³	Present Value - Total Project Costs ⁴
1	2 Weeks	\$2,687,000	\$711,000	\$851,322	\$4,249,322
	4 Weeks	\$2,687,000	\$711,000	\$1,012,619	\$4,410,619
	6 Weeks	\$2,687,000	\$711,000	\$1,173,916	\$4,571,916
	8 Weeks	\$2,687,000	\$711,000	\$1,335,214	\$4,733,214
2	2 Weeks	\$3,051,000	\$766,000	\$857,843	\$4,674,843
	4 Weeks	\$3,051,000	\$766,000	\$1,020,381	\$4,837,381
	6 Weeks	\$3,051,000	\$766,000	\$1,182,919	\$4,999,919
	8 Weeks	\$3,051,000	\$766,000	\$1,345,457	\$5,162,457
3	2 Weeks	\$2,677,000	\$706,000	\$830,673	\$4,213,673
	4 Weeks	\$2,677,000	\$706,000	\$988,041	\$4,371,041
	6 Weeks	\$2,677,000	\$706,000	\$1,145,409	\$4,528,409
	8 Weeks	\$2,677,000	\$706,000	\$1,302,777	\$4,685,777
4	2 Weeks	\$3,826,000	\$881,000	\$781,767	\$5,488,767
	4 Weeks	\$3,826,000	\$881,000	\$929,830	\$5,636,830
	6 Weeks	\$3,826,000	\$881,000	\$1,077,892	\$5,784,892
	8 Weeks	\$3,826,000	\$881,000	\$1,225,955	\$5,932,955
5	2 Weeks	\$4,403,000	\$966,000	\$742,643	\$6,111,643
	4 Weeks	\$4,403,000	\$966,000	\$883,261	\$6,252,261
	6 Weeks	\$4,403,000	\$966,000	\$1,023,879	\$6,392,879
	8 Weeks	\$4,403,000	\$966,000	\$1,164,497	\$6,533,497

Table 6-11Total Costs Requiring Funding – 20-cfs Pump Station

Notes:

1. Total project implementation cost includes construction, engineering, administration, contingency, and taxes.

2. Present value of annual O&M costs through assumed 50-year project life cycle.

3. Present value of annual power costs through assumed 50-year project life cycle.

4. Sum of total project implementation cost, present value of O&M costs, and present value of power costs.

Alternative	Annual Pumping Duration	Total Project Implementation Cost ¹	Present Value - O&M Costs ²	Present Value - Power Costs ³	Present Value - Total Project Costs ⁴
1	2 Weeks	\$3,899,000	\$891,000	\$1,760,966	\$6,550,966
	4 Weeks	\$3,899,000	\$891,000	\$2,095,348	\$6,885,348
	6 Weeks	\$3,899,000	\$891,000	\$2,429,729	\$7,219,729
	8 Weeks	\$3,899,000	\$891,000	\$2,764,110	\$7,554,110
2	2 Weeks	\$4,434,000	\$971,000	\$1,778,355	\$7,183,355
	4 Weeks	\$4,434,000	\$971,000	\$2,116,045	\$7,521,045
	6 Weeks	\$4,434,000	\$971,000	\$2,453,735	\$7,858,735
	8 Weeks	\$4,434,000	\$971,000	\$2,791,425	\$8,196,425
3	2 Weeks	\$3,970,000	\$901,000	\$1,724,015	\$6,595,015
	4 Weeks	\$3,970,000	\$901,000	\$2,051,366	\$6,922,366
	6 Weeks	\$3,970,000	\$901,000	\$2,378,716	\$7,249,716
	8 Weeks	\$3,970,000	\$901,000	\$2,706,067	\$7,577,067
4	2 Weeks	\$5,362,000	\$1,111,000	\$1,634,899	\$8,107,899
	4 Weeks	\$5,362,000	\$1,111,000	\$1,945,292	\$8,418,292
	6 Weeks	\$5,362,000	\$1,111,000	\$2,255,686	\$8,728,686
	8 Weeks	\$5,362,000	\$1,111,000	\$2,566,079	\$9,039,079
5	2 Weeks	\$6,153,000	\$1,231,000	\$1,565,344	\$8,949,344
	4 Weeks	\$6,153,000	\$1,231,000	\$1,862,503	\$9,246,503
	6 Weeks	\$6,153,000	\$1,231,000	\$2,159,662	\$9,543,662
	8 Weeks	\$6,153,000	\$1,231,000	\$2,456,821	\$9,840,821

Table 6-12 Total Costs Requiring Funding – 40-cfs Pump Station

Notes:

1. Total project implementation cost includes construction, engineering, administration, contingency, and taxes.

2. Present value of annual O&M costs through assumed 50-year project life cycle.

3. Present value of annual power costs through assumed 50-year project life cycle.

4. Sum of total project implementation cost, present value of O&M costs, and present value of power costs.

Funding would also need to be set aside or secured during the life of the project to enable removal and replacement of facilities at the end of the assumed life cycle, as outlined in Tables 6-7, 6-8, and 6-9.

6.6 Key Cost Factors

The opinions of probable project costs and long-term operating costs will need to be refined as part of a more detailed feasibility study and detailed design of the preferred pump exchange project alternative. At this stage, several key elements of the project that could have a significant impact on project and long-term operating costs have not been well defined. The following key cost factors will need to be reviewed and considered in more detail as part of future efforts to refine opinions of cost:

- Subsurface Materials No subsurface geological exploration has been done to verify the condition of subsurface soils that will need to be excavated or used to construct the pump station, delivery pipeline, delivery structure, and other improvements.
- Slope Stability Additional study is needed to more accurately assess slope stability risks and determine mitigation measures, which could be very expensive.
- River Channel Modifications As indicated by the geomorphic review of the existing river channel, there may be need to modify the river channel to enable diversion to the pump station at low flows at some pump station locations.
- River Channel Maintenance The geomorphic review also indicated that regular channel maintenance may be required to ensure the long-term ability to divert water.
- Pump Station Design A more detailed design analysis is needed to determine the size, configuration, and cost of the structure, pumps, and screening facilities.
- Pipeline Alignment The proposed pipeline alignment selected for the proposed alternative may vary depending on permit requirements, property constraints, locations of existing infrastructure, and other factors. If an alternate pipeline alignment is selected, the cost of the pipeline and pumping requirements will change.
- Easements and Property Acquisition At this point, property owners have not been contacted and the cost of negotiating and securing easements and property for the facilities has not been evaluated. The location, alignment, and cost of the facilities could be significantly impacted by negotiations with property owners.
- Coordination with Other Projects Development of the project will require coordination with public agencies and others who are developing projects within the study area. For example, the cost of constructing a pump station at the site proposed for Alternative 5 could be impacted by negotiations with others who are targeting that site for projects, including the Yakama Nation and WDFW. Construction of a shared diversion with a proposed fish rearing facility that the Yakama Nation is proposing to develop at the same site could potentially result in a cost savings for both projects.

• Power Requirements – The extent and magnitude of upgrades and extensions to existing power supply facilities has not been well defined. Project costs could be significantly impacted by the need to extend power from existing infrastructure and upgrade existing power infrastructure to support the new pump station.

7 SUMMARY OF ALTERNATIVES AND FATAL FLAW EVALUATION

This section includes a comparison of the preliminary alternatives evaluated as part of this Appraisal Study and a brief summary of challenges and potential fatal flaws associated with each alternative.

7.1 Comparison of Preliminary Alternatives

Table 7-1 provides a comparison of key characteristics of the five preliminary alternatives that were evaluated as part of this Appraisal Study. The following information is summarized for comparison of the alternatives in Table 7-1:

- Pump Station and Discharge Pipe Location The proposed pump station sites are located along the right bank of the Wenatchee River from RM 16.4 (Alternative 2), near the town of Dryden, to RM 17.8 (Alternative 5), just upstream of Dryden Dam. Water would be delivered to the ditch within a reach extending from 19,560 feet downstream of the diversion (Alternatives 1 and 2) up to 12,360 feet downstream of the diversion (Alternative 5).
- Pipeline The proposed delivery pipelines would vary in length from 1,240 feet (Alternative 1) to 4,910 feet (Alternative 5). The size of the pipe would primarily depend on the design flow. A 20-inch pipe is recommended for a 10-cfs flow, a 30-inch pipe is recommended for a 20-cfs flow, and a 36-inch pipe is recommended for a 40-cfs flow.
- Pump Station Horsepower The total horsepower required for pumping would vary from 360 horsepower (Alternative 5) to 400 horsepower (Alternative 2) for a 10-cfs pump Station, and from 1,440 horsepower (Alternative 5) pump station to 1,650 horsepower (Alternative 2) for a 40-cfs pump station.
- Project Costs The estimated cost to design and construct the pump exchange project would vary from \$1.86 million (Alternative 3) to \$3.17 million (Alternative 5) for a 10-cfs project. The estimated cost to design and construct the pump exchange project would vary from \$3.90 million (Alternative 1) to \$6.15 million (Alternative 5) for a 40-cfs project.

 Operating Costs – Annual pumping costs would be lowest for Alternative 5 and highest for Alternatives 1, 2, and 3, similar to pumping horsepower requirements. However, O&M costs, which were partially developed based on project costs, would be highest for Alternative 5. The overall opinion of probable annual long-term operating costs for an 8-week pumping duration, which would include both pumping and O&M costs, would range from \$25,200 (Alternative 3) to \$28,300 (Alternative 5), for a 10-cfs pump station, and from \$72,100 (Alternative 3) to \$75,200 (Alternative 2), for a 40-cfs pump station.

Table 7-1 also outlines specific challenges identified through the analysis of the key characteristics of each alternative. The following conclusions can be drawn from a comparison of the challenges and benefits listed.

- Configuration/Location Accessibility of the proposed pump station site would likely be a challenge for Alternatives 1, 3, and 4. For alternatives 3, 4, and 5, the delivery to the canal would be located upstream of the most vulnerable section of the PID Canal, which includes segment of steel pipe suspended above a slide area near the Dryden Transfer Station. Although delivering the water further upstream would increase the number of customers that could potential be served through the pump station, the delivery would still be subject to the reliability of the canal through the slide area. Delivering the water downstream of the slide area, as shown for Alternatives 1 and 2, would improve system reliability by allowing the majority of the PID Canal to operate in the event that a slide damages the canal through this vulnerable section.
- Hydraulics From a hydraulic perspective, Alternative 5 would require the least pumping head and horsepower, since it would have the smallest elevation gain between the river and the ditch. Alternatives 1, 2, and 3 would have higher pumping heads and horsepower requirements.
- Geomorphology The geomorphic analysis indicated that the pump station locations designated for Alternative 5 (upstream of Dryden Dam) and Alternative 1 (upstream of easternmost U.S. Highway 2 bridge) would be the most ideal for diversion from the Wenatchee River under a variety of flow conditions. Alternative 5 would take advantage of the pool conditions created by Dryden Dam. Alternative 1 would take advantage of a pool created by exposed bedrock on the right bank of the river.

Alternatives 2, 3, and 4 would require a higher level of channel modification and channel maintenance to maintain diversions under low flow conditions.

- Geology Alternatives 1, 2, 3, and 4 would likely require excavation of boulders and cobbles for pump station installation. Alternative 5 would probably require less excavation of rock for pump station installation and the slope stability risk along the pipeline route would be lower than for the other alternatives. The slope stability risk would be low to moderate for Alternatives 1, 2, and 4 and high for Alternative 3. The delivery pipelines for Alternatives 1, 2, and 3 would all ascend steep slopes to the PID Canal. If Alternative 2 requires jacking and boring, the preliminary review of geology indicates that jacking and boring could be very challenging.
- Property Issues Alternative 1 would impact the fewest number of parcels and would not impact WSDOT right-of-way along U.S. Highway 2. However, both Alternatives 1 and 2 would impact a private orchard located south of U.S Highway 2. Alternative 3 would impact Chelan County-owned property, including the access road to the existing Dryden Transfer Station. Alternative 4 would impact a private orchard north of U.S. Highway 2 and would require construction in Chelan County right-of-way and a crossing of WSDOT right-of-way at U.S. Highway 2. Alternative 5 would impact the greatest number of parcels and would cross WSDOT right-of-way at U.S. Highway 2. However, most of Alternative 5 would be constructed in public roadways or driveways, so impacts to private property would be minimal.
- Environmental Impacts and Permitting No environmental or permitting fatal flaws were identified for any of the alternatives based on the preliminary review that was done for this Appraisal Study. All of the alternatives have the potential to impact ESA-listed species, riparian habitat, and upland habitat features. Alternative 5 would have the least impact on steep slopes, planted areas, and undisturbed areas because the facilities would be mostly located within public rights-of-way or the PID Canal easement. Alternative 5 could face challenges from a right-of-way permit perspective because the proposed alignment extends up Deadman Hill Road, which is currently being resurfaced by Chelan County.

Table 7-1Comparison of Preliminary Alternatives

	Alignment							
Features	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5			
	South of Hwy 2; approximately 7,250 ft	North of Hwy 2; approximately 7,800 ft	South of Hwy 2; approximately 4,650 ft	North of Hwy 2; approximately 2,560 ft	North of Hwy 2; approximately 700 ft			
Pump Station Location	downstream of Peshastin Creek	downstream of Peshastin Creek						
	(approximately RM 16.5)	(approximately RM 16.4)	(approximately RM 17.0)	(approximately RM 17.4)	(approximately RM 17.8)			
Discharge Location	PID Ditch, approximately 19,560 ft D/S of	PID Ditch, approximately 19,560 ft D/S of	PID Ditch, approximately 14,720 ft D/S of	PID Ditch, approximately 14,240 ft D/S of	PID Ditch, approximately 12,860 ft D/S of			
	Diversion, elevation approximately 1,146 ft	Diversion, elevation approximately 1,146 ft	Diversion, elevation approximately 1,152 ft	Diversion, elevation approximately 1,153 ft	Diversion, elevation approximately 1,158 f			
Delivery Pipe Length (ft)	1,240	1,790	1,490	3,240	4,910			
Delivery Pipe Size (in)		Γ		Γ				
10 cfs	20	20	20	20	20			
20 cfs	30	30	30	30	30			
40 cfs	36	36	36	36	36			
Pumping Head, TDH (ft)								
10 cfs	244	247	239	228	219			
20 cfs	240	242	235	221	210			
40 cfs	250	252	244	232	222			
Total Horsepower					•			
10 cfs	400	400	400	370	360			
20 cfs	780	780	780	720	680			
40 cfs	1,620	1,650	1,590	1,500	1,440			
Number of Pumps Recomr	nended	l		1				
10 cfs	2	2	2	2	2			
20 cfs	3	3	3	3	3			
40 cfs	3	3	3	3	3			
Horsepower/Pump		l		1				
10 cfs	200	200	200	190	180			
20 cfs	260	260	260	240	230			
40 cfs	540	550	530	500	480			
Opinion of Probable Proje	ct Costs (2012 Dollars)	1	1	1	1			
10 cfs	\$1,880,000	\$2,133,000	\$1,861,000	\$2,755,000	\$3,165,000			
20 cfs	\$2,687,000	\$3,051,000	\$2,677,000	\$3,826,000	\$4,403,000			
40 cfs	\$3,899,000	\$4,434,000	\$3,970,000	\$5,362,000	\$6,153,000			
	al Operating Costs (2 Weeks of Pumping, 201		. , -,		. , ,			
10 cfs	\$20,600	\$21,600	\$20,400	\$22,800	\$23,900			
20 cfs	\$31,200	\$32,500	\$30,700	\$33,300	\$34,200			
40 cfs	\$53,000	\$55,000	\$52,500	\$54,900	\$55,900			
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	Alignment							
Features	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5			
Parcels Impacted	2 Private Parcels Directly Impacted	3 Private Parcels Directly Impacted	1 Private Parcels Directly Impacted	• 2 Private Parcels Directly Impacted	• 1 Private Parcel Directly Impacted			
	• 1 Adjacent Parcel Potentially Impacted	• 1 Adjacent Parcel Potentially Impacted	• 2 County Parcels Directly Impacted	• 1 County Parcel Directly Impacted	• 1 County PUD Parcel Directly Impacted			
		WSDOT R-O-W Impacted	• 1 Adjacent Parcel Potentially Impacted	• 1 Adjacent Parcel Potentially Impacted	3 State-Owned Parcels Directly			
				WSDOT and County ROW Impacted	Impacted			
					4 Adjacent Parcel Potentially Impacted			
					WSDOT and County ROW Impacted			
Challenges								
Location/Configuration	Proximity to U.S. Highway 2	Proximity to U.S. Highway 2	Proximity to U.S. Highway 2	Crosses U.S. Highway 2 R-O-W	Crosses U.S. Highway 2 R-O-W			
	Difficult Access to Pump Station Site	Crosses U.S. Highway 2 R-O-W	Difficult Access to Pump Station Site	Least Accessible Pump Station Site	Impact to Newly Resurfaced County			
			Delivery Upstream of Most Vulnerable	Long Pipe Length	Road			
			Section of PID Canal	Delivery Upstream of Most Vulnerable	Longest Pipe Length			
				Section of PID Canal	Delivery Upstream of Most Vulnerable			
					Section of PID Canal			
Hydraulics	High Elevation Gain	Highest Elevation Gain	High Elevation Gain					
	High TDH, Horsepower Required	Highest TDH, Horsepower Required	High TDH, Horsepower Required					
Geomorphology	Steep Bank at Pump Station Site	Steep Bank at Pump Station Site	Main Flow is at Center of Channel	Minimal Shelter from High Flows				
		Main Flow is Away from Right Bank	Channel Modification and Regular	Main Flow near Far Bank of River				
		Some Channel Modification and	Maintenance Would Likely be Required	Channel Modification and Regular				
		Maintenance Would Likely be Required	 Gravel Deposits at Right Bank 	Maintenance Would Likely be Required				
		Gravel Bedload Transport		Gravel Deposits at Right Bank				
Geology	Excavation of Large Rock for Pump	Excavation of Large Rock for Pump	Excavation of Large Rock for Pump	Excavation of Large Rock for Pump				
	Station	Station	Station	Station				
	Steep Pipeline Alignment	Steep Pipeline Alignment	Steep Pipeline Alignment	Low to Moderate Slope Stability Risk				
	Low to Moderate Slope Stability Risk	• Difficult Jack/Bore Across U.S. Highway 2	Potentially High Slope Stability Risk					
		Low to Moderate Slope Stability Risk						
Property Issues	Impact to Private Orchard	Impact to Private Orchards	Impact to Transfer Station Access	Impact to Private Orchard	Impact to Existing and Planned Fish			
	Impact to Private Property for Access	Impact to Private Property for Access		Impact to County and WSDOT R-O-W	Rearing Activities at Dryden Dam			
		Impact to WSDOT R-O-W			Impact to County and WSDOT R-O-W			
Environmental/Permit	• ESA-listed Chinook salmon, Steelhead,	• ESA-listed Chinook salmon, Steelhead,	• ESA-listed Chinook salmon, Steelhead,	• ESA-listed Chinook salmon, Steelhead,	• ESA-listed Chinook salmon, Steelhead,			
	Bull Trout	Bull Trout	Bull Trout	Bull Trout	Bull Trout			
	Impacts to Riparian Habitat	Impacts to Riparian Habitat	Impacts to Riparian Habitat	Impacts to Riparian Habitat	Impacts to Riparian Habitat			
	Impacts to Steep Slopes, Planted Areas	Impacts to Steep Slopes, Planted Areas	Impacts to Steep Slopes, Planted Areas					
Benefits								
Location/Configuration	Shortest Pipe Length	Short Pipe Length	Short Pipe Length	Uses Existing Driveways and Roadways	Uses Existing Driveways and Roadways			
	No U.S. Highway 2 Crossing Required	Relatively Accessible Pump Station Site	No U.S. Highway 2 Crossing Required	Relatively Accessible Delivery Location	Most Accessible Pump Station Site			
	Relatively Accessible Delivery Location	Relatively Accessible Delivery Location	Relatively Accessible Pump Station Site		Most Accessible Delivery Location			
	Delivery Downstream of Most	Delivery Downstream of Most	Relatively Accessible Delivery Location					
	Vulnerable Section of PID Canal	Vulnerable Section of PID Canal			Lowest Floyation Coin			
Hydraulics				Lower Elevation Gain Lower TDH, Hersenower Required	Lowest Elevation Gain			
				Lower TDH, Horsepower Required	 Lowest TDH, Horsepower Required 			

	Alignment						
Features	Alternative 1	Alternative 2	Alternative 3	Alternative 4	Alternative 5		
Geomorphology	 Exposed Bedrock at Proposed Diversion Large, Deep Pool Near Exposed Bedrock Minimal Channel Maintenance 	Protection from High Flows by Bridge	Less Steep Bank Conditions	Less Steep Bank Conditions	 Dam Creates Pool No Channel Modification Needed No Channel Maintenance Expected Upstream of Dam, Level Control Highest Certainty of Diversion at Low Flows 		
Geology	• No Jack/Bore		No Jack/Bore	Less Difficult Jack/Bore	 Easier Excavation for Pump Station Less Difficult Jack/Bore Low Slope Stability Risk 		
Property Issues			Primarily Impacts County Properties		Primarily Impacts State Properties		
Environmental/Permit	No Known Permitting Fatal Flaws	No Known Permitting Fatal Flaws	No Known Permitting Fatal Flaws	 No Known Permitting Fatal Flaws Less Impact to Steep Slopes, and Undisturbed or Planted Areas 	 No Known Permitting Fatal Flaws Least Impact to Steep Slopes, and Undisturbed or Planted Areas 		

Notes:

cfs = cubic feet per second County = Chelan County D/S = downstream ESA = Endangered Species Act ft = feet Hwy = U.S. Highway in = inches PID = Peshastin Irrigation District RM = River Mile R-O-W = right-of-way TDH = Total Dynamic Head WSDOT = Washington State Department of Transportation

7.2 Summary of Preliminary Fatal Flaw Evaluation

The preliminary alternatives were reviewed for the purpose of identifying significant challenges, or fatal flaws, that could prevent design and installation of the pump exchange project defined by each alternative. The following lists the most significant challenges associated with each of the alternatives:

- Overall
 - Project Cost In order for the project to be viable to PID, funding sources will need to be identified for design and construction of the facilities.
 - Long-Term Operating Costs Funding of long-term operating costs, especially
 power costs, represents a significant long-term financial commitment. In order to
 make the project viable to PID, funding assistance or some arrangement will likely
 need to be made to help PID cover long-term O&M and power costs.
- Alternative 1
 - Access The ideal pump station site for this alternative would be located near a bedrock outcropping upstream of the U.S. Highway 2 bridge. Constructing temporary and permanent access to the proposed site will be difficult because of property constraints and stability issues.
 - Pump Station Construction Pump station construction would likely require excavation of material with large boulders and cobbles.
 - Slope Stability There is low to moderate slope stability risk of the proposed pipeline alignment where it ascends the steep hillside to the ditch. The stability of the hillside above the proposed pump station location could present a significant challenge.
 - Private Property Construction would impact a private orchard and would require cooperation of private property owners.
- Alternative 2
 - U.S. Highway 2 Crossing The proposed pipeline would cross U.S. Highway 2 under the bridge adjacent to the east abutment. Construction of a pipeline under the bridge will likely require extensive coordination with WSDOT and may be a challenging installation. An alternate alignment could include boring or jacking under the highway east of the bridge. The preliminary review of geology

indicates that the bore or jack could encounter rock and would likely be a difficult and expensive installation.

- Channel Modifications The main flow in the channel is located toward the center of the channel, rather than at the right bank where the pump station would be located. Channel modification would be required to enable diversion to the pump station at low flow rates.
- Pump Station Construction Pump station construction would likely require excavation of material with large boulders and cobbles.
- Slope Stability There is low to moderate slope stability risk of the proposed pipeline alignment where it ascends the steep hillside to the ditch.
- Property Issues Construction would impact a private orchard and would require cooperation of private property owners.
- Alternative 3
 - Access Access to the pump station would likely need to be established directly from U.S. Highway 2 through property owned by Chelan County adjacent to the eastbound bridge approach.
 - Reduced Benefit to PID System Reliability This alternative would discharge to the PID Canal upstream of the most vulnerable section of the Canal, which is a segment of steel pipeline suspended over a slide area downstream of where the delivery to the Canal would be made.
 - Channel Modifications The main flow in the channel is located toward the center of the channel, rather than at the right bank where the pump station would be located. Channel modification would be required to enable diversion to the pump station at low flow rates.
 - Pump Station Construction Pump station construction would likely require excavation of material with large boulders and cobbles.
 - Slope Stability There is high slope stability risk of the proposed pipeline alignment where it ascends the steep hillside to the ditch.
- Alternative 4
 - U.S. Highway 2 Crossing The proposed pipeline would cross under U.S.
 Highway 2 via boring or jacking. The crossing would require coordination with
 WSDOT and would be an expensive installation.

- Reduced Benefit to PID System Reliability This alternative would discharge to the PID Canal upstream of the most vulnerable section of the Canal, which is a segment of steel pipeline suspended over a slide area downstream of where the delivery to the Canal would be made.
- Channel Modifications The main flow in the channel is located toward the far side of the channel, rather than at the right bank where the pump station would be located. Channel modification would be required to enable diversion to the pump station at low flow rates.
- Pump Station Construction Pump station construction would likely require excavation of material with large boulders and cobbles.
- Private Property Construction would impact a private orchard and would require cooperation of private property owners.
- Alternative 5
 - Pump Station Site The pump station site would be located upstream of Dryden Dam. As noted previously, the site is also being evaluated by the Yakama Nation for fish rearing facilities and Chelan PUD currently operates a fish capture facility adjacent to where the proposed pump station would be constructed. Extensive coordination would be required with the Yakama Nation, Chelan PUD, the State, Wenatchee Reclamation District, and other stakeholders to make this site work for a new pump station.
 - U.S. Highway 2 Crossing The proposed pipeline would cross under U.S.
 Highway 2 via boring or jacking. The crossing would require coordination with
 WSDOT and would be an expensive installation.
 - Reduced Benefit to PID System Reliability This alternative would discharge to the PID Canal upstream of the most vulnerable section of the Canal, which is a segment of steel pipeline suspended over a slide area downstream of where the delivery to the Canal would be made.
 - Right-of-way Impacts A portion of the proposed pipeline alignment would follow Deadman Hill Road to the PID Canal from U.S. Highway 2. Chelan County is currently finishing a resurfacing project on Deadman Hill Road. Trenching in the roadway in the near term may not be allowed by the County.

There is a very narrow shoulder, so installation of the pipeline outside the paved roadway would be difficult.

At this point, none of these challenges has been identified as a fatal flaw. It is anticipated that most of these challenges could be addressed with additional engineering analysis, appropriate design elements, coordination with property owners, and coordination with other stakeholders. However, additional review with stakeholders may indicate that some of challenges are effectively fatal flaws because there may not be a reasonable or affordable way to address them. It is recommended that these challenges be reviewed in detail with stakeholders as part of the selection of a preferred alternative, or alternatives, to more clearly determine which challenges cannot be addressed.

8 DESCRIPTION OF PREFERRED ALTERNATIVE(S)

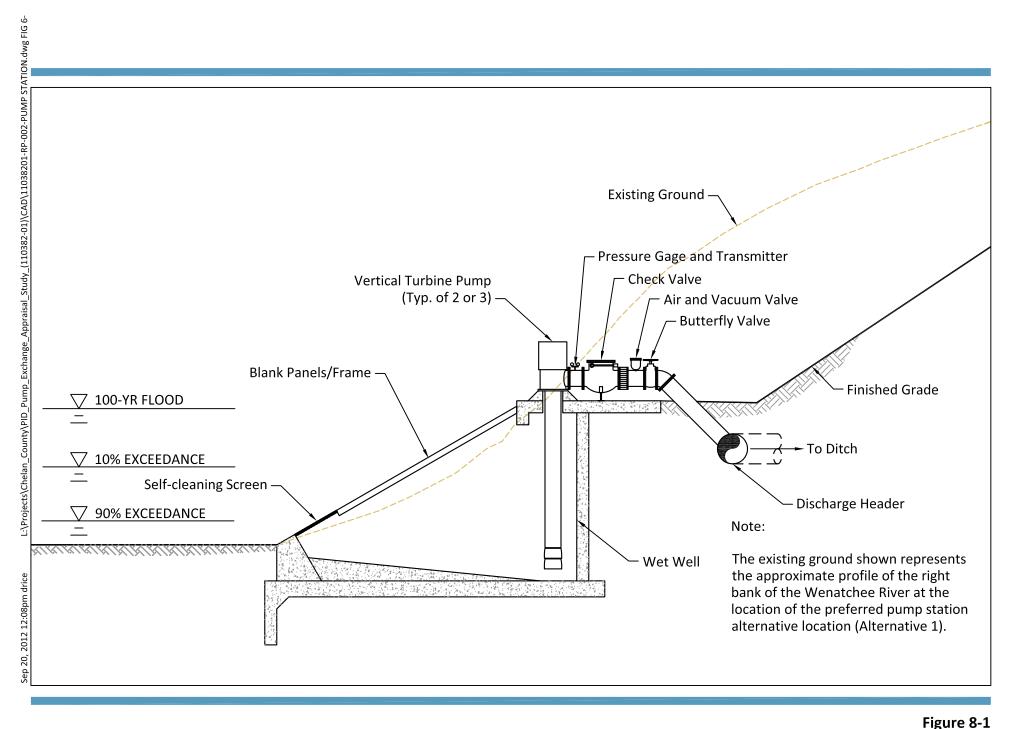
The preliminary alternatives evaluation provided in Sections 1 through 7 of this report were reviewed by PID, Chelan PUD, Chelan County, and Ecology. Based on review of the alternatives with PID and others, Alternative 1 was selected as the preferred project alternative for the following reasons:

- It would provide a connection to the PID system downstream of the slide area, which is probably the most vulnerable section of the PID Canal system. It would increase the reliability of the system by enabling PID to maintain supply to the system in the event that a portion of the ditch or pipeline through the slide area becomes damaged or inoperable.
- It is projected to be one of the less expensive alternatives to construct and maintain.
- Based on the preliminary geomorphic evaluation, the recommended pump station location would likely require little modification and maintenance within the river channel to divert the design flow rate over a range of flow conditions than all of the other alternatives, except Alternative 5.

PID also recommended that Alternative 5 be studied further as a backup to the preferred alternative. Alternative 5 would not provide the same benefit to the PID system's reliability as Alternative 1, because it would discharge to the PID Canal upstream of the most vulnerable section of the canal through the slide area. Alternative 5 would also be the most expensive alternative to construct. However, the pumping costs for Alternative 5 would be the lowest of the alternatives studied. In addition, Alternative 5 would draw water from the pool upstream of Dryden Dam, which would create the most favorable hydraulic conditions for operation.

8.1 Pump Station

Figure 8-1 provides a conceptual section showing what a pump station might look like on the right bank of the Wenatchee River. The following summarizes pump station characteristics for the preferred alternative, Alternative 1, and the backup alternative, Alternative 5.





Conceptual Pump Station Section Peshastin Irrigation District Pump Exchange Appraisal Study Chelan County Natural Resources Department

8.1.1 Alternative 1

As indicated in Section 4.1, the pump station for Alternative 1 would be constructed on the right bank of the Wenatchee River, just southwest (upstream) of U.S. Highway 2, approximately 7,250 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 16.5). The pump station would be constructed just downstream of an exposed bedrock outcropping at the right bank of the river. Pump station characteristics would be as follows:

- Pump station elevation Approximately 922 feet (NAVD 88)
- Pumping head:
 - 244 feet if designed to deliver 10 cfs
 - 240 feet if designed to deliver 20 cfs
 - 250 feet if designed to deliver 40 cfs
- Total horsepower requirement:
 - 400 horsepower if designed to deliver 10 cfs
 - 780 horsepower if designed to deliver 20 cfs
 - 1,620 horsepower if designed to deliver 40 cfs
- Type of pumps Vertical turbine
- Number of pumps Two or three pumps, depending on design flow rate
- Pump station structure Reinforced concrete with screened intake and sump
- Electrical and controls VFDs on each pump, three-phase power service, and a small building to house control panels and equipment
- Other equipment Steel or ductile iron discharge pipe and fittings, a check valve or pump control valve on each pump discharge line, pressure transmitters and switches, air release valves, butterfly valves on each discharge line for isolation, a flow meter, and other appurtenances
- Other notable design constraints and opportunities include:
 - The recommended pump station location would be located just downstream of a bedrock outcropping that extends into the river, creating a pool at the proposed pump station location that should allow for diversion of water over a range of flows with little or no modification to the river.

- Temporary and permanent access to the pump station location will be challenging. The recommended location is along a relatively steep, high river bank and would need to be accessed by crossing an existing orchard. The river bank downstream of the pump station has been eroding.
- Construction of the pump station may require excavation of large rock.
- The pump station would be located downhill of a potential slide area.

8.1.2 Alternative 5

As indicated in Section 4.5, the pump station for Alternative 5 would be constructed on the right bank of the Wenatchee River, just upstream of the Dryden Dam and Chelan PUD fish facility, approximately 700 feet downstream of the confluence of Peshastin Creek with the Wenatchee River (approximately RM 17.8). Pump station characteristics would be as follows:

- Pump station elevation Approximately 971 feet (NAVD 88)
- Pumping head:
 - 219 feet if designed to deliver 10 cfs
 - 210 feet if designed to deliver 20 cfs
 - 222 feet if designed to deliver 40 cfs
- Total horsepower requirement:
 - 360 horsepower if designed to deliver 10 cfs
 - 680 horsepower if designed to deliver 20 cfs
 - 1,440 horsepower if designed to deliver 40 cfs
- Type of pumps Vertical turbine
- Number of pumps Two or three pumps, depending on design flow rate
- Pump station structure Reinforced concrete with screened intake and sump
- Electrical and controls VFDs on each pump, three-phase power service, and a small building to house control panels and equipment
- Other equipment Steel or ductile iron discharge pipe and fittings, a check valve or pump control valve on each pump discharge line, pressure transmitters and switches, an air release valve and butterfly valves on each discharge line for isolation, a flow meter, and other appurtenances

- Other notable design constraints and opportunities include:
 - The recommended pump station location would draw from the pool created by the Dryden Dam. Water levels would be more consistent and very little in river modifications or maintenance would be needed.
 - Permanent access is already available to the site.
 - Extensive coordination would be required with Chelan PUD, which currently operates a fish rearing facility at the site, the Yakama Nation, which has plans for an additional fish rearing facility at the site, and WDFW, which would like to improve portage conditions for recreational rafting at the site.
 - The pump station could potentially share intake facilities with fish rearing facilities currently being planned by the Yakama Nation.
 - The pump station would need to be designed to minimize adverse impacts to current fish operations and passage.

8.2 Fish Screening

Fish screening facilities would be designed to meet the most current requirements for screening of diversions from the WDFW and the NMFS *Anadromous Salmonid Passage Facility Design Guidelines* (NMFS 2008), developed by the NMFS Northwest Region. Those criteria are outlined in Section 2.2.3.

Sizing of fish screening facilities was outlined in Section 5.1 and summarized in Table 5-2. Fish screen calculations are included in Appendix D. An inclined fixed-plate screen was identified as the most likely screening option for all project alternatives. The sizing would depend on the design flow rate and would be the same for either Alternative 1 or Alternative 5. However, Alternative 5 has the potential for sharing intake facilities with a fish rearing facility proposed by the Yakama Nation. If screening and intake facilities were shared, the sizing would need to accommodate flows for both facilities.

For either alternative, fish screen facilities would likely include:

- An inclined fixed-plate screen, consisting of a flat screen supported by the sump structure, inclined at an angle to generally match the slope of the riverbank
- Non-corrosive stainless steel or plastic screen material

• A self-cleaning mechanism, such as an air-burst system or mechanical brush, or an inclined traveling water screen that would rotate on a conveyor to lift debris out of the water for removal from the screen with internal jets or a brush

8.3 Pipeline

Figure 5-1 provides a conceptual illustration of the proposed pipeline alignments for each alternative. The delivery pipeline would be designed according to the criteria outlined in Section 2.2.4. The following summarizes pipeline characteristics for the preferred alternative, Alternative 1, and the backup alternative, Alternative 5.

8.3.1 Alternative 1

As indicated in Section 4.1, the delivery pipeline constructed as part of Alternative 1 would consist of approximately 1,240 feet of pipe. The pipeline would extend from the pump station south and east through an existing orchard, then west up a relatively steep hill to a delivery location at the existing PID Canal. The pipeline characteristics would be as follows:

• Size:

- 20-inch nominal diameter if designed to deliver 10 cfs
- 30-inch nominal diameter if designed to deliver 20 cfs
- 36-inch nominal diameter if designed to deliver 40 cfs
- Pipe material HDPE (for installation on steep slope), ductile iron, or PVC
- Maximum slope Approximately 30 percent
- Minimum recommended cover 30 inches
- Backfill Imported bedding to 6 inches above pipe, native material above bedding
- Other notable design constraints and opportunities include:
 - The exact alignment would need to be selected to minimize impact to the existing orchard.
 - Trenching for pipeline installation could potentially encounter rock. Additional geotechnical exploration is needed to verify subsurface soil conditions.
 - This alternative would require pipeline construction on a steep slope.
 Construction would be challenging.

 A slide area is mapped west of the pipeline alignment. Slope stability risk would be low to moderate.

8.3.2 Alternative 5

As indicated in Section 4.5, the delivery pipeline constructed as part of Alternative 5 would consist of approximately 4,910 feet of pipe. The pipeline would extend south and west from the pump station along the Dryden Dam access road, across U.S. Highway 2, and up Deadman Hill Road to the PID Canal. The pipeline characteristics would be as follows:

- Size:
 - 20-inch nominal diameter if designed to deliver 10 cfs
 - 30-inch nominal diameter if designed to deliver 20 cfs
 - 36-inch nominal diameter if designed to deliver 40 cfs
- Pipe material HDPE, ductile iron, or PVC
- Maximum slope Less than 10 percent
- Minimum recommended cover 30 inches
- Backfill Imported bedding to 6 inches above pipe, native material above bedding
- Other notable design constraints and opportunities include:
 - The alignment would be almost entirely within public roadways or the Dryden Dam Access Road.
 - Trenching for pipeline installation could potentially encounter rock, especially in the Dryden Dam Access Road near Peshastin Creek. Additional geotechnical exploration is needed to verify subsurface soil conditions.
 - Directional drilling, jacking, or boring would likely be required for pipeline installation at U.S. Highway 2.
 - The pipeline would follow Deadman Hill Road from U.S. Highway 2 to the delivery location at the PID Canal. There is limited right-of-way along Deadman Hill Road and the roadway was recently resurfaced by Chelan County.
 - Slope stability risk would be low.

8.4 Delivery to PID Canal

Delivery facilities at the PID Canal would be designed to deliver the design flow conveyed by the pipeline to PID Canal while preventing erosion or damage to canal facilities. For both Alternatives 1 and 5, the delivery location would be at an open section of concrete-lined canal. Delivery facilities would likely include a baffled reinforced concrete structure, constructed within the existing ditch easement.

The reinforced concrete structure for either alternative would feature the following:

- Baffles, diffusers, or a riser designed to dissipate energy and reduce the velocity of water discharged from the end of the pipeline
- A stilling well with a water level transmitter or other device designed to accurate measure the water level for hydraulic control of pump operation
- Access for maintenance and cleaning

8.5 Project Costs

Opinions of probable project costs for each alternative were outlined in Section 6. Opinions of probable costs were developed for project implementation, long-term operating costs, and replacement costs. Table 8-1 summarizes those costs for Alternatives 1 and 5.

Summary of Opinions of Project Costs – Alternatives 1 and 5

		Alternative 1		Alternative 5			
Item	10-cfs	20-cfs	40-cfs	10-cfs	20-cfs	40-cfs	
Project Implementation	n Costs:						
Construction Subtotal	\$1,027,000	\$1,480,000	\$2,160,000	\$1,748,000	\$2,443,000	\$3,425,000	
Other Costs	\$853,000	\$1,207,000	\$1,739,000	\$1,417,000	\$1,960,000	\$2,728,000	
Total Project Cost	\$1,880,000	\$2,687,000	\$3,899,000	\$3,165,000	\$4,403,000	\$6,153,000	
Annual Operating Costs	:						
Annual O&M	\$11,920	\$14,220	\$17,820	\$16,120	\$19,320	\$24,620	
Annual Power Costs							
2-Week Pumping	\$8,658	\$17,026	\$35,219	\$7,789	\$14,853	\$31,307	
8-Week Pumping	\$13,559	\$26,704	\$55,282	\$12,193	\$23,290	\$49,136	
Total Annual Operating	Cost:						
2-Week Pumping	\$20,600	\$31,200	\$53,000	\$23,900	\$34,200	\$55,900	
8-Week Pumping	\$25,500	\$40,900	\$73,100	\$28,300	\$42,600	\$73,800	
Present Value of Projec	t Implementatio	on and Annual C	perating Costs	Through 50-Yea	r Design Life Cy	cle:	
Total Project Cost	\$1,880,000	\$2,687,000	\$3,899,000	\$3,165,000	\$4,403,000	\$6,153,000	
Total Operating Cost							
2-Week Pumping	\$1,028,908	\$1,562,322	\$2,651,966	\$1,195,436	\$1,708,643	\$2,796,344	
8-Week Pumping	\$1,273,955	\$2,046,214	\$3,655,110	\$1,415,669	\$2,130,497	\$3,687,821	
Total							
2-Week Pumping	\$2,908,908	\$4,249,322	\$6,550,966	\$4,360,436	\$6,111,643	\$8,949,344	
8-Week Pumping	\$3,153,955	\$4,733,214	\$7,554,110	\$4,580,669	\$6,533,497	\$9,840,821	
Annual Replacement Fund Costs (50% Replacement) ¹ :							
Fixed Deposit	\$37,466	\$53,518	\$77,673	\$63,118	\$87,700	\$122,562	
Increasing Deposit	•					•	
Year 1	\$19,858	\$28,366	\$41,169	\$33,454	\$46,484	\$64,962	
Year 25	\$40,368	\$57,663	\$83,688	\$68,006	\$94,492	\$132,055	
Year 50	\$84,522	\$120,733	\$175,225	\$142,389	\$197,846	\$276,493	

Notes:

1. These costs assume only 50 percent replacement of the system, as it is not likely that all of the facilities would need to be replaced with the 50-year life cycle.

9 OTHER CONSIDERATIONS

The scope of this appraisal study is limited to the tasks outlined in Section 1.2.1. Other improvements that may warrant consideration as part of the pump exchange project or as part of an overall approach to improving management of water resources within the Peshastin Creek Subbasin include:

- Other potential improvements to the Peshastin Creek channel and floodplain to further improve fish habitat, spawning, and passage.
- Design of pump exchange facilities to also deliver water to IID to reduce late summer diversions from Icicle Creek.

9.1 Fish Habitat and Passage Improvements

An instream flow benefit analysis was completed by Hydrology Northwest as part of the development of this Appraisal Study (see Appendix B). The analysis focused primarily on the benefit of increased late summer flows in Peshastin Creek to fish passage for ESA-listed bull trout and Chinook salmon. The analysis also evaluated WUA, a measure of habitat abundance, versus flow in lower Peshastin Creek. The results indicate that the increased late summer flow resulting from reduced diversions made possible by the proposed pump exchange project would improve passage and habitat abundance in lower Peshastin Creek. The analysis also notes that channel modifications may be needed, in conjunction with increased late summer flows, to improve passage at the worst transects.

Comments provided by WDFW following their review of the draft of this Appraisal Study indicate that a more detailed evaluation of the long-term biological benefits of the proposed pump exchange project may be warranted. WDFW indicated that channel improvements need to be evaluated in the context of existing and potential floodplain values, surface flow connectivity, side channel values, riparian connectivity, and channel meandering values.

It is recommended that additional coordination and discussion with WDFW, CCNRD, Ecology, irrigators, and other stakeholders be included as part of future phases of study for the pump exchange project to develop a more comprehensive strategy for improvements in lower Peshastin Creek. As part of that larger strategy, stakeholders should consider a more detailed evaluation of channel and floodplain improvements designed to further improve fish passage, add habitat complexity, and restore floodplain functions.

9.2 Pumping to Icicle Irrigation District

CCNRD, irrigators, and other stakeholders may also want to give consideration to designing the pump exchange project to reduce late summer diversions from Icicle Creek. The PID pump exchange project could potentially be designed to reduce late summer diversions from Icicle Creek by replacing flows delivered to PID at the bifurcation on the IID Division 2 Canal and/or expanding the pump exchange project to include facilities that would deliver water directly to the IID Division 3A Canal from the Wenatchee River.

As was noted previously, IID diverts water for irrigation from lower Icicle Creek. PID also has a water right on Icicle Creek and jointly operates the diversion facilities at Icicle Creek and the IID Division 1 and 2 Canals with IID. Up to 14 cfs has historically been conveyed from the bifurcation structure at the downstream end of the IID Division 2 Canal to the PID Canal to supplement PID's diversions from Peshastin Creek. PID relies on the additional supply from the bifurcation to meet irrigation demands during the late summer when low flows limit PID's diversions from Peshastin Creek. In addition to reducing diversions from Peshastin Creek, the pump exchange project could also be used to replace water delivered to PID from the bifurcation, which would reduce diversions from Icicle Creek.

9.2.1 Description of Potential IID Pump Exchange

The IID Division 3A and 3B Canals run parallel to the PID Canal along the hillsides south of the Wenatchee River Valley and are approximately 160 to 170 feet higher than the PID Canal. IID conveys up to 30 cfs from the bifurcation at the downstream end of the Division 2 Canal through the Peshastin Siphon to the Division 3A Canal. The PID pump exchange project could be expanded to also deliver flows to the IID Division 3A Canal directly from the Wenatchee River, reducing the flow that would need to be diverted from Icicle Creek. The following provides a preliminary analysis of the additional facilities that would be required.

9.2.1.1 Pumping Location and System Configuration

Because the IID Division 3A Canal is approximately 160 to 170 feet higher in elevation than the PID Canal, delivery of water to the IID Canal from a pump station along the Wenatchee River would require additional pumping. Three potential pumping configurations were identified for delivering water to the IID Canal:

- Install a pump station on the Wenatchee River with pumps sized (in terms of pumping head) to deliver water to the elevation of the PID Canal, as described in Section 8.1. Install a booster pump station adjacent to the PID Canal sized to deliver water from the PID Canal up to the IID Division 3A Canal.
- Install a pump station on the Wenatchee River that would deliver water to both the PID Canal and the IID Division 3A Canal. The pumps would be sized (in terms of pumping head) to deliver water through a single discharge pipeline to the elevation of the IID Canal.
- Install a pump station on the Wenatchee River with two sets of pumps and two separate, parallel discharge pipelines. One set of pumps and discharge pipeline would be sized (in terms of pumping head) to deliver water to the elevation of the PID Canal and the other would be sized to deliver water to the elevation of the IID Canal.

The first configuration would be the most efficient. The pumps and pipe would not have to be oversized and less power would be required than for the other configurations. However, three phase power would have to be extended to both the booster pump station and the river pump station site. Operating pumps at different locations could also be a challenge.

The second configuration would result in over sizing of pumps and wasted pumping energy. For example, if the system had to deliver water to both canals at the same time, the deliveries to PID would have to be pumped (in terms of pumping head) more than 170 feet higher than needed to deliver water to the elevation of the PID Canal. The benefit of this configuration is that it would include a consolidated pump station location and a single discharge line.

The third configuration would essentially result in two separate systems built adjacent to one another. This approach would keep all of the pumping in one location and would minimize

the over sizing and inefficiencies noted for the second configuration. However, the cost of two parallel discharge pipelines would be high.

This analysis assumes that pumping to IID would match the first configuration. The initial cost of extending three-phase power to the pump stations would be higher than for the other potential configurations, but the cost of pumps, piping, and power would be lower.

9.2.1.2 Flow Rates

In order to evaluate a range of potential pumping and delivery conditions, the pump station evaluation in Section 5.1 of this study was completed for pump station flow rates of 10, 20, and 40 cfs. A more detailed operational evaluation would be needed as part of future study to more clearly define pump station flow rates as they relate to water supply needs and late summer flows in Peshastin and Icicle Creeks. For the sake of completing this preliminary analysis, the following pump station flow rates were evaluated:

- A 10-cfs Wenatchee River Pump Station with a 10-cfs booster pump station at the PID Canal. The maximum combined delivery to PID Canal and IID Division 3A would be 10 cfs. The system would be capable of delivering up to 10 cfs to either canal, or some combination of deliveries that total 10 cfs.
- A 20-cfs Wenatchee River Pump Station with a 20-cfs booster pump station at the PID Canal. The maximum combined delivery to PID Canal and IID Division 3A would be 20 cfs. The system would be capable of delivering up to 20 cfs to either canal, or some combination of deliveries that total 20 cfs.
- A 40-cfs Wenatchee River Pump Station with a 40-cfs booster pump station at the PID Canal. The maximum combined delivery to PID Canal and IID Division 3A would be 40 cfs. The system would be capable of delivering up to 40 cfs to either canal, or some combination of deliveries that total 40 cfs.

9.2.1.3 Hydraulic Analysis

A preliminary hydraulic analysis was completed to determine the size of pumps, pipe, and other facilities needed to deliver water to the IID Division 3A Canal from the PID Canal. The analysis was completed for the preferred alternative (Alternative 1) and the backup preferred alternative (Alternative 5) as outlined in Section 8. The conceptual layout of the additional facilities that would be needed for these alternatives is shown in Figure 9-1. The detailed results of the analysis are included in Appendix H. The same assumptions and method were used for this analysis as was used for the evaluation of preliminary alternatives described in Section 5.1. The results are summarized in Table 9-1.

Design Parameter	Alternative 1	Alternative 5		
Pump Sump Elevation (feet)	1,146	1,158		
Discharge Elevation (feet)	1,318	1,320		
Elevation Gain (feet)	172	162		
Delivery Pipe ¹ Length (feet)	2,250	2,700		
Delivery Pipe ¹ Size (inches)				
At 10 cfs P.S. Capacity	20	20		
At 20 cfs P.S. Capacity	30	30		
At 40 cfs P.S. Capacity	36	36		
Total Headloss ² (feet)				
At 10 cfs P.S. Capacity	24	25		
At 20 cfs P.S. Capacity	19	19		
At 40 cfs P.S. Capacity	28	29		
Pumping Head, TDH (feet)				
At 10 cfs P.S. Capacity	196	187		
At 20 cfs P.S. Capacity	191	181		
At 40 cfs P.S. Capacity	200	191		
Number of Pumps				
At 10 cfs P.S. Capacity	2	2		
At 20 cfs P.S. Capacity	3	3		
At 40 cfs P.S. Capacity	3	3		
Flow Rate per Pump (cfs)				
At 10 cfs P.S. Capacity	5.00	5.00		
At 20 cfs P.S. Capacity	6.67 6.67			
At 40 cfs P.S. Capacity	13.33	13.33		

Table 9-1Summary of Hydraulic Analysis Results – Pumping to IID Canal

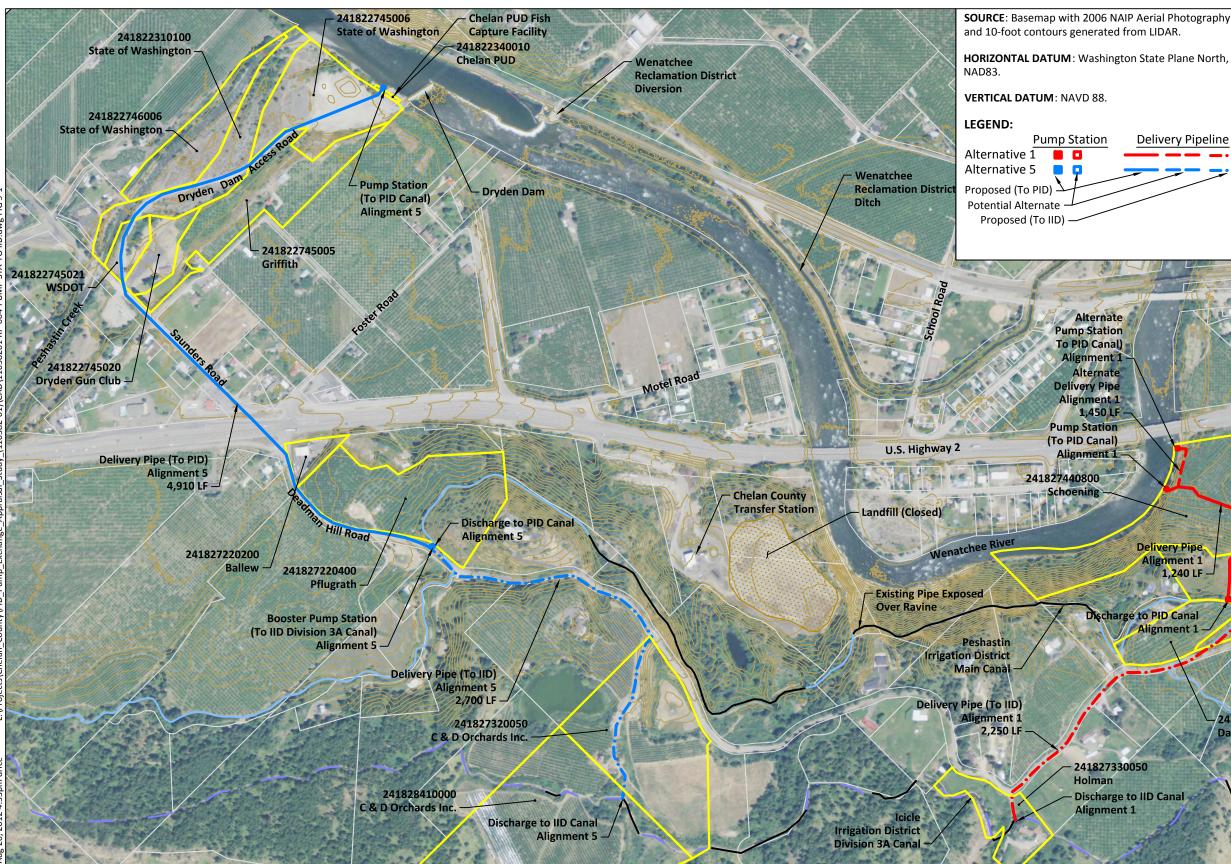
Notes:

1. Delivery Pipe includes transmission pipeline from the pump station to the delivery at the PID Canal.

2. Total headloss includes both friction and minor losses through the pump station and delivery pipeline.

cfs = cubic feet per second

TDH = Total Dynamic Head, or Pumping Head



CANCHOR QEA

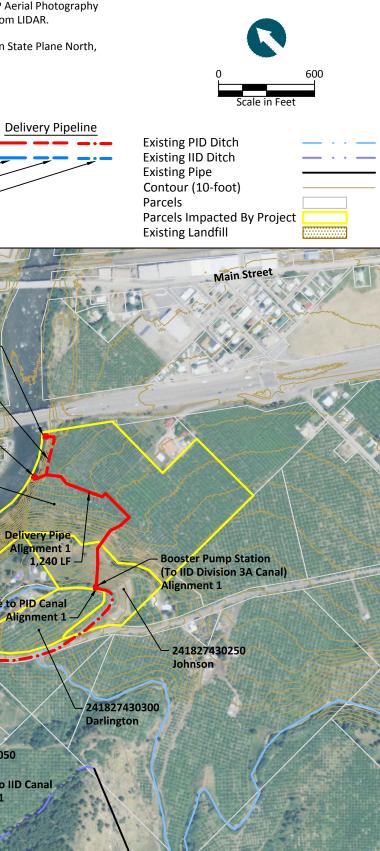


Figure 9-1

Preferred Alternatives With Pumping to IID Division 3A Canal Peshastin Irrigation District Pump Exchange Appraisal Study **Chelan County Natural Resources Department**

9.2.1.4 Description of Additional Facilities Required

The conceptual layout of additional facilities needed to deliver water from the PID Canal to the IID Division 3A Canal for Alternatives 1 and 5 is shown in Figure 9-1. Additional facilities would include a booster pump station at the PID Canal, additional discharge pipeline to connect to the IID Division 3A Canal, and a delivery facility at the IID Canal.

ALTERNATIVE 1

The following is a more detailed description of the major facilities needed to extend deliveries to the IID Division 3A Canal for Alternative 1.

Pump Station

- Elevation Approximately 1,146 feet (NAVD 88)
- Location On PID Canal approximately 19,560 feet downstream of the diversion on Peshastin Creek, adjacent to the delivery to the PID Canal
- Pumping head, or TDH As indicated in Table 9-1
- Total horsepower requirement
 - 320 horsepower if designed to deliver 10 cfs
 - 620 horsepower if designed to deliver 20 cfs
 - 1,300 horsepower if designed to deliver 40 cfs
- Type of pumps Vertical turbine or centrifugal
- Number of pumps 2 or 3 pumps, depending on design flow rate
- Configuration In-line boosters with hydraulic connection on suction side to delivery structure to PID Canal.
- Electrical and controls VFDs on each pump, 3-phase power service, and a small building to house control panels and equipment
- Other equipment Steel or ductile iron suction and discharge pipe and fittings, a check valve or pump control valve on each pump discharge line, pressure transmitters and switches, air release valves, butterfly valves on each discharge line for isolation, a flow meter, and other appurtenances
- Discharge Pipeline
 - Length Approximately 2,250 feet

- Alignment South and west along PID Canal to Deadman Hill Road, west in Deadman Hill Road, and up a private driveway to the IID Division 3A Canal
- Diameter As shown in Table 9-1
- Pipe material HDPE, ductile iron, or PVC
- Maximum slope Approximately 23 percent
- Minimum recommended Cover 30 inches
- Backfill Imported bedding to 6 inches above pipe, native material above bedding

Delivery at IID Division 3A Canal

• Baffled reinforced concrete structure constructed within the ditch easement or other device designed to dissipate energy prior to discharging to the open canal

ALTERNATIVE 5

The following is a more detailed description of the major facilities needed to extend deliveries to the IID Division 3A Canal for Alternative 5.

Pump Station

- Elevation Approximately 1,158 feet (NAVD 88)
- Location On PID Canal approximately 12,860 feet downstream of the diversion on Peshastin Creek, adjacent to the delivery to the PID Canal
- Pumping head, or TDH As indicated in Table 9-1
- Total horsepower requirement
 - 305 horsepower if designed to deliver 10 cfs
 - 590 horsepower if designed to deliver 20 cfs
 - 1,240 horsepower if designed to deliver 40 cfs
- Type of pumps Vertical turbine or centrifugal
- Number of pumps 2 or 3 pumps, depending on design flow rate
- Configuration In-line boosters with hydraulic connection on suction side to delivery structure to PID Canal.
- Electrical and controls VFDs on each pump, 3-phase power service, and a small building to house control panels and equipment

• Other equipment – Steel or ductile iron suction and discharge pipe and fittings, a check valve or pump control valve on each pump discharge line, pressure transmitters and switches, air release valves, butterfly valves on each discharge line for isolation, a flow meter, and other appurtenances

Discharge pipeline

- Length Approximately 2,700 feet
- Alignment Southeast along Deadman Hill Road and southwest up a private driveway to the IID Division 3A Canal
- Diameter As shown in Table 9-1
- Pipe material HDPE, ductile iron, or PVC
- Maximum slope Approximately 50 percent
- Minimum recommended cover 30 inches
- Backfill imported bedding to 6 inches above pipe, native material above bedding

Delivery at IID Division 3A Canal

• Baffled reinforced concrete structure constructed within the ditch easement or other device designed to dissipate energy prior to discharging to the open canal

ALTERNATIVES 1 AND 5

The following design constraints, opportunities, and limitations apply to both alternatives:

- Temporary and permanent access to the pump station location will require improvement of existing access along ditch bank.
- Construction of the discharge pipeline may require excavation of large rock.
- The discharge pipeline would mostly be constructed within public roadways.
- There is limited right-of-way along Deadman Hill Road and the roadway was recently resurfaced by Chelan County.
- The site has not been reviewed to determine whether the pump station location, pipeline alignment, and delivery location are feasible. Additional geotechnical exploration and other site investigations are needed to verify site constraints.

9.2.1.5 Additional Property Impacts

Extension of the project to deliver water to the IID Division 3A Canal would impact additional properties. In addition to those properties listed in Table 5-3, extending Alternative 1 to deliver water to the IID Canal would also impact one private parcel listed in Table 9-2. In addition to the properties listed in Table 5-7, extending Alternative 5 to deliver water to the IID Canal would also impact two private properties listed in Table 9-3. For both alternatives, installation of a discharge pipeline from the PID Canal to the IID Division 3A Canal would impact the Deadman Hill Road right-of-way. As was noted previously, Deadman Hill Road was recently resurfaced.

Table 9-2Properties Impacted by Alternative 1 – Extension to IID

Parcel	Owner	Address	Impacted/Adjacent
241827330050	Don Holman	90732 Deadman Hill Road Cashmere, WA 98815	Impacted

Table 9-3Properties Impacted by Alternative 5 – Extension to IID

Parcel	Parcel Owner Ac		Impacted/Adjacent
241827320050	C & D Orchards, Inc.	PO Box 578 Peshastin, WA 98847	Impacted
241828410000	C & D Orchards, Inc.	PO Box 578 Peshastin, WA 98847	Impacted

9.2.1.6 Description of Additional Costs

The additional costs for the facilities needed to extend deliveries to the IID Division 3 A Canal are included in Table 9-4. The totals for pumping to both PID (from Table 8-1) and IID (Table 9-4) are summarized in Table 9-5. The same assumptions and methodology were used to develop these costs as were outlined in Section 6. More detailed cost information, including a list of major items, estimated quantities, and unit costs used to develop the opinions of cost is included in Appendix H.

Table 9	9-4
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Summary of Additional Costs for Delivery to IID – Alternatives 1 and 5

		Alternative 1		Alternative 5				
Item	10-cfs	20-cfs	40-cfs	10-cfs	20-cfs	40-cfs		
Project Implementation	Costs:							
Construction Subtotal	\$897,000	\$1,354,000	\$1,843,000	\$969,000	\$1,451,000	\$2,382,000		
Other Costs	\$751,000	\$1,109,000	\$1,491,000	\$808,000	\$1,185,000	\$1,913,000		
Total Project Cost	\$1,648,000	\$2,463,000	\$3,334,000	\$1,777,000	\$2,636,000	\$4,295,000		
Annual Operating Costs:								
Annual O&M	\$11,920	\$14,220	\$17,820	\$16,120	\$19,320	\$24,620		
Annual Power Costs								
2-Week Pumping	\$7,028	\$13,549	\$28,329	\$6,702	\$12,897	\$27,025		
8-Week Pumping	\$10,998	\$21,241	\$44,459	\$10,486	\$20,217	\$42,410		
Total Annual Operating Cost:								
2-Week Pumping	\$18,900	\$27,800	\$46,100	\$22,800	\$32,200	\$51,600		
8-Week Pumping	\$22,900	\$35,500	\$62,300	\$26,600	\$39,500	\$67,000		
Present Value of Projec	t Implementatio	on and Annual C	perating Costs	Through 50-Yea	r Design Life Cy	cle:		
Total Project Cost	\$1,648,000	\$2,463,000	\$3,334,000	\$1,777,000	\$2,636,000	\$4,295,000		
Total Operating Cost								
2-Week Pumping	\$947,398	\$1,388,436	\$2,307,454	\$1,141,096	\$1,610,832	\$2,582,246		
8-Week Pumping	\$1,145,918	\$1,773,067	\$3,113,939	\$1,330,310	\$1,976,852	\$3,351,509		
Total								
2-Week Pumping	\$2,595,398	\$3,851,436	\$5,641,454	\$2,918,096	\$4,246,832	\$6,877,246		
8-Week Pumping	\$2,793,918	\$4,236,067	\$6,447,939	\$3,107,310	\$4,612,852	\$7,646,509		
Annual Replacement Fu	ind Costs (50% F	Replacement) ¹ :						
Fixed Deposit	\$32,958	\$49,165	\$66,693	\$36,145	\$53,362	\$86,456		
Increasing Deposit	•			•				
Year 1	\$17,469	\$26,059	\$35,350	\$19,158	\$28,284	\$45,825		
Year 25	\$35,510	\$52,973	\$71,858	\$38,944	\$57,495	\$93,152		
Year 50	\$74,351	\$110,913	\$150,456	\$81,541	\$120,382	\$195,040		

Notes:

1. These costs assume only 50 percent replacement of the system, as it is not likely that all of the facilities would need to be replaced with the 50-year life cycle.

Table 9-5
Summary of Costs for Delivery to Both PID and IID – Alternatives 1 and 5

		Alternative 1			Alternative 5			
Item	10-cfs	20-cfs	40-cfs	10-cfs	20-cfs	40-cfs		
Project Implementation	Costs:							
Construction Subtotal	\$1,924,000	\$2,834,000	\$4,003,000	\$2,717,000	\$3,894,000	\$5,807,000		
Other Costs	\$1,604,000	\$2,316,000	\$3,230,000	\$2,225,000	\$3,145,000	\$4,641,000		
Total Project Cost	\$3,528,000	\$5,150,000	\$7,233,000	\$4,942,000	\$7,039,000	\$10,448,000		
Annual Operating Costs:								
Annual O&M	\$23,840	\$28,440	\$35,640	\$32,240	\$38,640	\$49,240		
Annual Power Costs								
2-Week Pumping	\$15,686	\$30,575	\$63,548	\$14,491	\$27,750	\$58,332		
8-Week Pumping	\$24,557	\$47,945	\$99,741	\$22,679	\$43,507	\$91,546		
Total Annual Operating	Cost:							
2-Week Pumping	\$39,500	\$59,000	\$99,100	\$46,700	\$66,400	\$107,500		
8-Week Pumping	\$48,400	\$76,400	\$135,400	\$54,900	\$82,100	\$140,800		
Present Value of Project	t Implementatio	on and Annual C	Operating Costs	Through 50-Yea	r Design Life Cy	cle:		
Total Project Cost	\$3,528,000	\$5,150,000	\$7,233,000	\$4,942,000	\$7,039,000	\$10,448,000		
Total Operating Cost								
2-Week Pumping	\$1,976,306	\$2,950,758	\$4,959,420	\$2,336,532	\$3,319,475	\$5,378,590		
8-Week Pumping	\$2,419,873	\$3,819,281	\$6,769,049	\$2,745,979	\$4,107,349	\$7,039,330		
Total			•		•			
2-Week Pumping	\$5,504,306	\$8,100,758	\$12,192,420	\$7,278,532	\$10,358,475	\$15,826,590		
8-Week Pumping	\$5,947,873	\$8,969,281	\$14,002,049	\$7,687,979	\$11,146,349	\$17,487,330		
Annual Replacement Fu	nd Costs (50% F	Replacement) ² :						
Fixed Deposit	\$70,424	\$102,683	\$144,366	\$99,263	\$141,062	\$209,018		
Increasing Deposit			•					
Year 1	\$37,327	\$54,425	\$76,519	\$52,612	\$74,768	\$110,787		
Year 25	\$75,878	\$110,636	\$155,546	\$106,950	\$151,987	\$225,207		
Year 50	\$158,873	\$231,646	\$325,681	\$223,930	\$318,228	\$471,533		

Notes:

1. Includes total of costs from Table 8-1 and Table 9-4.

2. These costs assume only 50 percent replacement of the system, as it is not likely that all of the facilities would need to be replaced with the 50-year life cycle.

10 SUMMARY AND RECOMMENDATIONS

10.1 Preliminary Alternatives Evaluation

The purpose of this Appraisal Study is to provide a preliminary evaluation of project alternatives to pump water from the Wenatchee River to the PID Canal, assess whether the alternatives are viable, select one or two preferred project alternatives, and recommend steps for additional study and implementation of the project. Five alternatives were identified and evaluated as part of this appraisal study, each including a pump station on the right bank of the Wenatchee River, a delivery pipeline from the pump station to the PID Canal, and delivery facilities at the PID Canal. The alternatives were evaluated by completing preliminary hydraulic analyses, geomorphic evaluations of proposed pump station sites, geologic reviews of pipeline alignments, reviews of property impacts, environmental and permitting reviews, and opinions of probable cost. The five preliminary alternatives for pumping from the Wenatchee River to the PID Canal were characterized as follows:

- Alternative 1 would have the lowest cost (\$1.88 to \$3.90 million depending on pump capacity). The pump station site is more suitable for diverting flows under low flow conditions than the other alternatives, except Alternative 5. However, pump station access would be a challenge, a private orchard would be impacted, and the pipeline would be steep and difficult to construct.
- Alternative 2 would have the third lowest cost (\$2.13 to \$4.43 million depending on pump capacity). The pump station site would be more accessible than Alternative 1. However, channel modifications would likely be required for diversion from the river, the pipeline would cross U.S. Highway 2 and a private orchard, and the pipeline would be steep and difficult to construct.
- Alternative 3 would have the second lowest cost (\$1.86 to \$3.97 million depending on pump capacity). This alternative would have little impact to private property. However, the Dryden Transfer Station would be impacted, channel modifications would likely be required for diversion from the river, the pipeline would be steep and difficult to construct, and slope stability risk would be high.
- Alternative 4 would have the second highest cost (\$2.76 to \$5.36 million depending on pump capacity). The pipeline alignment is more favorable from a construction standpoint. However, channel modifications would likely be required for diversion from the river, pump station access could be challenging, a private orchard would be

impacted, the pipeline would have to cross U.S. Highway 2, and the overall cost would be relatively high.

• Alternative 5 offers the most favorable pumping conditions, best pump station access, lowest power requirements, and least impact to private property. However, this alternative would have the highest cost (\$3.17 to \$6.15 million depending on pump capacity), would require coordination with other existing and proposed uses of the pump station site, the delivery pipeline would need to cross U.S. Highway 2, and pipeline construction would impact a newly paved roadway.

10.2 Preferred Alternative

Following the evaluation of the proposed alternatives, the evaluation was presented to PID, Chelan PUD, Chelan County, and Ecology. Based on the review of preliminary alternatives with PID and others, Alternative 1 was selected as the preferred project alternative because of more favorable hydraulic conditions at the proposed diversion location, a lower projected project cost, and the potential for improving the reliability of the PID system by providing an alternate source of supply downstream of the most vulnerable part of the system.

PID also recommended that Alternative 5 be studied further as a backup to the preferred alternative. Alternative 5 would not provide the same benefit to the PID system's reliability as Alternative 1 and would have the highest implementation costs of the project alternatives evaluated. However, Alternative 5 would provide the most favorable hydraulic conditions at the proposed diversion location and would likely have the least impact on private property.

10.3 Other Considerations

Two additional items that were not included initially in the scope of this study that may warrant future consideration include:

- Other potential improvements to the Peshastin Creek channel and floodplain to further improve fish habitat, spawning, and passage
- Design of pump exchange facilities to also deliver water to IID to reduce late summer diversions from Icicle Creek

A preliminary evaluation of additional facilities that would be needed to also delivery water to the IID Division 3A Canal was completed. The evaluation indicated that additional facilities would likely include a booster pump station on the PID Canal, additional discharge pipeline between the PID Canal and the IID Diversion 3A Canal, and a delivery facility at the IID Canal. The system could be designed with the flexibility to deliver a combination of flow rates to the PID and IID canals. For Alternative 1, the additional facilities would add \$1.65 million to \$3.33 million to the total project implementation costs, depending on pump capacity. For Alternative 5, the additional facilities would add \$1.77 million to \$4.30 million to the total project implementation costs, depending on pump capacity. The additional facilities would also increase the operating costs. Additional analysis is recommended to determine the need for reducing diversions from Icicle Creek, identify the most efficient method of reducing diversions, and refine the preliminary analysis and opinions of cost presented in this report.

10.4 Data Gaps

The evaluation of preliminary project alternatives completed as part of this appraisal study is based on visual observations of site conditions made during brief site visits, experience with similar projects, geology mapping, LiDAR data, stream flow data, diversion data, and other available data. No detailed site investigations were completed as part of this study. Additional data would need to be collected as part of further study and detailed design of the preferred project alternative, including:

- Topographic survey data
- River stage and flow data specific to preferred pump station locations
- Geotechnical field data and subsurface soil test results
- Field surveys of aquatic and terrestrial habitat

10.5 Recommendations for Additional Study and Implementation

It is recommended that Alternatives 1 and 5 be studied in more detail by gathering the additional information noted in Section 9.3 and completing a detailed feasibility study of these alternatives. Completion of the following tasks is recommended as part of the feasibility study:

- Detailed evaluation of pump station operations Prepare a detailed evaluation of pump station operations and more clearly define operating conditions (design flow rate, timing, and duration). Provide analysis of operational implications on irrigation supplies from other sources, including diversions from Icicle Creek conveyed through the IID Canal. Hold meetings with PID and IID to discuss operations and narrow down design flows and operating conditions to be used for design of the project.
- Property owner coordination Work with CCNRD to schedule and carry out meetings for project review and coordination with private property owners that will be impacted by the project, both as a group and individually.
- Additional evaluation of habitat improvements in Peshastin Creek Provide further evaluation of the benefits of the proposed project on fish passage and habitat. Coordinate with regulatory agencies to develop a more comprehensive strategy for improving habitat in the Peshastin Creek Subbasin, which would include increasing flow s and also improving channel and floodplain functions.
- Additional site investigations Complete detailed site investigations including:
 - Topographic Survey Complete a topographic survey of the preferred pump station location. Survey river levels during the course of an irrigation season to obtain a rating curve of the water surface elevation versus discharge.
 - Geotechnical Exploration Complete geotechnical field investigations and prepare a report documenting soil conditions for the proposed pump station site and pipeline alignment.
- Detailed environmental and permitting review Complete a more detailed review of permit requirements and environmental resources. Complete field surveys of critical aquatic and terrestrial habitat, as well as a cultural resources survey.
- Additional engineering analyses and cost estimates Develop the engineering design analysis, drawings, and cost estimates to the level of detail needed to support the feasibility study. The work should include:
 - Engineering analyses Refine sizing calculations and identify major materials required, including structures, fish screens, pumps, piping, valves, appurtenances, electrical power supply, and control equipment. Develop feasibility-level (approximately 30 percent complete) drawings.

- Refined cost analysis Refine the preliminary opinions of probable costs for the preferred alternative based on the additional engineering analyses.
- Prepare a Feasibility Study report Prepare a report to summarize the detailed analysis of the detailed operational evaluation, property owner coordination issues, detailed site investigations, detailed environmental and permitting review, additional engineering analyses, and refined opinions of probable cost. Include feasibility-level (30 percent complete) drawings and applicable design information.

It is anticipated that this additional work will provide a basis for determining whether the preferred alternative (Alternative 1) can be implemented, or if the backup alternative, such as Alternative 5, should be pursued. It is also anticipated that the recommended feasibility study will provide the information needed to pursue funding for detailed design, permitting, construction, and operation of the project.

In conclusion, the proposed pump exchange project has the potential to reduce later summer diversions from Peshastin Creek and improve overall management of water in the Peshastin Creek subbasin. With additional pumping and conveyance facilities, the project could be expanded to also reduce diversions from Icicle Creek. The project also has the potential to add reliability to the PID system and improve management of supply to the PID system from Icicle Creek via the Icicle Irrigation District system. The alternatives evaluation summarized in this report led to the selection of a preferred project alternative, Alternative 1. It is also recommended that a backup project alternative, Alternative 5, be carried forward for further consideration.

11 REFERENCES

- Anchor Environmental, LLC, 2007. Peshastin Subbasin Needs and Alternatives Study.
 Prepared for Chelan County Natural Resources Department under Washington State
 Department of Ecology Grant G0700003. January.
- Anchor QEA, LLC, 2010. Campbell Creek Reservoir Feasibility Study. Prepared for Chelan County Natural Resources Department under Washington State Department of Ecology Grant G0900155. October.
- EES Consulting, 2005. Final Technical Report Lower Wenatchee River PHABSIM Studies.
- Franklin, J.F. and C.T. Dyrness. 1998. Natural Vegetation of Oregon and Washington. Reprinted with new bibliographic supplement by Oregon State University Press. 452 pages.
- Hydrology Northwest, 2012. *Lower Peshastin Creek Fish Habitat and Passage Assessment 2011.* Prepared for Chelan County Natural Resources Department.
- Klohn, L., 1993. *Peshastin Irrigation District Comprehensive Water Conservation Plan*. Prepared for Peshastin Irrigation District. May.
- National Marine Fisheries Service (NMFS), 2008. *Anadromous Salmonid Passage Facility Design Guidelines*. February.
- Thompson, K., 1972. *Determining Stream Flows for Fish*. Presented at Instream Flow Requirement Workshop, Pacific Northwest River Basins Commission, March 1972.

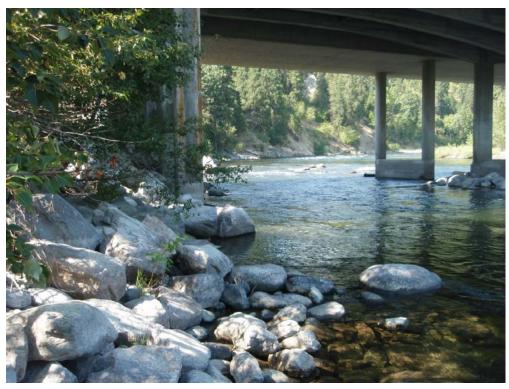
APPENDIX A PHOTOGRAPHS OF EXISTING SITE



Alternative 1 – Pump Station Location



Alternative 1 – Alternate Pump Station Location



Alternative 2 – Pump Station Location



Alternative 1 and 2 – Pipeline Alignment



Alternative 1 and 2 – Pipeline Alignment



Alternative 2 – Pipeline Alignment (Under U.S. Highway 2)



Alternative 3 – Pump Station Location



Alternative 3 – Pipeline Alignment



Alternative 3 – Closed Landfill



Alternative 3 – PID Canal Delivery Location



Alternative 4 – Pump Station Location



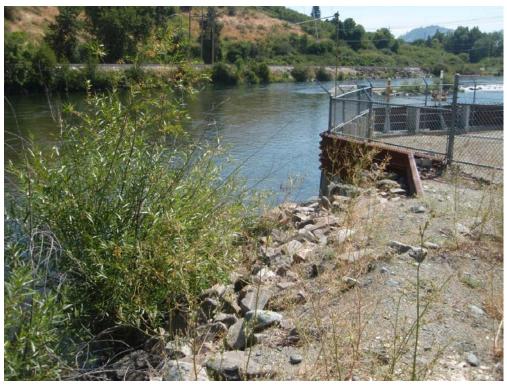
Alternative 4 – Pump Station Location



Alternative 4 – PID Canal Delivery Location



Alternative 4 – PID Canal Delivery Location



Alternative 5 – Pump Station Location



Alternative 5 – Pipeline Alignment



Alternative 5 – Pipeline Alignment



Alternative 5 – PID Canal Delivery Location

APPENDIX B INSTREAM FLOW ANALYSIS

LOWER PESHASTIN CREEK FISH HABITAT AND PASSAGE ASSESSMENT 2011

1.0 INTRODUCTION

This report presents an analysis of potential flow benefits for selected salmonid species of the proposed Peshastin Irrigation District (PID) pump exchange project. The project would provide an alternate supply of water for PID by pumping water to the PID Canal from the Wenatchee River. PID provides water for irrigation and diverts up to 40 cubic feet per second (cfs) from Peshastin Creek, approximately 2.4 miles upstream of its confluence with the Wenatchee River. This Appraisal Study was funded under Grant No. G1100240 from the Columbia Basin Water Development Account administered by the Washington Department of Ecology's Office of the Columbia River.

Increased flows during the summer months may be beneficial to salmonids due to increased rearing habitat and could improve upstream fish passage conditions in mid to late summer. An initial analysis of flow requirements for adult fish passage in Lower Peshastin Creek was presented in *Peshastin subbasin needs and alternatives study* (Anchor Environmental and EES Consulting, 2007).

1.1 Study Purpose

The purpose of this study is to provide data on the benefits of increasing flows to assess habitat and provide upstream passage for migrating Chinook salmon, bull trout, and steelhead in lower Peshastin Creek.

2.0 Methods

Two methods were used to complete the information needs for this study. First, the "Oregon method" was used to estimate preferred upstream passage stream flows for Chinook salmon and Bull trout. After 10 years of research on depth and velocity in streams in Oregon, Thompson (1972) concluded that the depth over "the shallow bars most critical of adult passage" was the feature that determined the likelihood of successful migration. Thompson's recommended minimum depths of 0.8 feet for Chinook and 0.6 feet for large trout to achieve successful passage, have been used by biologists in the Northwest since the 1970's. The "Oregon method" concludes that the passage flow is adequate when the depth criteria is met on at least 25% of the transect width <u>and</u> on at least a 10% continuous portion. Rather than relying on individual transects Thompson recommends the average flow of all transects.

Secondly, weighted usable area (WUA) was computed for the lower 2.4 miles of Peshastin Creek by utilizing the calibrated transects from the Peshastin Creek Instream flow study a part of the Final Technical Report Lower Wenatchee River PHABSIM Studies (EES Consulting, 2005). Transects from the 2005 study were given a different transect weighting in order to have them represent the frequency of habitat types in the lower 2.4 miles of Peshastin Creek.

2.1 Field Measurements

Fish Passage

In August, 2011 a stream survey was completed on lower Peshastin Creek from the Peshastin irrigation district diversion, downstream to the Wenatchee River. Potential study transects on shallow bars were flagged and noted on a map for inclusion in the passage study. Five transects were selected for field study. The transects were selected on both representative and critical and shallow bars in lower Peshastin Creek. Figure 1 shows the location of all transects.

Cross sections were surveyed at each transect. Head pins on each bank as well as a bench mark were surveyed to establish elevations. A tape was stretched horizontally across the channel and attached to the head pins. With an auto level and stadia rod, elevation of the stream bed and banks were surveyed at regular intervals along the tape and water surface elevations were surveyed at locations where accurate measurements could be obtained at all flows. Water depth was also measured at each station in order to cross check the bed and water surface elevation measurements.

Transects were surveyed at high, medium and low stream flows. At each flow level discharge measurements were taken so a rating curve could be computed for each transect. Discharges were taken above and below significant areas of inflow (e.g. Icicle pipeline) to account for changes in discharge. Recorded stage measurements from the gage on Peshastin Creek at Green Bridge, operated by the Department of Ecology, were examined to determine flow changes during the day of the measurements. Table 1 shows the dates and computed discharges for each transect during the field study.

TABLE 1 DISCHARGES FOR TRANSECTS AT LOWER PESHASTIN CREEK								
Date	T-1	T-2	T-3	T-4	T-5			
August 25, 2011	37.4	36.4	36.4	36	36			
September 7, 2011	23.5	23.5	24	24	24			
September 19, 2011	16	15.5	15.2	14.7	14.7			

IFIM Transect Weighting

In September 2011 Lower Peshastin Creek was surveyed from the confluence with the Wenatchee River upstream to the PID diversion dam. The habitat type was evaluated every 100 feet based on the same habitat types used in the original 2005 IFIM report. Transect weighting based on relative frequencies of habitat types are shown in Table 2.

Table 2 Transect Weighting for Lower Peshastin Creek							
Transect No.	Description	Description Percent					
1	Boulder Run		28.5%				
2	Plunge Pool in ce	Plunge Pool in center 1/3 14.1%					
3	Pool Tailout	1.3%					
4	Pool		4.7%				
5	Narrow Run		3.6%				
6	Narrow Run		3.6%				
7	Med-width Run	3.6%					
8	Riffle	12.0%					
9	Wide Boulder run		28.5%				

2.2 Computations

Fish Passage

Station location, stream bed elevations, discharges, water surface stage, slope and stage of zero flow were entered into the PHABSIM hydraulic model and depths at each station were simulated for a range of flows between 5 and 50 cfs. Depths equal to or exceeding the passage depth criteria for each species were tallied at each modeled flow. Adjoining cells with depths equal or exceeding the criteria were also tallied. The total width of the cells in each of these categories at each modeled flow was divided by the total wetted width at each flow to compute the percent of the transect passable.

IFIM WUA

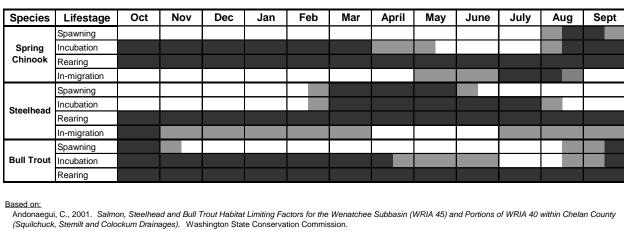
Figure 2.

Transect weighting for Lower Peshastin Creek was entered into the approved, calibrated Peshastin Creek PHABSIM model and WDFW approved HSI curves were used to output WUA for the selected species and life stages of salmonids.

2.3 Periodicity of Migrating Chinook and Bull Trout in Peshastin Creek

Peshastin Creek Fish Periodicity

Salmonid periodicity information for Peshastin creek is based on information from Andonaegui, (2001), and local resource agencies. Migration and spawning timing for salmonids using Peshastin creek are shown in Figure 2. No in-migration timing is given for bull trout and this study assumes that in-migration occurs from July through September.



Comments from: USFS (Cam Thomas, Cindy Raekes), WDFW (Andrew Murdoch, Bob Vadas, Mark Cookson), USFWS (Kate Terrell) and NOAA-Fisheries (Dale Bambrick) Key:

Black indicates periods of heaviest use

Grey indicates periods of moderate use

Blank areas indicate periods of little or no use

3.0 **RESULTS**

3.1 Bull Trout

Figures 3 through 7 show the relationship at each transect between stream discharge and the percent of total width and contiguous width (adjoining cells) that is passable to bull trout at each flow. Table 3 shows that 25 % of total width, the passage depth criteria of 0.6 feet for bull trout, are met on individual transects at 5 to 18 cfs. Discharges for contiguous passage range from 5 to 30 cfs. The discharge at which both passage criteria are met ranges from 5 to 30 cfs with an average of 13.4 cfs.

TABLE 3DISCHARGES FOR MEETING BULL TROUT PASSAGE CRITERIA(CFS)						
Passage Criteria	T-1	T-2	гз) Т-3	T-4	T-5	Mean
Total (25%)	6	5	10	5	18	8.8
Contiguous (10%)	6	5	6	4	30	10.2
Both Criteria	6	5	10	5	30	11.2

3.2 Chinook Salmon

Figures 8 through 12 show the relationship between stream discharge and the percent of total width and contiguous width (adjoining cells) at each transect that is passable to Chinook. Table 4 shows that 25 % of total width passage depth criteria of 0.8 feet for Chinook are met on individual transects from 15 to 34 cfs. Discharges for contiguous passage range from 13 to 41 cfs. The discharge at which both passage criteria are met ranges from 15 to 41 cfs with an average of 23.4 cfs.

TABLE 4 DISCHARGES FOR MEETING CHINOOK SALMON PASSAGE CRITERIA (CFS)						
Passage Criteria	T-1	T-2	T-3	T-4	T-5	Mean
Total (25%)	15	17	28	16	34	22
Contiguous (10%)	13	13	25	10	41	20.4
Both Criteria	15	17	28	16	41	23.4

3.3 WUA RESULTS

Figure 3 shows that WUA (a relative measure of habitat abundance) increases relatively consistently for all species expect Bull trout. Table 5 shows that WUA for juvenile steelhead rearing is nearly 4 times more abundant at 22 cfs than at 5 cfs and Chinook spawning habitat increases over 20 times at 20 cfs compared to 5 cfs.

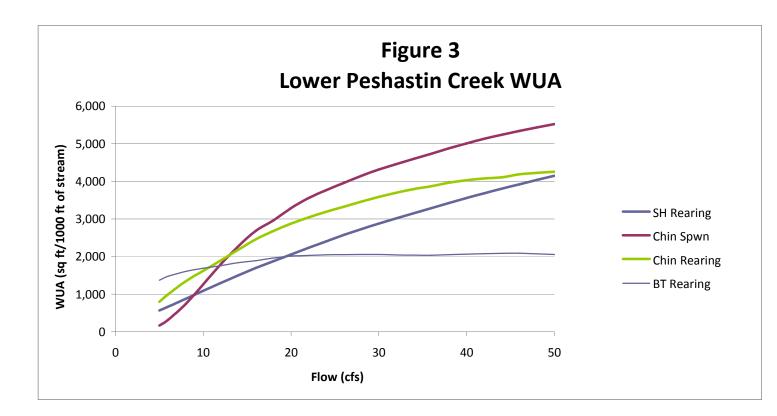


TABLE 5 WUA FOR SELECTED SPECIES									
Flow	Steelhead	% of	Ch	inook Salmo	n	Bull Trout			
cfs	Juv Rearing	Max	Spawning	Spawning % of Max Rea		Juv/Ad Rearing			
5	567	13.7%	166	3.0%	797	1,370			
6	668	16.1%	319	5.8%	994	1,477			
8	876	21.1%	740	13.4%	1,346	1,604			
10	1,090	26.3%	1,261	22.8%	1,623	1,688			
12	1,299	31.3%	1,798	32.5%	1,896	1,759			
14	1,500	36.2%	2,276	41.2%	2,182	1,838			
16	1,697	40.9%	2,684	48.6%	2,462	1,891			
18	1,876	45.2%	2,962	53.6%	2,679	1,963			
20	2,056	49.6%	3,288	59.5%	2,877	2,013			
22	2,234	53.8%	3,556	64.4%	3,044	2,028			
24	2,406	58.0%	3,765	68.2%	3,191	2,048			
26	2,574	62.0%	3,957	71.6%	3,325	2,051			
28	2,729	65.8%	4,145	75.0%	3,459	2,054			
30	2,876	69.3%	4,316	78.1%	3,588	2,054			
32	3,018	72.7%	4,460	80.8%	3,699	2,042			
34	3,155	76.0%	4,600	83.3%	3,797	2,038			
36	3,289	79.3%	4,737	85.8%	3,871	2,032			
38	3,425	82.5%	4,879	88.3%	3,965	2,053			
40	3,555	85.7%	5,009	90.7%	4,031	2,063			
42	3,682	88.7%	5,130	92.9%	4,077	2,076			
44	3,803	91.7%	5,240	94.9%	4,106	2,083			
46	3,920	94.5%	5,341	96.7%	4,186	2,088			
48	4,036	97.3%	5,435	98.4%	4,224	2,074			
50	4,149	100.0%	5,524	100.0%	4,256	2,057			

4.0 **DISSCUSSION**

Fish Passage

The results of this study show that the wide gravel bars in lower Peshastin Creek likely pose a significant barrier at low flows. Chinook salmon require higher flows for passage than bull trout. This is due to the greater depth criteria for Chinook passage. Thompson (1972) recommends the average passage flow. For Peshastin Creek the average for the 5 transects was 23.4 cfs. Although a flow of 23.4 cfs would likely pass fish in the majority of lower Peshastin Creek it is also likely that there are some critical areas (Transects 2 and 5) where more than 23.4 cfs or channel modifications would be required for upstream passage of Chinook during the late part of their migration period.

Flow records for water years 2003 through 2011 for Peshastin Creek at Green Bridge indicate that consistent snow melt almost always keeps flows higher than required for minimum depth passage, for the Chinook migration period between May and mid July. Some time between mid July and mid August the flows generally recede to below the flow indicated for Chinook migration. In most years flows stay low through September, except when rainstorms temporarily increased the flow. The timing of the flows at which passage is likely impeded is important because it happens during the time of heaviest use for Chinook in-migration.

Similarly, bull trout migration may be impacted during late July, August and September when prolonged periods of low flow may occur during times of expected in-migration. Since bull trout require less flow for passage than Chinook, a smaller increase in flow may prove beneficial to bull trout migration.

IFIM WUA

In addition to the passage issues discussed above, habitat for rearing juvenile steelhead and spawning Chinook salmon are the two life stages that are most likely impacted by low summer flows in Peshastin Creek. Table 5 shows that higher stream flow can drastically increase habitat for both life stages depending on the base flow selected and how much the stream flow is increased.

The results presented in this report are meant to be tools that can be used by resource agencies to quantify the benefits to fish of additional flow in Peshastin Creek. When used with existing stream flow records this data can be used to address when flow additional flow would be beneficial and the benefits of specific amounts of added flow.

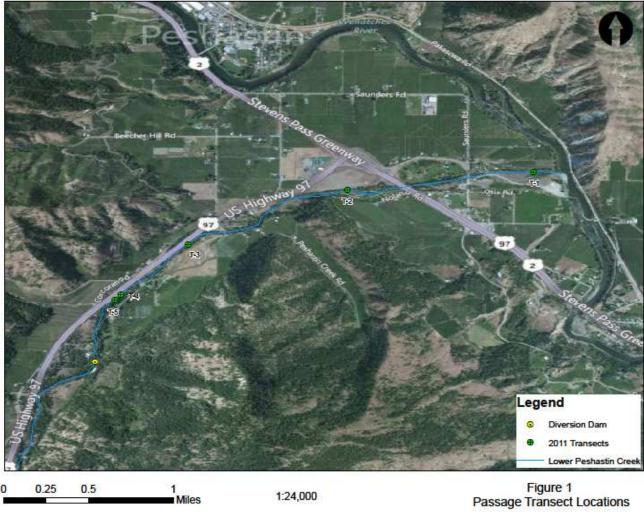
5.0 References

Anchor Environmental, L.L.C. and EES Consulting, Inc., 2007. *Peshastin Subbasin Needs and Alternatives Study*. Prepared for Chelan County Natural Resources Department under Washington State Department of Ecology Grant G0700003. January.

Andonaegui, C., 2001. Salmon, Steelhead and Bull Trout Habitat Limiting Factors for the Wenatchee Subbasin (WRIA 45) and Portions of WRIA 40 within Chelan County (Squilchuck, Stemilt and Colockum Drainages). Washington State Conservation Commission.

EES Consulting, 2005. *Final Technical Report Lower Wenatchee River PHABSIM Studies* Prepared for Chelan County Natural Resources Department

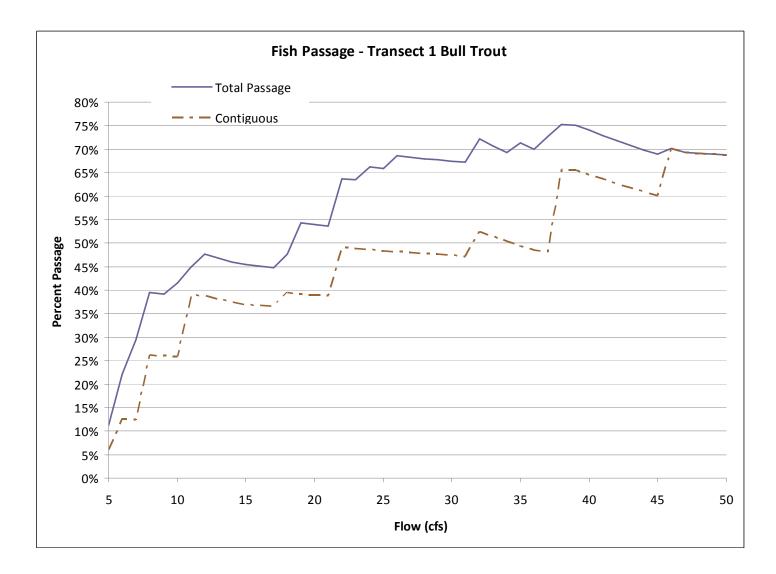
Thompson, K., 1972. Determining Stream Flows For Fish. Presented at Instream Flow Requirement Workshop, Pacific Northwest River Basins Commission, March 1972.

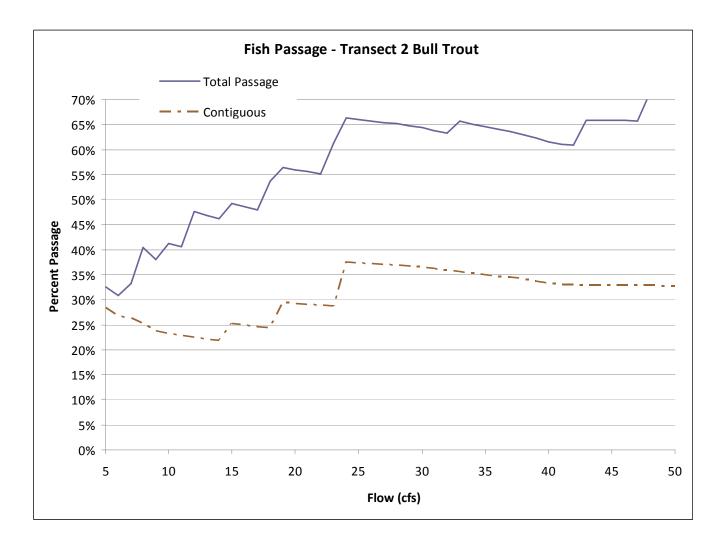


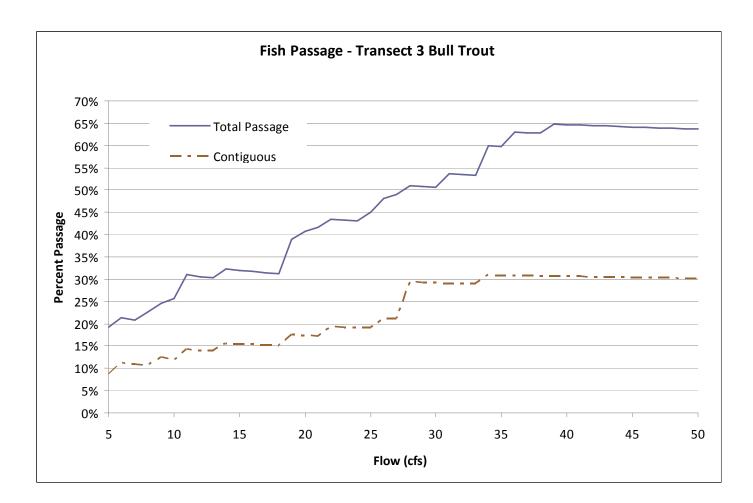
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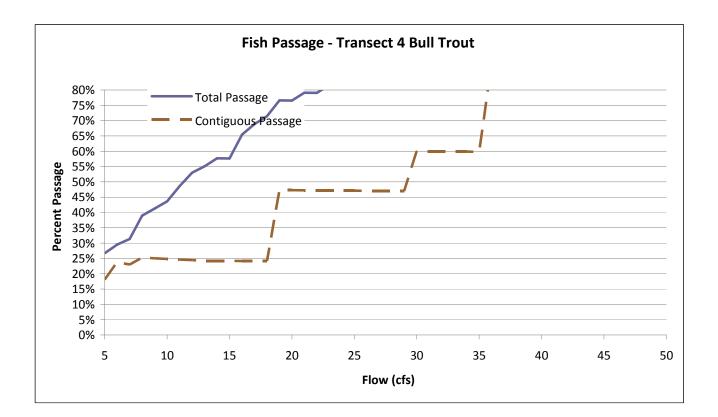
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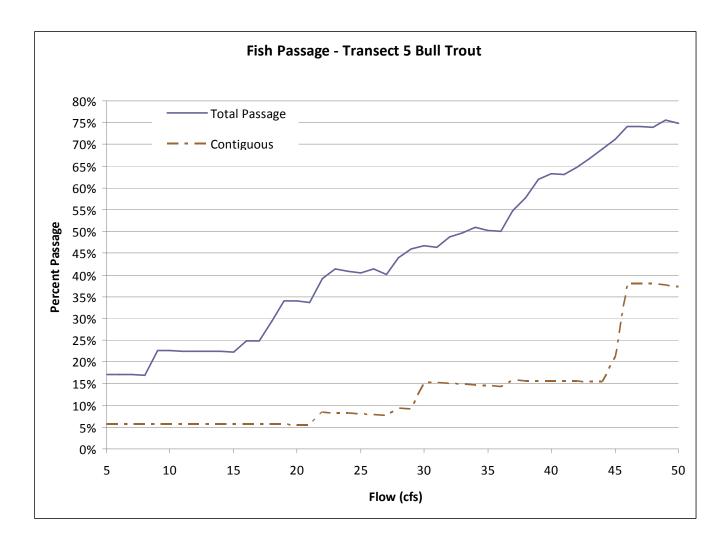
Figure 1 Passage Transect Locations

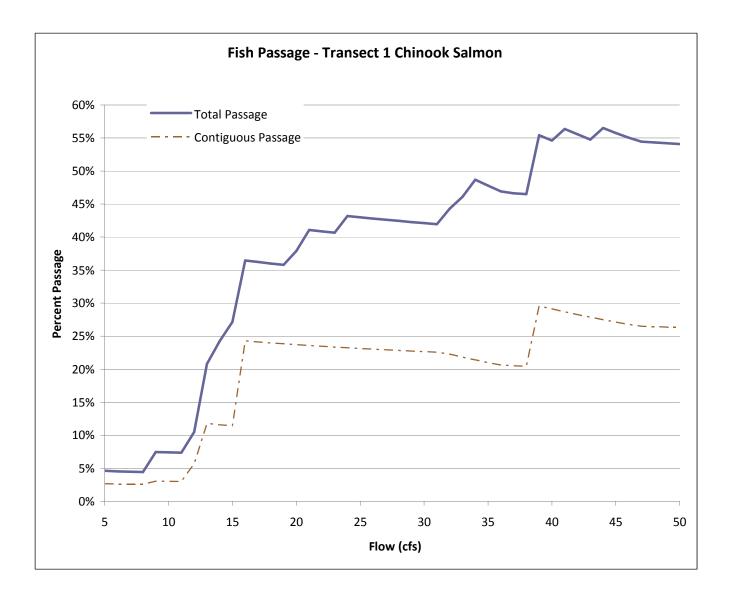


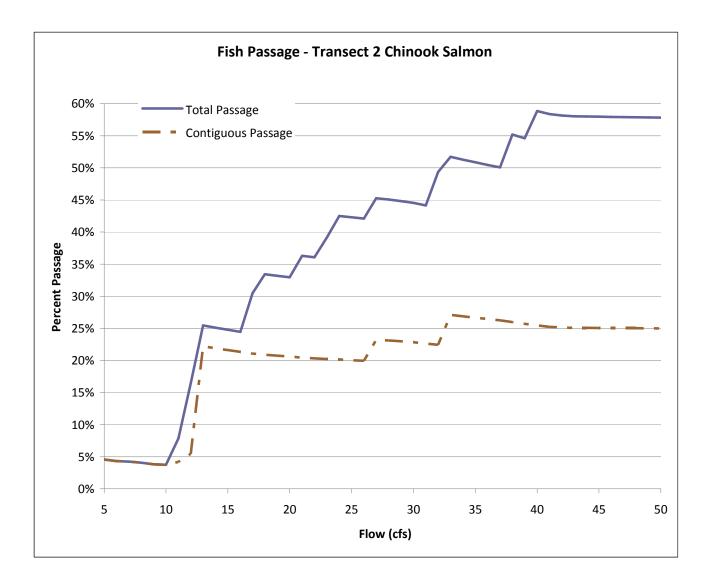


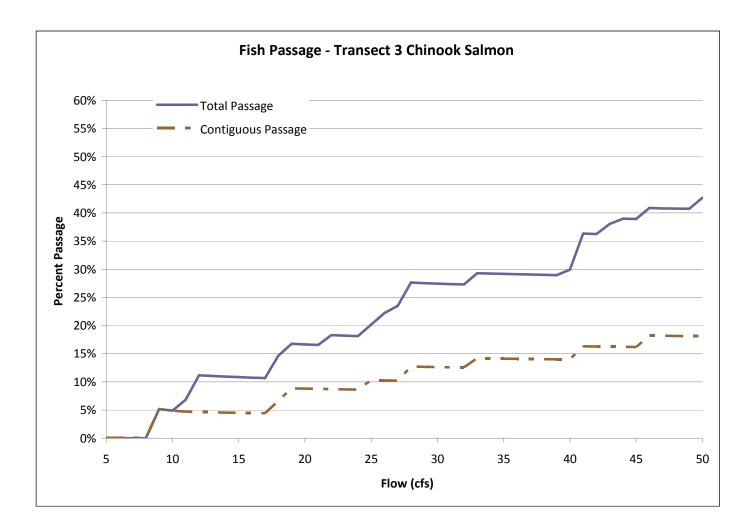


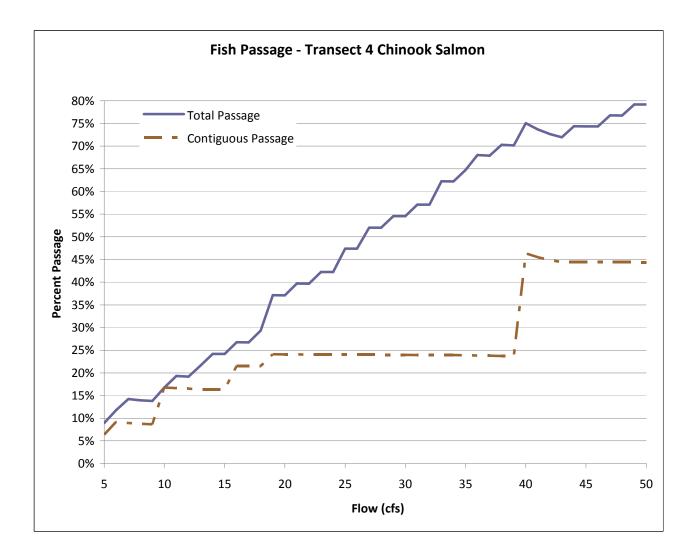


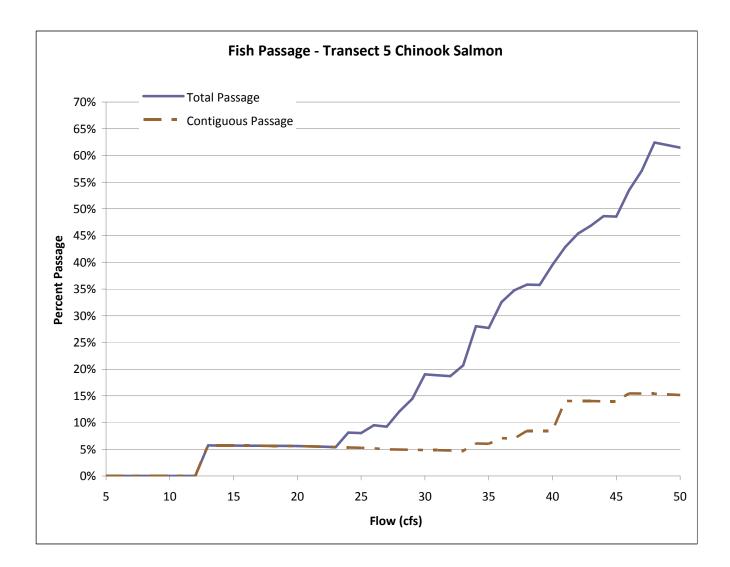












APPENDIX C HYDRAULIC CALCULATIONS

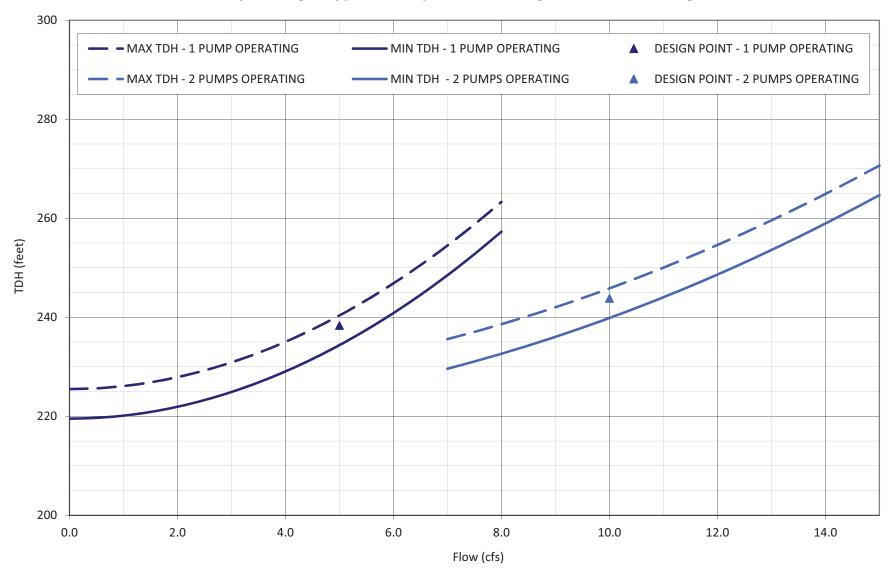
SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 1 - 10 CFS DESIGN FLOW

BY:	David Rice, P.E.
DATE:	13-Sep-12

UCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTION	N PIPING		PS PI	PING		DISCHARGE PIPING		
ELEV	922.0 feet	Invert at Diversion		PROPERTIES			HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS	
HWL	927.0 feet	Wet Well (High River)		NOM. DIAM. (in)					12	12		20	
LWL	. 923.0 feet Wet Well (Low River)		O.D. (in)					12	12		20		
				I.D. (in)					12	12		20	
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE	
ELEV	1146.0 feet	Canal Bottom		С					110	110		130	
HIGH	1148.5 feet	Canal (High Flow)		LENGTH (feet)					8	12		1,240	
LWL	1146.5 feet	Canal (Low Flow)		K				1	10	10		10	

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2244	5.0	238	193.1
2	4488	10.0	244	395.2
*Assumes	70%	Efficiency		

TOTAI	FLOW		VELO	CITIES		SUCTION	LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0			219.5	225.5	223.5	219.5	225.5	223.5			
224	0.5	0.64	0.64		0.23			0.0	0.0	0.1	0.0	0.0	0.0			219.7	225.7	223.7	219.6	225.6	223.6			
449	1.0	1.27	1.27		0.46			0.0	0.1	0.5	0.0	0.1	0.0			220.1	226.1	224.1	219.7	225.7	223.7			
673	1.5	1.91	1.91		0.69			0.1	0.1	1.1	0.0	0.3	0.0			220.9	226.9	224.9	220.0	226.0	224.0			
898	2.0	2.55	2.55		0.92			0.1	0.2	2.0	0.1	0.5	0.0			221.9	227.9	225.9	220.4	226.4	224.4			
1122	2.5	3.18	3.18		1.15			0.2	0.3	3.1	0.1	0.8	0.0			223.3	229.3	227.3	220.8	226.8	224.8			
1346	3.0	3.82	3.82		1.37			0.3	0.5	4.5	0.1	1.1	0.0			224.9	230.9	228.9	221.4	227.4	225.4			
1571	3.5	4.46	4.46		1.60			0.4	0.6	6.2	0.2	1.5	0.0			226.8	232.8	230.8	222.1	228.1	226.1			
1795	4.0	5.09	5.09		1.83			0.5	0.8	8.1	0.2	2.0	0.1			229.0	235.0	233.0	222.9	228.9	226.9			
2020	4.5	5.73	5.73		2.06			0.7	1.0	10.2	0.3	2.5	0.1			231.6	237.6	235.6	223.7	229.7	227.7			
2244	5.0	6.37	6.37		2.29			0.8	1.2	12.6	0.3	3.1	0.1			234.4	240.4	238.4	224.7	230.7	228.7			
2468	5.5	7.00	7.00		2.52			1.0	1.4	15.2	0.4	3.8	0.1			237.5	243.5	241.5	225.8	231.8	229.8			
2693	6.0	7.64	7.64		2.75			1.2	1.6	18.1	0.4	4.5	0.1			240.9	246.9	244.9	226.9	232.9	230.9			
2917	6.5	8.27	8.27		2.98			1.4	1.9	21.3	0.5	5.3	0.1			244.5	250.5	248.5	228.2	234.2	232.2			
3142	7.0	8.91	8.91		3.21			1.6	2.2	24.7	0.6	6.2	0.2			248.5	254.5	252.5	229.6	235.6	233.6			
3366	7.5	9.55	9.55		3.44			1.8	2.5	28.3	0.6	7.1	0.2			252.8	258.8	256.8	231.1	237.1	235.1			
3590	8.0	10.18	10.18		3.67			2.1	2.8	32.2	0.7	8.1	0.2			257.3	263.3	261.3	232.6	238.6	236.6			
3815	8.5	10.82	10.82		3.90			2.4	3.1	36.4	0.8	9.1	0.2			262.1	268.1	266.1	234.3	240.3	238.3			
4039	9.0	11.46	11.46		4.12			2.6	3.4	40.8	0.9	10.2	0.3			267.3	273.3	271.3	236.0	242.0	240.0			
4264	9.5	12.09	12.09		4.35			2.9	3.8	45.4	1.0	11.4	0.3			272.7	278.7	276.7	237.9	243.9	241.9			
4488	10.0	12.73	12.73		4.58			3.3	4.2	50.3	1.1	12.6	0.3			278.4	284.4	282.4	239.8	245.8	243.8			
4712	10.5	13.37	13.37		4.81			3.6	4.6	55.5	1.2	13.9	0.3			284.4	290.4	288.4	241.9	247.9	245.9			
4937	11.0	14.00	14.00		5.04			3.9	5.0	60.9	1.3	15.2	0.4			290.7	296.7	294.7	244.0	250.0	248.0			
5161	11.5	14.64	14.64		5.27			4.3	5.4	66.6	1.4	16.6	0.4			297.2	303.2	301.2	246.3	252.3	250.3			
5386	12.0	15.28	15.28		5.50			4.7	5.9	72.5	1.5	18.1	0.4			304.1	310.1	308.1	248.6	254.6	252.6			
5610	12.5	15.91	15.91		5.73			5.1	6.3	78.6	1.7	19.7	0.5			311.2	317.2	315.2	251.0	257.0	255.0			
5834	13.0	16.55	16.55		5.96			5.5	6.8	85.1	1.8	21.3	0.5			318.7	324.7	322.7	253.6	259.6	257.6			
6059	13.5	17.19	17.19		6.19			5.9	7.3	91.7	1.9	22.9	0.5			326.4	332.4	330.4	256.2	262.2	260.2			
6283	14.0	17.82	17.82		6.42			6.4	7.8	98.7	2.1	24.7	0.6			334.4	340.4	338.4	258.9	264.9	262.9			
6508	14.5	18.46	18.46		6.65			6.9	8.3	105.8	2.2	26.5	0.6			342.7	348.7	346.7	261.7	267.7	265.7			
6732	15.0	19.10	19.10		6.87			7.3	8.9	113.2	2.3	28.3	0.6			351.3	357.3	355.3	264.7	270.7	268.7			



PID Pump Exchange - Appraisal Study - Alternative Alignment 1 - 10 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 1 - 20 CFS DESIGN FLOW

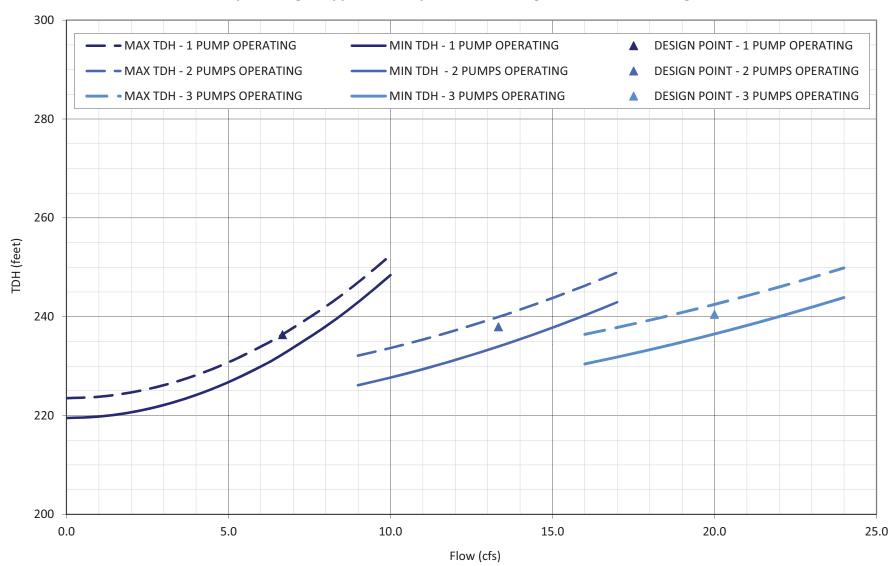
SUCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING	PS PIPING				DISCHARGE PIPING		
ELEV	922.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRAN	
HWL	927.0 feet	Wet Well (High River)	NOM. DIAM. (in)					14	14		30	
LWL	923.0 feet	Wet Well (Low River)	O.D. (in)					14	14		30	
			I.D. (in)					14	14		30	
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDP	
ELEV	1146.0 feet	Canal Bottom	С					110	110		130	
HIGH	1148.5 feet	Canal (High Flow)	LENGTH (feet)					8	12		1,24	
LWL	1146.5 feet	Canal (Low Flow)	К					10	10		10	

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2992	6.7	236	255.4
2	5984	13.3	238	514.2
3	8976	20.0	240	779.5
*Assumes	70%	Efficiency		

TOTAL	FLOW		VELO	CITIES		SUCTION	N LOSSES	DISCHARG	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DYN	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	219.5	225.5	223.5	219.5	225.5	223.5	219.5	225.5	223.5
449	1.0	0.94	0.94		0.20			0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	219.8	225.8	223.8	219.6	225.6	223.6	219.5	225.5	223.5
898	2.0	1.87	1.87		0.41			0.0	0.0	1.1	0.0	0.3	0.0	0.1	0.0	220.7	226.7	224.7	219.8	225.8	223.8	219.7	225.7	223.7
1346	3.0	2.81	2.81		0.61			0.1	0.1	2.4	0.1	0.6	0.0	0.3	0.0	222.1	228.1	226.1	220.2	226.2	224.2	219.9	225.9	223.9
1795	4.0	3.74	3.74		0.81			0.1	0.1	4.3	0.1	1.1	0.0	0.5	0.0	224.2	230.2	228.2	220.8	226.8	224.8	220.2	226.2	224.2
2244	5.0	4.68	4.68		1.02			0.2	0.2	6.8	0.1	1.7	0.0	0.8	0.0	226.8	232.8	230.8	221.6	227.6	225.6	220.6	226.6	224.6
2693	6.0	5.61	5.61		1.22			0.2	0.2	9.8	0.2	2.4	0.1	1.1	0.0	229.9	235.9	233.9	222.5	228.5	226.5	221.1	227.1	225.1
2992	6.7	6.24	6.24		1.36			0.3	0.3	12.1	0.2	3.0	0.1	1.3	0.0	232.4	238.4	236.4	223.1	229.1	227.1	221.4	227.4	225.4
3590	8.0	7.48	7.48		1.63			0.4	0.4	17.4	0.3	4.3	0.1	1.9	0.0	238.0	244.0	242.0	224.7	230.7	228.7	222.3	228.3	226.3
4039	9.0	8.42	8.42		1.83			0.5	0.5	22.0	0.4	5.5	0.1	2.4	0.1	242.9	248.9	246.9	226.1	232.1	230.1	223.0	229.0	227.0
4488	10.0	9.35	9.35		2.04			0.6	0.6	27.2	0.5	6.8	0.1	3.0	0.1	248.4	254.4	252.4	227.7	233.7	231.7	223.8	229.8	227.8
4937	11.0	10.29	10.29		2.24			0.8	0.7	32.9	0.6	8.2	0.2	3.7	0.1	254.5	260.5	258.5	229.4	235.4	233.4	224.7	230.7	228.7
5386	12.0	11.22	11.22		2.44			0.9	0.8	39.1	0.7	9.8	0.2	4.3	0.1	261.1	267.1	265.1	231.2	237.2	235.2	225.7	231.7	229.7
5984	13.3	12.47	12.47		2.72			1.1	1.0	48.3	0.9	12.1	0.2	5.4	0.1	270.8	276.8	274.8	234.0	240.0	238.0	227.1	233.1	231.1
6283	14.0	13.09	13.09		2.85			1.3	1.1	53.2	1.0	13.3	0.3	5.9	0.1	276.1	282.1	280.1	235.4	241.4	239.4	227.9	233.9	231.9
6732	15.0	14.03	14.03		3.06			1.4	1.2	61.1	1.1	15.3	0.3	6.8	0.1	284.4	290.4	288.4	237.8	243.8	241.8	229.1	235.1	233.1
7181	16.0	14.97	14.97		3.26			1.6	1.4	69.6	1.2	17.4	0.3	7.7	0.2	293.3	299.3	297.3	240.3	246.3	244.3	230.4	236.4	234.4
7630	17.0	15.90	15.90		3.46			1.9	1.6	78.5	1.4	19.6	0.4	8.7	0.2	302.8	308.8	306.8	242.9	248.9	246.9	231.8	237.8	235.8
8078	18.0	16.84	16.84		3.67			2.1	1.7	88.0	1.5	22.0	0.4	9.8	0.2	312.9	318.9	316.9	245.7	251.7	249.7	233.3	239.3	237.3
8527	19.0	17.77	17.77		3.87			2.3	1.9	98.1	1.7	24.5	0.5	10.9	0.2	323.5	329.5	327.5	248.7	254.7	252.7	234.9	240.9	238.9
8976	20.0	18.71	18.71		4.07			2.6	2.1	108.7	1.9	27.2	0.5	12.1	0.2	334.7	340.7	338.7	251.9	257.9	255.9	236.5	242.5	240.5
9425	21.0	19.64	19.64		4.28			2.8	2.3	119.8	2.1	30.0	0.6	13.3	0.3	346.5	352.5	350.5	255.2	261.2	259.2	238.2	244.2	242.2
9874	22.0	20.58	20.58		4.48			3.1	2.5	131.5	2.2	32.9	0.6	14.6	0.3	358.9	364.9	362.9	258.6	264.6	262.6	240.0	246.0	244.0
10322	23.0	21.51	21.51		4.68			3.4	2.7	143.7	2.4	35.9	0.7	16.0	0.3	371.8	377.8	375.8	262.2	268.2	266.2	241.9	247.9	245.9
10771	24.0	22.45	22.45		4.89			3.7	2.9	156.5	2.6	39.1	0.7	17.4	0.3	385.3	391.3	389.3	266.0	272.0	270.0	243.9	249.9	247.9
11220	25.0	23.38	23.38		5.09			4.0	3.2	169.8	2.8	42.5	0.8	18.9	0.4	399.3	405.3	403.3	269.9	275.9	273.9	245.9	251.9	249.9
11669	26.0	24.32	24.32		5.30			4.4	3.4	183.7	3.1	45.9	0.8	20.4	0.4	414.0	420.0	418.0	274.0	280.0	278.0	248.1	254.1	252.1
12118	27.0	25.25	25.25		5.50			4.7	3.6	198.1	3.3	49.5	0.9	22.0	0.4	429.2	435.2	433.2	278.3	284.3	282.3	250.3	256.3	254.3
12566	28.0	26.19	26.19		5.70			5.1	3.9	213.0	3.5	53.2	1.0	23.7	0.5	445.0	451.0	449.0	282.7	288.7	286.7	252.6	258.6	256.6
13015	29.0	27.12	27.12		5.91			5.4	4.2	228.5	3.7	57.1	1.0	25.4	0.5	461.3	467.3	465.3	287.2	293.2	291.2	255.0	261.0	259.0
13464	30.0	28.06	28.06		6.11			5.8	4.4	244.5	4.0	61.1	1.1	27.2	0.5	478.2	484.2	482.2	292.0	298.0	296.0	257.4	263.4	261.4

BY: David Rice, P.E.

DATE: 13-Sep-12



PID Pump Exchange - Appraisal Study - Alternative Alignment 1 - 20 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 1 - 40 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTIO	N PIPING	PS PIPING				DISCHARGE PIPING		
ELEV	922.0 feet	Invert at Diversion	7	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.	
HWL	927.0 feet	Wet Well (High River)		NOM. DIAM. (in)					18	18		36	
LWL	923.0 feet	Wet Well (Low River)		O.D. (in)					18	18		36	
				I.D. (in)					18	18		36	
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE	
ELEV	1146.0 feet	Canal Bottom		С					110	110		130	
HIGH	1148.5 feet	Canal (High Flow)	7	LENGTH (feet)					8	12		1,240	
LWL	1146.5 feet	Canal (Low Flow)	1	К					10	10		10	

ROPOSED E	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	5985	13.3	242	523.9
2	11970	26.7	245	1059.5
3	17952	40.0	250	1617.4
'Assumes	70%	Efficiency		

BY:

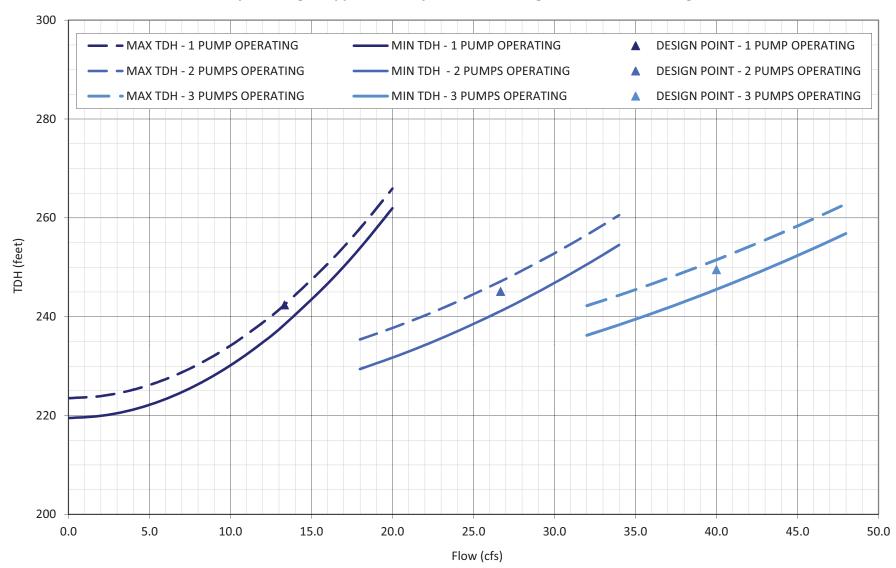
DATE:

David Rice, P.E.

13-Sep-12

TOTAL	FLOW		VELO	CITIES		SUCTION	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	219.5	225.5	223.5	219.5	225.5	223.5	219.5	225.5	223.5
898	2.0	1.13	1.13		0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	219.9	225.9	223.9	219.6	225.6	223.6	219.6	225.6	223.6
1795	4.0	2.26	2.26		0.57			0.0	0.0	1.6	0.0	0.4	0.0	0.2	0.0	221.2	227.2	225.2	220.0	226.0	224.0	219.8	225.8	223.8
2693	6.0	3.39	3.39		0.85			0.1	0.1	3.6	0.1	0.9	0.0	0.4	0.0	223.3	229.3	227.3	220.6	226.6	224.6	220.1	226.1	224.1
3590	8.0	4.53	4.53		1.13			0.2	0.2	6.4	0.1	1.6	0.0	0.7	0.0	226.3	232.3	230.3	221.5	227.5	225.5	220.6	226.6	224.6
4488	10.0	5.66	5.66		1.41			0.3	0.2	9.9	0.2	2.5	0.0	1.1	0.0	230.1	236.1	234.1	222.6	228.6	226.6	221.2	227.2	225.2
5386	12.0	6.79	6.79		1.70			0.4	0.3	14.3	0.2	3.6	0.1	1.6	0.0	234.8	240.8	238.8	223.9	229.9	227.9	221.9	227.9	225.9
5985	13.3	7.55	7.55		1.89			0.6	0.4	17.7	0.3	4.4	0.1	2.0	0.0	238.4	244.4	242.4	225.0	231.0	229.0	222.5	228.5	226.5
7181	16.0	9.05	9.05		2.26			0.8	0.6	25.5	0.4	6.4	0.1	2.8	0.0	246.7	252.7	250.7	227.3	233.3	231.3	223.7	229.7	227.7
8078	18.0	10.18	10.18		2.55			1.0	0.7	32.2	0.5	8.1	0.1	3.6	0.1	253.9	259.9	257.9	229.4	235.4	233.4	224.9	230.9	228.9
8976	20.0	11.32	11.32		2.83			1.2	0.9	39.8	0.6	9.9	0.2	4.4	0.1	261.9	267.9	265.9	231.7	237.7	235.7	226.1	232.1	230.1
9874	22.0	12.45	12.45		3.11			1.5	1.0	48.1	0.7	12.0	0.2	5.3	0.1	270.8	276.8	274.8	234.2	240.2	238.2	227.5	233.5	231.5
10771	24.0	13.58	13.58		3.39			1.8	1.2	57.3	0.8	14.3	0.2	6.4	0.1	280.5	286.5	284.5	237.0	243.0	241.0	229.0	235.0	233.0
11970	26.7	15.09	15.09		3.77			2.2	1.5	70.7	0.9	17.7	0.3	7.9	0.1	294.8	300.8	298.8	241.1	247.1	245.1	231.2	237.2	235.2
12566	28.0	15.84	15.84		3.96			2.4	1.6	77.9	1.0	19.5	0.3	8.7	0.1	302.5	308.5	306.5	243.3	249.3	247.3	232.3	238.3	236.3
13464	30.0	16.97	16.97		4.24			2.8	1.8	89.5	1.2	22.4	0.3	9.9	0.2	314.8	320.8	318.8	246.8	252.8	250.8	234.2	240.2	238.2
14362	32.0	18.11	18.11		4.53			3.2	2.1	101.8	1.3	25.5	0.4	11.3	0.2	327.9	333.9	331.9	250.6	256.6	254.6	236.2	242.2	240.2
15259	34.0	19.24	19.24		4.81			3.6	2.3	114.9	1.5	28.7	0.4	12.8	0.2	341.8	347.8	345.8	254.5	260.5	258.5	238.4	244.4	242.4
16157	36.0	20.37	20.37		5.09			4.0	2.6	128.9	1.6	32.2	0.5	14.3	0.2	356.6	362.6	360.6	258.8	264.8	262.8	240.6	246.6	244.6
17054	38.0	21.50	21.50		5.38			4.5	2.8	143.6	1.8	35.9	0.5	16.0	0.2	372.2	378.2	376.2	263.2	269.2	267.2	243.0	249.0	247.0
17952	40.0	22.63	22.63		5.66			5.0	3.1	159.1	2.0	39.8	0.6	17.7	0.3	388.6	394.6	392.6	267.9	273.9	271.9	245.5	251.5	249.5
18850	42.0	23.76	23.76		5.94			5.5	3.4	175.4	2.2	43.8	0.6	19.5	0.3	405.9	411.9	409.9	272.8	278.8	276.8	248.2	254.2	252.2
19747	44.0	24.90	24.90		6.22			6.0	3.7	192.5	2.4	48.1	0.7	21.4	0.3	424.1	430.1	428.1	278.0	284.0	282.0	250.9	256.9	254.9
20645	46.0	26.03	26.03		6.51			6.6	4.0	210.4	2.6	52.6	0.7	23.4	0.3	443.1	449.1	447.1	283.4	289.4	287.4	253.8	259.8	257.8
21542	48.0	27.16	27.16		6.79			7.2	4.4	229.1	2.8	57.3	0.8	25.5	0.4	462.9	468.9	466.9	289.1	295.1	293.1	256.8	262.8	260.8
22440	50.0	28.29	28.29		7.07			7.8	4.7	248.6	3.0	62.1	0.8	27.6	0.4	483.5	489.5	487.5	294.9	300.9	298.9	260.0	266.0	264.0
23338	52.0	29.42	29.42		7.36			8.4	5.0	268.8	3.2	67.2	0.9	29.9	0.4	505.0	511.0	509.0	301.1	307.1	305.1	263.2	269.2	267.2
24235	54.0	30.55	30.55		7.64			9.1	5.4	289.9	3.5	72.5	1.0	32.2	0.5	527.4	533.4	531.4	307.4	313.4	311.4	266.6	272.6	270.6
25133	56.0	31.69	31.69		7.92			9.7	5.8	311.8	3.7	77.9	1.0	34.6	0.5	550.5	556.5	554.5	314.0	320.0	318.0	270.2	276.2	274.2
26030	58.0	32.82	32.82		8.20			10.5	6.2	334.5	4.0	83.6	1.1	37.2	0.5	574.6	580.6	578.6	320.8	326.8	324.8	273.8	279.8	277.8
26928	60.0	33.95	33.95		8.49			11.2	6.6	357.9	4.2	89.5	1.2	39.8	0.6	599.4	605.4	603.4	327.9	333.9	331.9	277.6	283.6	281.6

PID Pump Exchange - Hydraulic Analysis - Alignment 1 - TO PID.xlsx



PID Pump Exchange - Appraisal Study - Alternative Alignment 1 - 40 CFS Design Flow

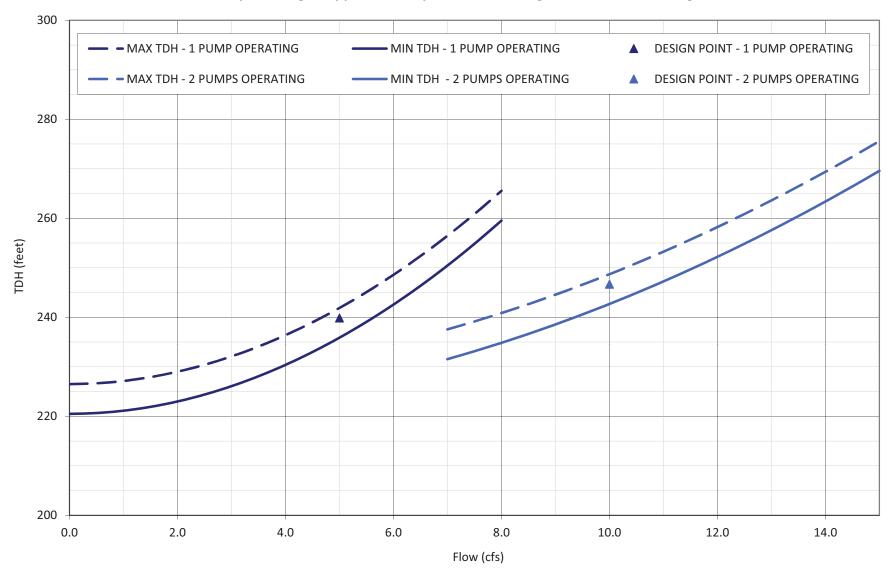
SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 2 - 10 CFS DESIGN FLOW

BY:	David Rice, P.E.
DATE:	13-Sep-12

ICTION W	VATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING		PS P	PING		DISCHAR	ge pipin
LEV	921.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRAN
HWL	926.0 feet	Wet Well (High River)	NOM. DIAM. (in)					12	12		20
LWL	922.0 feet	Wet Well (Low River)	O.D. (in)					12	12		20
			I.D. (in)					12	12		20
DISCHARGE	E WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1146.0 feet	Canal Bottom	С					110	110		130
HIGH	1148.5 feet	Canal (High Flow)	LENGTH (feet)					8	12		1,790
LWL	1146.5 feet	Canal (Low Flow)	K					10	10		10

PUMPS ON	FLOW (GPM)	FLOW (CFS)	TDH (FT)	POWER (HP)*
1	2244	5.0	240	194.4
2	4488	10.0	247	399.8
*Assumes	70%	Efficiency		

TOTAL	FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSE	S - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL D	NAMIC HEA	D - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0			220.5	226.5	224.5	220.5	226.5	224.5			
224	0.5	0.64	0.64		0.23			0.0	0.0	0.1	0.0	0.0	0.0			220.7	226.7	224.7	220.6	226.6	224.6			
449	1.0	1.27	1.27		0.46			0.0	0.1	0.5	0.0	0.1	0.0			221.1	227.1	225.1	220.7	226.7	224.7			
673	1.5	1.91	1.91		0.69			0.1	0.2	1.1	0.0	0.3	0.0			221.9	227.9	225.9	221.0	227.0	225.0			
898	2.0	2.55	2.55		0.92			0.1	0.3	2.0	0.1	0.5	0.0			223.0	229.0	227.0	221.5	227.5	225.5			
1122	2.5	3.18	3.18		1.15			0.2	0.5	3.1	0.1	0.8	0.0			224.4	230.4	228.4	222.0	228.0	226.0			
1346	3.0	3.82	3.82		1.37			0.3	0.7	4.5	0.1	1.1	0.0			226.1	232.1	230.1	222.6	228.6	226.6			
1571	3.5	4.46	4.46		1.60			0.4	0.9	6.2	0.2	1.5	0.0			228.1	234.1	232.1	223.4	229.4	227.4			
1795	4.0	5.09	5.09		1.83			0.5	1.1	8.1	0.2	2.0	0.1			230.4	236.4	234.4	224.2	230.2	228.2			
2020	4.5	5.73	5.73		2.06			0.7	1.4	10.2	0.3	2.5	0.1			233.0	239.0	237.0	225.2	231.2	229.2			
2244	5.0	6.37	6.37		2.29			0.8	1.7	12.6	0.3	3.1	0.1			235.9	241.9	239.9	226.2	232.2	230.2			
2468	5.5	7.00	7.00		2.52			1.0	2.0	15.2	0.4	3.8	0.1			239.1	245.1	243.1	227.4	233.4	231.4			
2693	6.0	7.64	7.64		2.75			1.2	2.3	18.1	0.4	4.5	0.1			242.6	248.6	246.6	228.7	234.7	232.7			
2917	6.5	8.27	8.27		2.98			1.4	2.7	21.3	0.5	5.3	0.1			246.4	252.4	250.4	230.1	236.1	234.1			
3142	7.0	8.91	8.91		3.21			1.6	3.1	24.7	0.6	6.2	0.2			250.5	256.5	254.5	231.5	237.5	235.5			
3366	7.5	9.55	9.55		3.44			1.8	3.5	28.3	0.6	7.1	0.2			254.8	260.8	258.8	233.1	239.1	237.1			
3590	8.0	10.18	10.18		3.67			2.1	4.0	32.2	0.7	8.1	0.2			259.5	265.5	263.5	234.8	240.8	238.8			
3815	8.5	10.82	10.82		3.90		1	2.4	4.5	36.4	0.8	9.1	0.2			264.5	270.5	268.5	236.6	242.6	240.6			
4039	9.0	11.46	11.46		4.12			2.6	5.0	40.8	0.9	10.2	0.3			269.8	275.8	273.8	238.6	244.6	242.6			
4264	9.5	12.09	12.09		4.35			2.9	5.5	45.4	1.0	11.4	0.3			275.4	281.4	279.4	240.6	246.6	244.6			
4488	10.0	12.73	12.73		4.58		1	3.3	6.0	50.3	1.1	12.6	0.3			281.2	287.2	285.2	242.7	248.7	246.7			
4712	10.5	13.37	13.37		4.81			3.6	6.6	55.5	1.2	13.9	0.3			287.4	293.4	291.4	244.9	250.9	248.9			
4937	11.0	14.00	14.00		5.04		1	3.9	7.2	60.9	1.3	15.2	0.4			293.9	299.9	297.9	247.2	253.2	251.2			
5161	11.5	14.64	14.64		5.27			4.3	7.8	66.6	1.4	16.6	0.4			300.6	306.6	304.6	249.7	255.7	253.7			
5386	12.0	15.28	15.28		5.50			4.7	8.5	72.5	1.5	18.1	0.4			307.7	313.7	311.7	252.2	258.2	256.2			
5610	12.5	15.91	15.91		5.73			5.1	9.1	78.6	1.7	19.7	0.5			315.0	321.0	319.0	254.8	260.8	258.8			
5834	13.0	16.55	16.55		5.96			5.5	9.8	85.1	1.8	21.3	0.5			322.7	328.7	326.7	257.6	263.6	261.6			
6059	13.5	17.19	17.19		6.19			5.9	10.5	91.7	1.9	22.9	0.5			330.6	336.6	334.6	260.4	266.4	264.4			
6283	14.0	17.82	17.82		6.42			6.4	11.3	98.7	2.1	24.7	0.6			338.9	344.9	342.9	263.4	269.4	267.4			
6508	14.5	18.46	18.46		6.65			6.9	12.0	105.8	2.2	26.5	0.6			347.4	353.4	351.4	266.4	272.4	270.4			
6732	15.0	19.10	19.10		6.87			7.3	12.8	113.2	2.3	28.3	0.6			356.2	362.2	360.2	269.6	275.6	273.6			



PID Pump Exchange - Appraisal Study - Alternative Alignment 2 - 10 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 2 - 20 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTIO	N PIPING		PS P	IPING		DISCHAR	GE PIPING
ELEV	921.0 feet	Invert at Diversion		PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	926.0 feet	Wet Well (High River)		NOM. DIAM. (in)					14	14		30
LWL	922.0 feet	922.0 feet Wet Well (Low River)		O.D. (in)					14	14		30
			_	I.D. (in)					14	14		30
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE
ELEV	1146.0 feet	Canal Bottom		С					110	110		130
HIGH	1148.5 feet	Canal (High Flow)		LENGTH (feet)					8	12		1,790
LWL	1146.5 feet	Canal (Low Flow)		К					10	10		10

PROPOSED [DESIGN POIN	rs:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2992	6.7	238	256.6
2	5984	13.3	239	517.3
3	8976	20.0	242	785.7
*Assumes	70%	Efficiency		

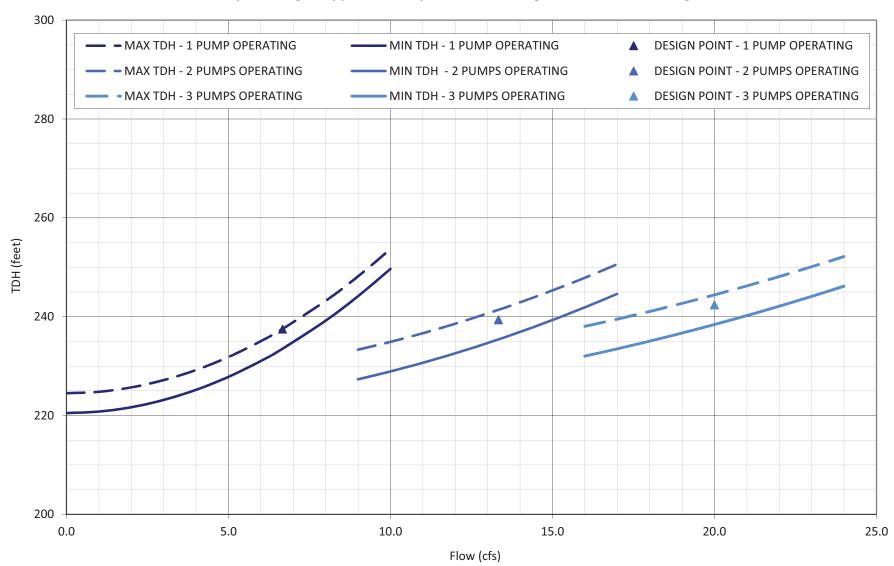
BY:

DATE:

David Rice, P.E.

13-Sep-12

TOTAL	FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSE	S - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	D - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	D - 3 PUMPS
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	220.5	226.5	224.5	220.5	226.5	224.5	220.5	226.5	224.5
449	1.0	0.94	0.94		0.20			0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	220.8	226.8	224.8	220.6	226.6	224.6	220.5	226.5	224.5
898	2.0	1.87	1.87		0.41			0.0	0.0	1.1	0.0	0.3	0.0	0.1	0.0	221.7	227.7	225.7	220.8	226.8	224.8	220.7	226.7	224.7
1346	3.0	2.81	2.81		0.61			0.1	0.1	2.4	0.1	0.6	0.0	0.3	0.0	223.1	229.1	227.1	221.3	227.3	225.3	220.9	226.9	224.9
1795	4.0	3.74	3.74		0.81			0.1	0.2	4.3	0.1	1.1	0.0	0.5	0.0	225.2	231.2	229.2	221.9	227.9	225.9	221.3	227.3	225.3
2244	5.0	4.68	4.68		1.02			0.2	0.2	6.8	0.1	1.7	0.0	0.8	0.0	227.8	233.8	231.8	222.6	228.6	226.6	221.7	227.7	225.7
2693	6.0	5.61	5.61		1.22			0.2	0.3	9.8	0.2	2.4	0.1	1.1	0.0	231.0	237.0	235.0	223.6	229.6	227.6	222.2	228.2	226.2
2992	6.7	6.24	6.24		1.36			0.3	0.4	12.1	0.2	3.0	0.1	1.3	0.0	233.5	239.5	237.5	224.3	230.3	228.3	222.6	228.6	226.6
3590	8.0	7.48	7.48		1.63			0.4	0.6	17.4	0.3	4.3	0.1	1.9	0.0	239.2	245.2	243.2	225.9	231.9	229.9	223.4	229.4	227.4
4039	9.0	8.42	8.42		1.83			0.5	0.7	22.0	0.4	5.5	0.1	2.4	0.1	244.1	250.1	248.1	227.3	233.3	231.3	224.2	230.2	228.2
4488	10.0	9.35	9.35		2.04			0.6	0.8	27.2	0.5	6.8	0.1	3.0	0.1	249.7	255.7	253.7	228.9	234.9	232.9	225.1	231.1	229.1
4937	11.0	10.29	10.29		2.24			0.8	1.0	32.9	0.6	8.2	0.2	3.7	0.1	255.8	261.8	259.8	230.7	236.7	234.7	226.0	232.0	230.0
5386	12.0	11.22	11.22		2.44			0.9	1.2	39.1	0.7	9.8	0.2	4.3	0.1	262.5	268.5	266.5	232.6	238.6	236.6	227.0	233.0	231.0
5984	13.3	12.47	12.47		2.72			1.1	1.4	48.3	0.9	12.1	0.2	5.4	0.1	272.3	278.3	276.3	235.4	241.4	239.4	228.6	234.6	232.6
6283	14.0	13.09	13.09		2.85			1.3	1.6	53.2	1.0	13.3	0.3	5.9	0.1	277.5	283.5	281.5	236.9	242.9	240.9	229.4	235.4	233.4
6732	15.0	14.03	14.03		3.06			1.4	1.8	61.1	1.1	15.3	0.3	6.8	0.1	286.0	292.0	290.0	239.3	245.3	243.3	230.7	236.7	234.7
7181	16.0	14.97	14.97		3.26			1.6	2.0	69.6	1.2	17.4	0.3	7.7	0.2	294.9	300.9	298.9	241.9	247.9	245.9	232.0	238.0	236.0
7630	17.0	15.90	15.90		3.46			1.9	2.2	78.5	1.4	19.6	0.4	8.7	0.2	304.5	310.5	308.5	244.6	250.6	248.6	233.5	239.5	237.5
8078	18.0	16.84	16.84		3.67			2.1	2.5	88.0	1.5	22.0	0.4	9.8	0.2	314.6	320.6	318.6	247.5	253.5	251.5	235.1	241.1	239.1
8527	19.0	17.77	17.77		3.87			2.3	2.7	98.1	1.7	24.5	0.5	10.9	0.2	325.4	331.4	329.4	250.6	256.6	254.6	236.7	242.7	240.7
8976	20.0	18.71	18.71		4.07			2.6	3.0	108.7	1.9	27.2	0.5	12.1	0.2	336.7	342.7	340.7	253.8	259.8	257.8	238.4	244.4	242.4
9425	21.0	19.64	19.64		4.28			2.8	3.3	119.8	2.1	30.0	0.6	13.3	0.3	348.5	354.5	352.5	257.2	263.2	261.2	240.2	246.2	244.2
9874	22.0	20.58	20.58		4.48			3.1	3.6	131.5	2.2	32.9	0.6	14.6	0.3	361.0	367.0	365.0	260.7	266.7	264.7	242.1	248.1	246.1
10322	23.0	21.51	21.51		4.68			3.4	3.9	143.7	2.4	35.9	0.7	16.0	0.3	374.0	380.0	378.0	264.4	270.4	268.4	244.1	250.1	248.1
10771	24.0	22.45	22.45		4.89			3.7	4.2	156.5	2.6	39.1	0.7	17.4	0.3	387.6	393.6	391.6	268.3	274.3	272.3	246.2	252.2	250.2
11220	25.0	23.38	23.38		5.09			4.0	4.6	169.8	2.8	42.5	0.8	18.9	0.4	401.7	407.7	405.7	272.3	278.3	276.3	248.3	254.3	252.3
11669	26.0	24.32	24.32		5.30			4.4	4.9	183.7	3.1	45.9	0.8	20.4	0.4	416.5	422.5	420.5	276.5	282.5	280.5	250.6	256.6	254.6
12118	27.0	25.25	25.25		5.50			4.7	5.3	198.1	3.3	49.5	0.9	22.0	0.4	431.8	437.8	435.8	280.9	286.9	284.9	252.9	258.9	256.9
12566	28.0	26.19	26.19		5.70			5.1	5.6	213.0	3.5	53.2	1.0	23.7	0.5	447.7	453.7	451.7	285.4	291.4	289.4	255.3	261.3	259.3
13015	29.0	27.12	27.12		5.91			5.4	6.0	228.5	3.7	57.1	1.0	25.4	0.5	464.2	470.2	468.2	290.1	296.1	294.1	257.8	263.8	261.8
13464	30.0	28.06	28.06		6.11			5.8	6.4	244.5	4.0	61.1	1.1	27.2	0.5	481.2	487.2	485.2	294.9	300.9	298.9	260.4	266.4	264.4



PID Pump Exchange - Appraisal Study - Alternative Alignment 2 - 20 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 2 - 40 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING		PS P	IPING		DISCHAR	GE PIPING
ELEV	921.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	926.0 feet	Wet Well (High River)	NOM. DIAM. (in)					18	18		36
LWL	922.0 feet	Wet Well (Low River)	O.D. (in)					18	18		36
			I.D. (in)					18	18		36
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1146.0 feet	Canal Bottom	С					110	110		130
HIGH	1148.5 feet	Canal (High Flow)	LENGTH (feet)					8	12		1,790
LWL	1146.5 feet	Canal (Low Flow)	K	1				10	10		10

ROPOSED	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	5985	13.3	244	526.4
2	11970	26.7	247	1066.6
3	17952	40.0	252	1632.8
'Assumes	70%	Efficiency		

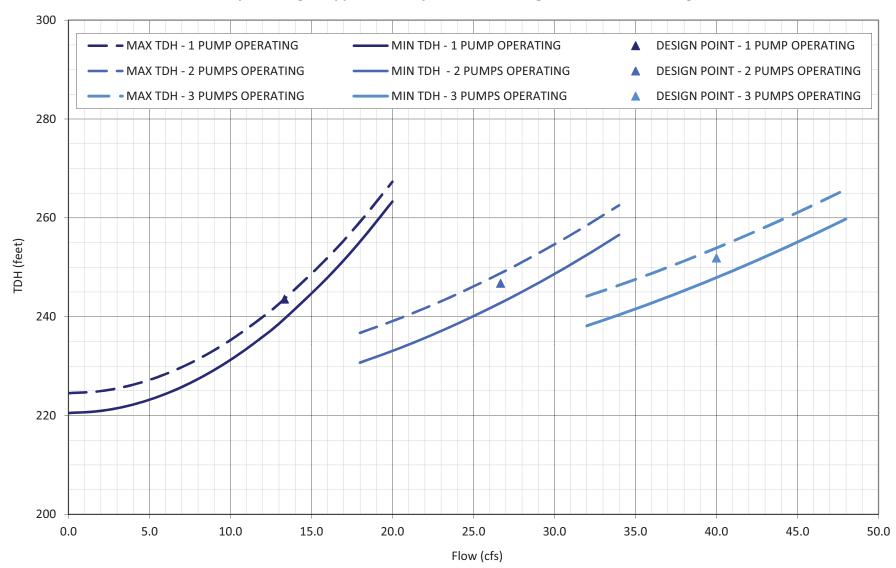
BY:

DATE:

David Rice, P.E.

13-Sep-12

TOTAL	. FLOW		VELO	CITIES		SUCTION	LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	220.5	226.5	224.5	220.5	226.5	224.5	220.5	226.5	224.5
898	2.0	1.13	1.13		0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	220.9	226.9	224.9	220.6	226.6	224.6	220.6	226.6	224.6
1795	4.0	2.26	2.26		0.57			0.0	0.1	1.6	0.0	0.4	0.0	0.2	0.0	222.2	228.2	226.2	221.0	227.0	225.0	220.8	226.8	224.8
2693	6.0	3.39	3.39		0.85			0.1	0.1	3.6	0.1	0.9	0.0	0.4	0.0	224.4	230.4	228.4	221.7	227.7	225.7	221.2	227.2	225.2
3590	8.0	4.53	4.53		1.13			0.2	0.2	6.4	0.1	1.6	0.0	0.7	0.0	227.4	233.4	231.4	222.5	228.5	226.5	221.6	227.6	225.6
4488	10.0	5.66	5.66		1.41			0.3	0.3	9.9	0.2	2.5	0.0	1.1	0.0	231.3	237.3	235.3	223.7	229.7	227.7	222.3	228.3	226.3
5386	12.0	6.79	6.79		1.70			0.4	0.5	14.3	0.2	3.6	0.1	1.6	0.0	236.0	242.0	240.0	225.1	231.1	229.1	223.0	229.0	227.0
5985	13.3	7.55	7.55		1.89			0.6	0.6	17.7	0.3	4.4	0.1	2.0	0.0	239.6	245.6	243.6	226.1	232.1	230.1	223.6	229.6	227.6
7181	16.0	9.05	9.05		2.26			0.8	0.8	25.5	0.4	6.4	0.1	2.8	0.0	247.9	253.9	251.9	228.6	234.6	232.6	225.0	231.0	229.0
8078	18.0	10.18	10.18		2.55			1.0	1.0	32.2	0.5	8.1	0.1	3.6	0.1	255.2	261.2	259.2	230.7	236.7	234.7	226.2	232.2	230.2
8976	20.0	11.32	11.32		2.83			1.2	1.2	39.8	0.6	9.9	0.2	4.4	0.1	263.3	269.3	267.3	233.1	239.1	237.1	227.5	233.5	231.5
9874	22.0	12.45	12.45		3.11			1.5	1.5	48.1	0.7	12.0	0.2	5.3	0.1	272.3	278.3	276.3	235.7	241.7	239.7	228.9	234.9	232.9
10771	24.0	13.58	13.58		3.39			1.8	1.7	57.3	0.8	14.3	0.2	6.4	0.1	282.1	288.1	286.1	238.6	244.6	242.6	230.5	236.5	234.5
11970	26.7	15.09	15.09		3.77			2.2	2.1	70.7	0.9	17.7	0.3	7.9	0.1	296.5	302.5	300.5	242.8	248.8	246.8	232.8	238.8	236.8
12566	28.0	15.84	15.84		3.96			2.4	2.3	77.9	1.0	19.5	0.3	8.7	0.1	304.2	310.2	308.2	245.0	251.0	249.0	234.0	240.0	238.0
13464	30.0	16.97	16.97		4.24			2.8	2.6	89.5	1.2	22.4	0.3	9.9	0.2	316.6	322.6	320.6	248.6	254.6	252.6	236.0	242.0	240.0
14362	32.0	18.11	18.11		4.53			3.2	3.0	101.8	1.3	25.5	0.4	11.3	0.2	329.8	335.8	333.8	252.5	258.5	256.5	238.1	244.1	242.1
15259	34.0	19.24	19.24		4.81			3.6	3.3	114.9	1.5	28.7	0.4	12.8	0.2	343.8	349.8	347.8	256.6	262.6	260.6	240.4	246.4	244.4
16157	36.0	20.37	20.37		5.09			4.0	3.7	128.9	1.6	32.2	0.5	14.3	0.2	358.7	364.7	362.7	260.9	266.9	264.9	242.7	248.7	246.7
17054	38.0	21.50	21.50		5.38			4.5	4.1	143.6	1.8	35.9	0.5	16.0	0.2	374.4	380.4	378.4	265.5	271.5	269.5	245.3	251.3	249.3
17952	40.0	22.63	22.63		5.66			5.0	4.5	159.1	2.0	39.8	0.6	17.7	0.3	391.0	397.0	395.0	270.3	276.3	274.3	247.9	253.9	251.9
18850	42.0	23.76	23.76		5.94			5.5	4.9	175.4	2.2	43.8	0.6	19.5	0.3	408.5	414.5	412.5	275.3	281.3	279.3	250.7	256.7	254.7
19747	44.0	24.90	24.90		6.22			6.0	5.3	192.5	2.4	48.1	0.7	21.4	0.3	426.7	432.7	430.7	280.6	286.6	284.6	253.6	259.6	257.6
20645	46.0	26.03	26.03		6.51			6.6	5.8	210.4	2.6	52.6	0.7	23.4	0.3	445.8	451.8	449.8	286.2	292.2	290.2	256.6	262.6	260.6
21542	48.0	27.16	27.16		6.79			7.2	6.3	229.1	2.8	57.3	0.8	25.5	0.4	465.8	471.8	469.8	292.0	298.0	296.0	259.8	265.8	263.8
22440	50.0	28.29	28.29		7.07			7.8	6.8	248.6	3.0	62.1	0.8	27.6	0.4	486.6	492.6	490.6	298.0	304.0	302.0	263.1	269.1	267.1
23338	52.0	29.42	29.42		7.36			8.4	7.3	268.8	3.2	67.2	0.9	29.9	0.4	508.3	514.3	512.3	304.3	310.3	308.3	266.5	272.5	270.5
24235	54.0	30.55	30.55		7.64			9.1	7.8	289.9	3.5	72.5	1.0	32.2	0.5	530.8	536.8	534.8	310.8	316.8	314.8	270.0	276.0	274.0
25133	56.0	31.69	31.69		7.92			9.7	8.4	311.8	3.7	77.9	1.0	34.6	0.5	554.1	560.1	558.1	317.6	323.6	321.6	273.7	279.7	277.7
26030	58.0	32.82	32.82		8.20			10.5	8.9	334.5	4.0	83.6	1.1	37.2	0.5	578.3	584.3	582.3	324.6	330.6	328.6	277.6	283.6	281.6
26928	60.0	33.95	33.95		8.49			11.2	9.5	357.9	4.2	89.5	1.2	39.8	0.6	603.3	609.3	607.3	331.8	337.8	335.8	281.5	287.5	285.5



PID Pump Exchange - Appraisal Study - Alternative Alignment 2 - 40 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 3 - 10 CFS DESIGN FLOW

CTION WA	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTIO	N PIPING		PS PI	PING		DISCHAR	GE PIPING
LEV	934.0 feet	Invert at Diversion		PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS
HWL	939.0 feet	Wet Well (High River)		NOM. DIAM. (in)					12	12		20
LWL	935.0 feet	Wet Well (Low River)		O.D. (in)					12	12		20
			_	I.D. (in)					12	12		20
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE
ELEV	1152.0 feet	Canal Bottom		С					110	110		130
HIGH	1154.5 feet	Canal (High Flow)		LENGTH (feet)					8	12		1,490
LWL	1152.5 feet	Canal (Low Flow)		К					10	10		10

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2244	5.0	233	188.5
2	4488	10.0	239	386.8
*Assumes	70%	Efficiency		

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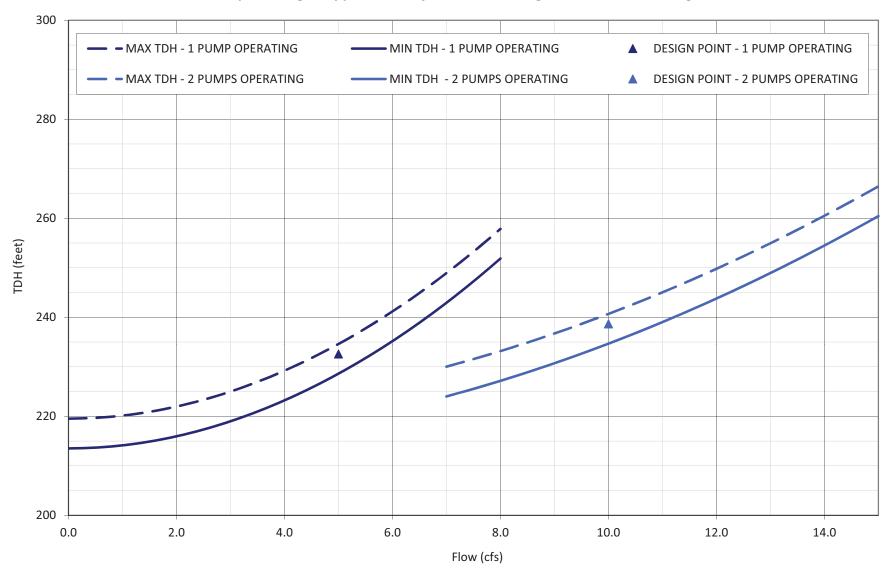
DATE:

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13-Sep-12

TOTAL	. FLOW		VELO	CITIES		SUCTION	LOSSES	DISCHAR	GE LOSSES	PS LOSSES	5 - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0			213.5	219.5	217.5	213.5	219.5	217.5			
224	0.5	0.64	0.64		0.23			0.0	0.0	0.1	0.0	0.0	0.0			213.7	219.7	217.7	213.6	219.6	217.6			
449	1.0	1.27	1.27		0.46			0.0	0.1	0.5	0.0	0.1	0.0			214.1	220.1	218.1	213.7	219.7	217.7			
673	1.5	1.91	1.91		0.69			0.1	0.2	1.1	0.0	0.3	0.0			214.9	220.9	218.9	214.0	220.0	218.0			
898	2.0	2.55	2.55		0.92			0.1	0.3	2.0	0.1	0.5	0.0			216.0	222.0	220.0	214.4	220.4	218.4			
1122	2.5	3.18	3.18		1.15			0.2	0.4	3.1	0.1	0.8	0.0			217.3	223.3	221.3	214.9	220.9	218.9			
1346	3.0	3.82	3.82		1.37			0.3	0.5	4.5	0.1	1.1	0.0			219.0	225.0	223.0	215.5	221.5	219.5			
1571	3.5	4.46	4.46		1.60			0.4	0.7	6.2	0.2	1.5	0.0			220.9	226.9	224.9	216.2	222.2	220.2			
1795	4.0	5.09	5.09		1.83			0.5	0.9	8.1	0.2	2.0	0.1			223.2	229.2	227.2	217.0	223.0	221.0			
2020	4.5	5.73	5.73		2.06			0.7	1.1	10.2	0.3	2.5	0.1			225.8	231.8	229.8	217.9	223.9	221.9			
2244	5.0	6.37	6.37		2.29			0.8	1.4	12.6	0.3	3.1	0.1			228.6	234.6	232.6	218.9	224.9	222.9			
2468	5.5	7.00	7.00		2.52			1.0	1.7	15.2	0.4	3.8	0.1			231.7	237.7	235.7	220.1	226.1	224.1			
2693	6.0	7.64	7.64		2.75			1.2	2.0	18.1	0.4	4.5	0.1			235.2	241.2	239.2	221.3	227.3	225.3			
2917	6.5	8.27	8.27		2.98			1.4	2.3	21.3	0.5	5.3	0.1			238.9	244.9	242.9	222.6	228.6	226.6			
3142	7.0	8.91	8.91		3.21			1.6	2.6	24.7	0.6	6.2	0.2			242.9	248.9	246.9	224.0	230.0	228.0			
3366	7.5	9.55	9.55		3.44			1.8	3.0	28.3	0.6	7.1	0.2			247.2	253.2	251.2	225.5	231.5	229.5			
3590	8.0	10.18	10.18		3.67			2.1	3.3	32.2	0.7	8.1	0.2			251.9	257.9	255.9	227.2	233.2	231.2			
3815	8.5	10.82	10.82		3.90			2.4	3.7	36.4	0.8	9.1	0.2			256.8	262.8	260.8	228.9	234.9	232.9			
4039	9.0	11.46	11.46		4.12			2.6	4.1	40.8	0.9	10.2	0.3			262.0	268.0	266.0	230.7	236.7	234.7			
4264	9.5	12.09	12.09		4.35			2.9	4.6	45.4	1.0	11.4	0.3			267.4	273.4	271.4	232.7	238.7	236.7			
4488	10.0	12.73	12.73		4.58			3.3	5.0	50.3	1.1	12.6	0.3			273.2	279.2	277.2	234.7	240.7	238.7			
4712	10.5	13.37	13.37		4.81			3.6	5.5	55.5	1.2	13.9	0.3			279.3	285.3	283.3	236.8	242.8	240.8			
4937	11.0	14.00	14.00		5.04			3.9	6.0	60.9	1.3	15.2	0.4			285.7	291.7	289.7	239.0	245.0	243.0			
5161	11.5	14.64	14.64		5.27			4.3	6.5	66.6	1.4	16.6	0.4			292.3	298.3	296.3	241.4	247.4	245.4			
5386	12.0	15.28	15.28		5.50			4.7	7.0	72.5	1.5	18.1	0.4			299.3	305.3	303.3	243.8	249.8	247.8			
5610	12.5	15.91	15.91		5.73			5.1	7.6	78.6	1.7	19.7	0.5			306.5	312.5	310.5	246.3	252.3	250.3			
5834	13.0	16.55	16.55		5.96			5.5	8.2	85.1	1.8	21.3	0.5			314.0	320.0	318.0	248.9	254.9	252.9			
6059	13.5	17.19	17.19		6.19			5.9	8.8	91.7	1.9	22.9	0.5			321.9	327.9	325.9	251.7	257.7	255.7			
6283	14.0	17.82	17.82		6.42			6.4	9.4	98.7	2.1	24.7	0.6			330.0	336.0	334.0	254.5	260.5	258.5			
6508	14.5	18.46	18.46		6.65			6.9	10.0	105.8	2.2	26.5	0.6			338.4	344.4	342.4	257.4	263.4	261.4			
6732	15.0	19.10	19.10		6.87			7.3	10.6	113.2	2.3	28.3	0.6			347.1	353.1	351.1	260.4	266.4	264.4			

ANCHOR QEA, LLC



PID Pump Exchange - Appraisal Study - Alternative Alignment 3 - 10 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 3 - 20 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTION PIPING						DISCHARGE PIPING		
ELEV	934.0 feet	Invert at Diversion		PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRAN	
HWL	939.0 feet			NOM. DIAM. (in)					14	14		30	
LWL	935.0 feet	Wet Well (Low River)		O.D. (in)					14	14		30	
				I.D. (in)					14	14		30	
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE	
ELEV	1152.0 feet	Canal Bottom		С					110	110		130	
HIGH	H 1154.5 feet Canal (High Flow)			LENGTH (feet)					8	12		1,490	
LWL	1152.5 feet	Canal (Low Flow)		К					10	10		10	

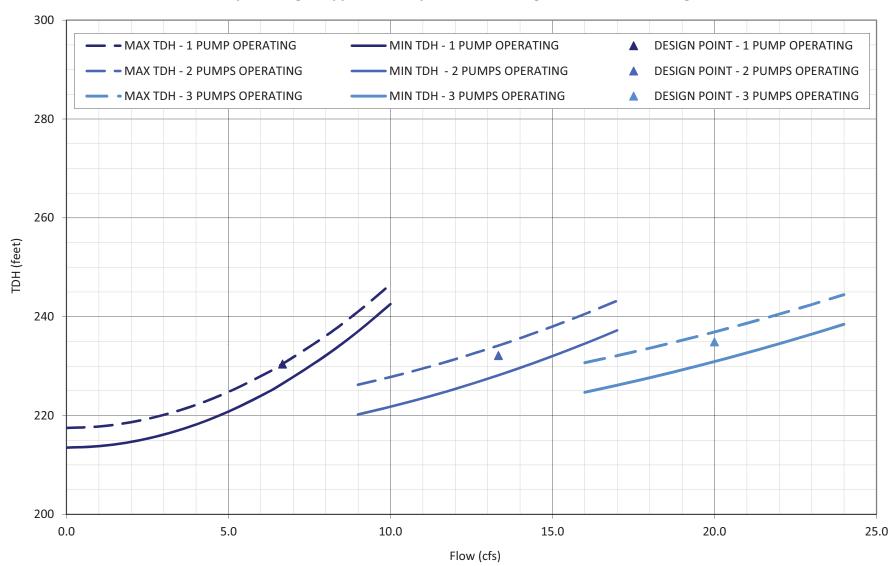
		DATE:	13-Sep-12	
PROPOSED D	DESIGN POIN	rs:		
PUMPS	FLOW	FLOW	TDH	PO

David Rice, P.E.

FLOW	FLOW	TDH	POWER
(GPM)	(CFS)	(FT)	(HP)*
2992	6.7	230	249.0
5984	13.3	232	501.6
8976	20.0	235	761.4
70%	Efficiency		
	(GPM) 2992 5984 8976	(GPM) (CFS) 2992 6.7 5984 13.3 8976 20.0	(GPM) (CFS) (FT) 2992 6.7 230 5984 13.3 232 8976 20.0 235

BY:

TOTAI	. FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	5 - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	S - 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD) - 3 PUMPS
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	213.5	219.5	217.5	213.5	219.5	217.5	213.5	219.5	217.5
449	1.0	0.94	0.94		0.20			0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	213.8	219.8	217.8	213.6	219.6	217.6	213.5	219.5	217.5
898	2.0	1.87	1.87		0.41			0.0	0.0	1.1	0.0	0.3	0.0	0.1	0.0	214.7	220.7	218.7	213.8	219.8	217.8	213.7	219.7	217.7
1346	3.0	2.81	2.81		0.61			0.1	0.1	2.4	0.1	0.6	0.0	0.3	0.0	216.1	222.1	220.1	214.3	220.3	218.3	213.9	219.9	217.9
1795	4.0	3.74	3.74		0.81			0.1	0.1	4.3	0.1	1.1	0.0	0.5	0.0	218.2	224.2	222.2	214.8	220.8	218.8	214.2	220.2	218.2
2244	5.0	4.68	4.68		1.02			0.2	0.2	6.8	0.1	1.7	0.0	0.8	0.0	220.8	226.8	224.8	215.6	221.6	219.6	214.6	220.6	218.6
2693	6.0	5.61	5.61		1.22			0.2	0.3	9.8	0.2	2.4	0.1	1.1	0.0	224.0	230.0	228.0	216.5	222.5	220.5	215.1	221.1	219.1
2992	6.7	6.24	6.24		1.36			0.3	0.3	12.1	0.2	3.0	0.1	1.3	0.0	226.4	232.4	230.4	217.2	223.2	221.2	215.5	221.5	219.5
3590	8.0	7.48	7.48		1.63			0.4	0.5	17.4	0.3	4.3	0.1	1.9	0.0	232.1	238.1	236.1	218.8	224.8	222.8	216.4	222.4	220.4
4039	9.0	8.42	8.42		1.83			0.5	0.6	22.0	0.4	5.5	0.1	2.4	0.1	237.0	243.0	241.0	220.2	226.2	224.2	217.1	223.1	221.1
4488	10.0	9.35	9.35		2.04			0.6	0.7	27.2	0.5	6.8	0.1	3.0	0.1	242.5	248.5	246.5	221.8	227.8	225.8	217.9	223.9	221.9
4937	11.0	10.29	10.29		2.24			0.8	0.8	32.9	0.6	8.2	0.2	3.7	0.1	248.6	254.6	252.6	223.5	229.5	227.5	218.8	224.8	222.8
5386	12.0	11.22	11.22		2.44			0.9	1.0	39.1	0.7	9.8	0.2	4.3	0.1	255.3	261.3	259.3	225.4	231.4	229.4	219.8	225.8	223.8
5984	13.3	12.47	12.47		2.72			1.1	1.2	48.3	0.9	12.1	0.2	5.4	0.1	265.0	271.0	269.0	228.2	234.2	232.2	221.3	227.3	225.3
6283	14.0	13.09	13.09		2.85			1.3	1.3	53.2	1.0	13.3	0.3	5.9	0.1	270.3	276.3	274.3	229.6	235.6	233.6	222.1	228.1	226.1
6732	15.0	14.03	14.03		3.06			1.4	1.5	61.1	1.1	15.3	0.3	6.8	0.1	278.7	284.7	282.7	232.0	238.0	236.0	223.4	229.4	227.4
7181	16.0	14.97	14.97		3.26			1.6	1.7	69.6	1.2	17.4	0.3	7.7	0.2	287.6	293.6	291.6	234.5	240.5	238.5	224.7	230.7	228.7
7630	17.0	15.90	15.90		3.46			1.9	1.9	78.5	1.4	19.6	0.4	8.7	0.2	297.1	303.1	301.1	237.2	243.2	241.2	226.1	232.1	230.1
8078	18.0	16.84	16.84		3.67			2.1	2.1	88.0	1.5	22.0	0.4	9.8	0.2	307.2	313.2	311.2	240.1	246.1	244.1	227.6	233.6	231.6
8527	19.0	17.77	17.77		3.87			2.3	2.3	98.1	1.7	24.5	0.5	10.9	0.2	317.9	323.9	321.9	243.1	249.1	247.1	229.2	235.2	233.2
8976	20.0	18.71	18.71		4.07			2.6	2.5	108.7	1.9	27.2	0.5	12.1	0.2	329.1	335.1	333.1	246.3	252.3	250.3	230.9	236.9	234.9
9425	21.0	19.64	19.64		4.28			2.8	2.8	119.8	2.1	30.0	0.6	13.3	0.3	341.0	347.0	345.0	249.6	255.6	253.6	232.7	238.7	236.7
9874	22.0	20.58	20.58		4.48			3.1	3.0	131.5	2.2	32.9	0.6	14.6	0.3	353.4	359.4	357.4	253.1	259.1	257.1	234.5	240.5	238.5
10322	23.0	21.51	21.51		4.68			3.4	3.3	143.7	2.4	35.9	0.7	16.0	0.3	366.3	372.3	370.3	256.8	262.8	260.8	236.5	242.5	240.5
10771	24.0	22.45	22.45		4.89			3.7	3.5	156.5	2.6	39.1	0.7	17.4	0.3	379.9	385.9	383.9	260.6	266.6	264.6	238.5	244.5	242.5
11220	25.0	23.38	23.38		5.09			4.0	3.8	169.8	2.8	42.5	0.8	18.9	0.4	394.0	400.0	398.0	264.6	270.6	268.6	240.6	246.6	244.6
11669	26.0	24.32	24.32		5.30			4.4	4.1	183.7	3.1	45.9	0.8	20.4	0.4	408.7	414.7	412.7	268.7	274.7	272.7	242.8	248.8	246.8
12118	27.0	25.25	25.25		5.50			4.7	4.4	198.1	3.3	49.5	0.9	22.0	0.4	423.9	429.9	427.9	273.0	279.0	277.0	245.0	251.0	249.0
12566	28.0	26.19	26.19		5.70			5.1	4.7	213.0	3.5	53.2	1.0	23.7	0.5	439.7	445.7	443.7	277.5	283.5	281.5	247.4	253.4	251.4
13015	29.0	27.12	27.12		5.91			5.4	5.0	228.5	3.7	57.1	1.0	25.4	0.5	456.1	462.1	460.1	282.1	288.1	286.1	249.8	255.8	253.8
13464	30.0	28.06	28.06		6.11			5.8	5.3	244.5	4.0	61.1	1.1	27.2	0.5	473.1	479.1	477.1	286.9	292.9	290.9	252.3	258.3	256.3



PID Pump Exchange - Appraisal Study - Alternative Alignment 3 - 20 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 3 - 40 CFS DESIGN FLOW

UCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTION PIPING						DISCHARGE PIPINO	
ELEV	934.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	939.0 feet	Wet Well (High River)	NOM. DIAM. (in)					18	18		36
LWL	935.0 feet	Wet Well (Low River)	O.D. (in)					18	18		36
			I.D. (in)					18	18		36
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1152.0 feet	Canal Bottom	С					110	110		130
HIGH	1154.5 feet Canal (High Flow)		LENGTH (feet)					8	12		1,490
LWL	1152.5 feet	Canal (Low Flow)	К			1		10	10		10

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	5995	13.4	237	512.1
2	11970	26.7	239	1034.8
3	17952	40.0	244	1582.6
*Assumes	70%	Efficiency		

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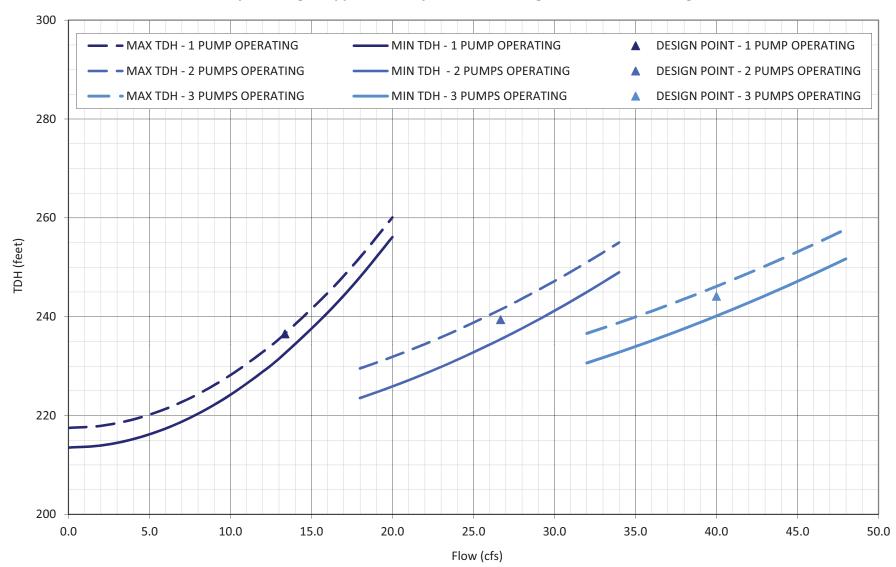
DATE:

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13-Sep-12

TOTAL	FLOW		VELO	CITIES		SUCTION	LOSSES	DISCHARO	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	D - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	213.5	219.5	217.5	213.5	219.5	217.5	213.5	219.5	217.5
898	2.0	1.13	1.13		0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	213.9	219.9	217.9	213.6	219.6	217.6	213.6	219.6	217.6
1795	4.0	2.26	2.26		0.57			0.0	0.1	1.6	0.0	0.4	0.0	0.2	0.0	215.2	221.2	219.2	214.0	220.0	218.0	213.8	219.8	217.8
2693	6.0	3.39	3.39		0.85			0.1	0.1	3.6	0.1	0.9	0.0	0.4	0.0	217.4	223.4	221.4	214.6	220.6	218.6	214.1	220.1	218.1
3590	8.0	4.53	4.53		1.13			0.2	0.2	6.4	0.1	1.6	0.0	0.7	0.0	220.4	226.4	224.4	215.5	221.5	219.5	214.6	220.6	218.6
4488	10.0	5.66	5.66		1.41			0.3	0.3	9.9	0.2	2.5	0.0	1.1	0.0	224.2	230.2	228.2	216.6	222.6	220.6	215.2	221.2	219.2
5386	12.0	6.79	6.79		1.70			0.4	0.4	14.3	0.2	3.6	0.1	1.6	0.0	228.9	234.9	232.9	218.0	224.0	222.0	216.0	222.0	220.0
5995	13.4	7.56	7.56		1.89			0.6	0.5	17.7	0.3	4.4	0.1	2.0	0.0	232.5	238.5	236.5	219.1	225.1	223.1	216.6	222.6	220.6
7181	16.0	9.05	9.05		2.26			0.8	0.7	25.5	0.4	6.4	0.1	2.8	0.0	240.8	246.8	244.8	221.4	227.4	225.4	217.9	223.9	221.9
8078	18.0	10.18	10.18		2.55			1.0	0.9	32.2	0.5	8.1	0.1	3.6	0.1	248.0	254.0	252.0	223.5	229.5	227.5	219.0	225.0	223.0
8976	20.0	11.32	11.32		2.83			1.2	1.0	39.8	0.6	9.9	0.2	4.4	0.1	256.1	262.1	260.1	225.9	231.9	229.9	220.3	226.3	224.3
9874	22.0	12.45	12.45		3.11			1.5	1.2	48.1	0.7	12.0	0.2	5.3	0.1	265.0	271.0	269.0	228.5	234.5	232.5	221.7	227.7	225.7
10771	24.0	13.58	13.58		3.39			1.8	1.5	57.3	0.8	14.3	0.2	6.4	0.1	274.8	280.8	278.8	231.3	237.3	235.3	223.2	229.2	227.2
11970	26.7	15.09	15.09		3.77			2.2	1.8	70.7	0.9	17.7	0.3	7.9	0.1	289.1	295.1	293.1	235.4	241.4	239.4	225.5	231.5	229.5
12566	28.0	15.84	15.84		3.96			2.4	1.9	77.9	1.0	19.5	0.3	8.7	0.1	296.8	302.8	300.8	237.6	243.6	241.6	226.7	232.7	230.7
13464	30.0	16.97	16.97		4.24			2.8	2.2	89.5	1.2	22.4	0.3	9.9	0.2	309.1	315.1	313.1	241.2	247.2	245.2	228.6	234.6	232.6
14362	32.0	18.11	18.11		4.53			3.2	2.5	101.8	1.3	25.5	0.4	11.3	0.2	322.3	328.3	326.3	245.0	251.0	249.0	230.6	236.6	234.6
15259	34.0	19.24	19.24		4.81			3.6	2.8	114.9	1.5	28.7	0.4	12.8	0.2	336.3	342.3	340.3	249.0	255.0	253.0	232.8	238.8	236.8
16157	36.0	20.37	20.37		5.09			4.0	3.1	128.9	1.6	32.2	0.5	14.3	0.2	351.1	357.1	355.1	253.3	259.3	257.3	235.1	241.1	239.1
17054	38.0	21.50	21.50		5.38			4.5	3.4	143.6	1.8	35.9	0.5	16.0	0.2	366.8	372.8	370.8	257.8	263.8	261.8	237.6	243.6	241.6
17952	40.0	22.63	22.63		5.66			5.0	3.7	159.1	2.0	39.8	0.6	17.7	0.3	383.3	389.3	387.3	262.5	268.5	266.5	240.1	246.1	244.1
18850	42.0	23.76	23.76		5.94			5.5	4.1	175.4	2.2	43.8	0.6	19.5	0.3	400.6	406.6	404.6	267.5	273.5	271.5	242.8	248.8	246.8
19747	44.0	24.90	24.90		6.22			6.0	4.5	192.5	2.4	48.1	0.7	21.4	0.3	418.8	424.8	422.8	272.7	278.7	276.7	245.7	251.7	249.7
20645	46.0	26.03	26.03		6.51			6.6	4.8	210.4	2.6	52.6	0.7	23.4	0.3	437.9	443.9	441.9	278.2	284.2	282.2	248.6	254.6	252.6
21542	48.0	27.16	27.16		6.79			7.2	5.2	229.1	2.8	57.3	0.8	25.5	0.4	457.8	463.8	461.8	283.9	289.9	287.9	251.7	257.7	255.7
22440	50.0	28.29	28.29		7.07			7.8	5.6	248.6	3.0	62.1	0.8	27.6	0.4	478.5	484.5	482.5	289.9	295.9	293.9	254.9	260.9	258.9
23338	52.0	29.42	29.42		7.36			8.4	6.1	268.8	3.2	67.2	0.9	29.9	0.4	500.0	506.0	504.0	296.1	302.1	300.1	258.3	264.3	262.3
24235	54.0	30.55	30.55		7.64			9.1	6.5	289.9	3.5	72.5	1.0	32.2	0.5	522.5	528.5	526.5	302.5	308.5	306.5	261.7	267.7	265.7
25133	56.0	31.69	31.69		7.92			9.7	7.0	311.8	3.7	77.9	1.0	34.6	0.5	545.7	551.7	549.7	309.2	315.2	313.2	265.3	271.3	269.3
26030	58.0	32.82	32.82		8.20			10.5	7.4	334.5	4.0	83.6	1.1	37.2	0.5	569.8	575.8	573.8	316.1	322.1	320.1	269.1	275.1	273.1
26928	60.0	33.95	33.95		8.49			11.2	7.9	357.9	4.2	89.5	1.2	39.8	0.6	594.7	600.7	598.7	323.2	329.2	327.2	272.9	278.9	276.9

PID Pump Exchange - Hydraulic Analysis - Alignment 3 - TO PID.xlsx



PID Pump Exchange - Appraisal Study - Alternative Alignment 3 - 40 CFS Design Flow

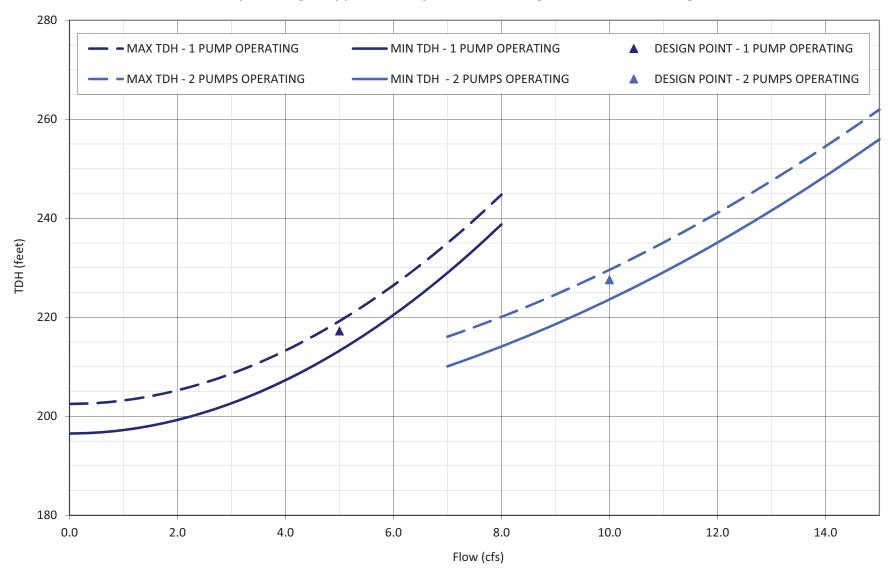
SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 4 - 10 CFS DESIGN FLOW

BY:	David Rice, P.E.
DATE:	13-Sep-12

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTION PIPING						DISCHARGE PIPING		
ELEV	952.0 feet	Invert at Diversion		PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.	
HWL	957.0 feet	Wet Well (High River)		NOM. DIAM. (in)					12	12		20	
LWL	953.0 feet	Wet Well (Low River)		O.D. (in)					12	12		20	
				I.D. (in)					12	12		20	
DISCHARGE	HARGE WATER SURFACE ELEVATIONS:			MATERIAL					STEEL	STEEL		HDPE	
ELEV	1153.0 feet	Canal Bottom		С					110	110		130	
HIGH	1155.5 feet	Canal (High Flow)		LENGTH (feet)					8	12		3,240	
LWL	1153.5 feet	3.5 feet Canal (Low Flow)		K					10	10		10	

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2244	5.0	217	176.0
2	4488	10.0	228	368.8
*Assumes	70%	Efficiency		

TOTAL	. FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	5 - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	D - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0			196.5	202.5	200.5	196.5	202.5	200.5			
224	0.5	0.64	0.64		0.23			0.0	0.0	0.1	0.0	0.0	0.0			196.7	202.7	200.7	196.6	202.6	200.6			
449	1.0	1.27	1.27		0.46			0.0	0.2	0.5	0.0	0.1	0.0			197.2	203.2	201.2	196.8	202.8	200.8			
673	1.5	1.91	1.91		0.69			0.1	0.3	1.1	0.0	0.3	0.0			198.1	204.1	202.1	197.2	203.2	201.2			
898	2.0	2.55	2.55		0.92			0.1	0.6	2.0	0.1	0.5	0.0			199.3	205.3	203.3	197.7	203.7	201.7			
1122	2.5	3.18	3.18		1.15			0.2	0.8	3.1	0.1	0.8	0.0			200.8	206.8	204.8	198.4	204.4	202.4			
1346	3.0	3.82	3.82		1.37			0.3	1.2	4.5	0.1	1.1	0.0			202.6	208.6	206.6	199.1	205.1	203.1			
1571	3.5	4.46	4.46		1.60			0.4	1.6	6.2	0.2	1.5	0.0			204.8	210.8	208.8	200.1	206.1	204.1			
1795	4.0	5.09	5.09		1.83			0.5	2.0	8.1	0.2	2.0	0.1			207.3	213.3	211.3	201.1	207.1	205.1			
2020	4.5	5.73	5.73		2.06			0.7	2.5	10.2	0.3	2.5	0.1			210.1	216.1	214.1	202.3	208.3	206.3			
2244	5.0	6.37	6.37		2.29			0.8	3.0	12.6	0.3	3.1	0.1			213.2	219.2	217.2	203.6	209.6	207.6			
2468	5.5	7.00	7.00		2.52			1.0	3.6	15.2	0.4	3.8	0.1			216.7	222.7	220.7	205.0	211.0	209.0			
2693	6.0	7.64	7.64		2.75			1.2	4.2	18.1	0.4	4.5	0.1			220.5	226.5	224.5	206.6	212.6	210.6			
2917	6.5	8.27	8.27		2.98			1.4	4.9	21.3	0.5	5.3	0.1			224.6	230.6	228.6	208.3	214.3	212.3			
3142	7.0	8.91	8.91		3.21			1.6	5.7	24.7	0.6	6.2	0.2			229.0	235.0	233.0	210.1	216.1	214.1			
3366	7.5	9.55	9.55		3.44			1.8	6.4	28.3	0.6	7.1	0.2			233.7	239.7	237.7	212.0	218.0	216.0			
3590	8.0	10.18	10.18		3.67			2.1	7.2	32.2	0.7	8.1	0.2			238.8	244.8	242.8	214.1	220.1	218.1			
3815	8.5	10.82	10.82		3.90			2.4	8.1	36.4	0.8	9.1	0.2			244.1	250.1	248.1	216.3	222.3	220.3			
4039	9.0	11.46	11.46		4.12			2.6	9.0	40.8	0.9	10.2	0.3			249.8	255.8	253.8	218.6	224.6	222.6			
4264	9.5	12.09	12.09		4.35			2.9	9.9	45.4	1.0	11.4	0.3			255.8	261.8	259.8	221.0	227.0	225.0			
4488	10.0	12.73	12.73		4.58			3.3	10.9	50.3	1.1	12.6	0.3			262.1	268.1	266.1	223.6	229.6	227.6			
4712	10.5	13.37	13.37		4.81			3.6	12.0	55.5	1.2	13.9	0.3			268.8	274.8	272.8	226.3	232.3	230.3			
4937	11.0	14.00	14.00		5.04			3.9	13.0	60.9	1.3	15.2	0.4			275.7	281.7	279.7	229.1	235.1	233.1			
5161	11.5	14.64	14.64		5.27			4.3	14.2	66.6	1.4	16.6	0.4			283.0	289.0	287.0	232.0	238.0	236.0			
5386	12.0	15.28	15.28		5.50			4.7	15.3	72.5	1.5	18.1	0.4			290.5	296.5	294.5	235.1	241.1	239.1			
5610	12.5	15.91	15.91		5.73			5.1	16.5	78.6	1.7	19.7	0.5			298.4	304.4	302.4	238.2	244.2	242.2			
5834	13.0	16.55	16.55		5.96			5.5	17.8	85.1	1.8	21.3	0.5			306.6	312.6	310.6	241.5	247.5	245.5			
6059	13.5	17.19	17.19		6.19			5.9	19.0	91.7	1.9	22.9	0.5			315.2	321.2	319.2	245.0	251.0	249.0			
6283	14.0	17.82	17.82		6.42			6.4	20.4	98.7	2.1	24.7	0.6			324.0	330.0	328.0	248.5	254.5	252.5			
6508	14.5	18.46	18.46		6.65			6.9	21.7	105.8	2.2	26.5	0.6			333.1	339.1	337.1	252.2	258.2	256.2			
6732	15.0	19.10	19.10		6.87			7.3	23.1	113.2	2.3	28.3	0.6			342.6	348.6	346.6	255.9	261.9	259.9			



PID Pump Exchange - Appraisal Study - Alternative Alignment 4 - 10 CFS Design Flow

SYSTEM CURVE CALCULATION

PROJECT: PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 4 - 20 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTION PIPING			DISCHARGE PIPING				
ELEV	952.0 feet	Invert at Diversion		PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS
HWL	957.0 feet	Wet Well (High River)		NOM. DIAM. (in)					14	14		30
LWL	953.0 feet	Wet Well (Low River)		O.D. (in)					14	14		30
			_	I.D. (in)					14	14		30
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE
ELEV	1153.0 feet	Canal Bottom		С					110	110		130
HIGH	1155.5 feet	Canal (High Flow)		LENGTH (feet)					8	12		3,240
LWL	1153.5 feet	Canal (Low Flow)		К			1	1	10	10		10

PROPOSED DESIGN POINTS:														
PUMPS ON	FLOW (GPM)	FLOW (CFS)	TDH (FT)	POWER (HP)*										
1	2992	6.7	214	231.0										
2	5984	13.3	217	467.9										
-														

20.0

David Rice, P.E.

13-Sep-12

221

715.9

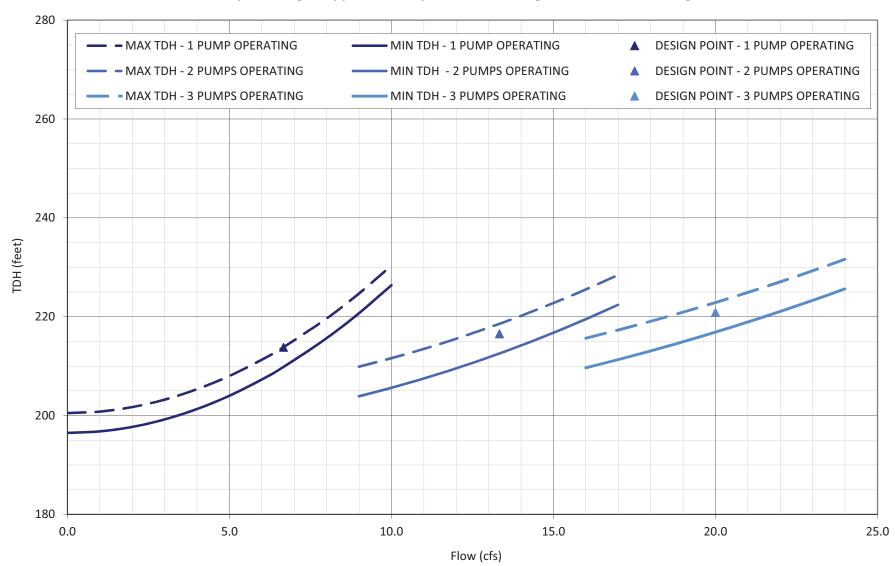
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DATE:

HIGH	1155.5		Canal (High I	,				LENGT	H (feet)					8	12		3,240							
LWL	1153.5 feet Canal (Low Flow)				1	ĸ					10	10		10		*Assumes	70%	Efficiency						
						-												•						
TOTAL	TOTAL FLOW VELOCITIES		SUCTION	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	5 - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DYNAMIC HEAD - 1 PUMP			TOTAL DYNAMIC HEAD - 2 PUMPS			TOTAL DYNAMIC HEAD - 3 PUM					
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00	1		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	196.5	202.5	200.5	196.5	202.5	200.5	196.5	202.5	200.5
449	1.0	0.94	0.94		0.20	1		0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	196.8	202.8	200.8	196.6	202.6	200.6	196.6	202.6	200.6
898	2.0	1.87	1.87		0.41			0.0	0.1	1.1	0.0	0.3	0.0	0.1	0.0	197.7	203.7	201.7	196.9	202.9	200.9	196.7	202.7	200.7
1346	3.0	2.81	2.81		0.61			0.1	0.2	2.4	0.1	0.6	0.0	0.3	0.0	199.2	205.2	203.2	197.3	203.3	201.3	197.0	203.0	201.0
1795	4.0	3.74	3.74		0.81			0.1	0.3	4.3	0.1	1.1	0.0	0.5	0.0	201.3	207.3	205.3	198.0	204.0	202.0	197.4	203.4	201.4
2244	5.0	4.68	4.68		1.02			0.2	0.4	6.8	0.1	1.7	0.0	0.8	0.0	204.0	210.0	208.0	198.8	204.8	202.8	197.9	203.9	201.9
2693	6.0	5.61	5.61		1.22			0.2	0.6	9.8	0.2	2.4	0.1	1.1	0.0	207.3	213.3	211.3	199.8	205.8	203.8	198.4	204.4	202.4
2992	6.7	6.24	6.24		1.36			0.3	0.7	12.1	0.2	3.0	0.1	1.3	0.0	209.8	215.8	213.8	200.6	206.6	204.6	198.9	204.9	202.9
3590	8.0	7.48	7.48		1.63			0.4	1.0	17.4	0.3	4.3	0.1	1.9	0.0	215.6	221.6	219.6	202.4	208.4	206.4	199.9	205.9	203.9
4039	9.0	8.42	8.42		1.83			0.5	1.2	22.0	0.4	5.5	0.1	2.4	0.1	220.7	226.7	224.7	203.9	209.9	207.9	200.8	206.8	204.8
4488	10.0	9.35	9.35		2.04			0.6	1.5	27.2	0.5	6.8	0.1	3.0	0.1	226.4	232.4	230.4	205.6	211.6	209.6	201.7	207.7	205.7
4937	11.0	10.29	10.29		2.24			0.8	1.8	32.9	0.6	8.2	0.2	3.7	0.1	232.6	238.6	236.6	207.5	213.5	211.5	202.8	208.8	206.8
5386	12.0	11.22	11.22		2.44			0.9	2.1	39.1	0.7	9.8	0.2	4.3	0.1	239.4	245.4	243.4	209.5	215.5	213.5	204.0	210.0	208.0
5984	13.3	12.47	12.47		2.72			1.1	2.6	48.3	0.9	12.1	0.2	5.4	0.1	249.4	255.4	253.4	212.6	218.6	216.6	205.7	211.7	209.7
6283	14.0	13.09	13.09		2.85			1.3	2.8	53.2	1.0	13.3	0.3	5.9	0.1	254.8	260.8	258.8	214.2	220.2	218.2	206.6	212.6	210.6
6732	15.0	14.03	14.03		3.06			1.4	3.2	61.1	1.1	15.3	0.3	6.8	0.1	263.4	269.4	267.4	216.8	222.8	220.8	208.1	214.1	212.1
7181	16.0	14.97	14.97		3.26			1.6	3.6	69.6	1.2	17.4	0.3	7.7	0.2	272.6	278.6	276.6	219.5	225.5	223.5	209.7	215.7	213.7
7630	17.0	15.90	15.90		3.46			1.9	4.1	78.5	1.4	19.6	0.4	8.7	0.2	282.3	288.3	286.3	222.4	228.4	226.4	211.3	217.3	215.3
8078	18.0	16.84	16.84		3.67			2.1	4.5	88.0	1.5	22.0	0.4	9.8	0.2	292.7	298.7	296.7	225.5	231.5	229.5	213.1	219.1	217.1
8527	19.0	17.77	17.77		3.87			2.3	5.0	98.1	1.7	24.5	0.5	10.9	0.2	303.6	309.6	307.6	228.8	234.8	232.8	214.9	220.9	218.9
8976	20.0	18.71	18.71		4.07			2.6	5.5	108.7	1.9	27.2	0.5	12.1	0.2	315.1	321.1	319.1	232.2	238.2	236.2	216.9	222.9	220.9
9425	21.0	19.64	19.64		4.28			2.8	6.0	119.8	2.1	30.0	0.6	13.3	0.3	327.2	333.2	331.2	235.9	241.9	239.9	218.9	224.9	222.9
9874	22.0	20.58	20.58		4.48			3.1	6.5	131.5	2.2	32.9	0.6	14.6	0.3	339.9	345.9	343.9	239.6	245.6	243.6	221.0	227.0	225.0
10322	23.0	21.51	21.51		4.68			3.4	7.1	143.7	2.4	35.9	0.7	16.0	0.3	353.2	359.2	357.2	243.6	249.6	247.6	223.3	229.3	227.3
10771	24.0	22.45	22.45		4.89			3.7	7.7	156.5	2.6	39.1	0.7	17.4	0.3	367.0	373.0	371.0	247.7	253.7	251.7	225.6	231.6	229.6
11220	25.0	23.38	23.38		5.09			4.0	8.3	169.8	2.8	42.5	0.8	18.9	0.4	381.4	387.4	385.4	252.0	258.0	256.0	228.0	234.0	232.0
11669	26.0	24.32	24.32		5.30			4.4	8.9	183.7	3.1	45.9	0.8	20.4	0.4	396.5	402.5	400.5	256.5	262.5	260.5	230.6	236.6	234.6
12118	27.0	25.25	25.25		5.50			4.7	9.5	198.1	3.3	49.5	0.9	22.0	0.4	412.1	418.1	416.1	261.2	267.2	265.2	233.2	239.2	237.2
12566	28.0	26.19	26.19		5.70			5.1	10.2	213.0	3.5	53.2	1.0	23.7	0.5	428.3	434.3	432.3	266.0	272.0	270.0	235.9	241.9	239.9
13015	29.0	27.12	27.12		5.91			5.4	10.9	228.5	3.7	57.1	1.0	25.4	0.5	445.0	451.0	449.0	271.0	277.0	275.0	238.7	244.7	242.7
13464	30.0	28.06	28.06		6.11			5.8	11.6	244.5	4.0	61.1	1.1	27.2	0.5	462.4	468.4	466.4	276.1	282.1	280.1	241.6	247.6	245.6



PID Pump Exchange - Appraisal Study - Alternative Alignment 4 - 20 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS ALTERNATIVE: ALTERNATIVE ALIGNMENT 4 - 40 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING		DISCHARGE PIPING				
ELEV	952.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	957.0 feet	Wet Well (High River)	NOM. DIAM. (in)					18	18		36
LWL	953.0 feet	Wet Well (Low River)	O.D. (in)					18	18		36
			I.D. (in)					18	18		36
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1153.0 feet	Canal Bottom	С					110	110		130
HIGH	1155.5 feet	Canal (High Flow)	LENGTH (feet)					8	12		3,240
LWL	1153.5 feet	Canal (Low Flow)	K	1		1		10	10		10

PROPOSED DESIGN POINTS:													
PUMPS	FLOW	FLOW	TDH	POWER									
ON	(GPM)	(CFS)	(FT)	(HP)*									
1	5985	13.3	220	475.6									
2	11970	26.7	224	970.3									
3	17952	40.0	232	1500.8									

Efficiency

Assumes

David Rice, P.E.

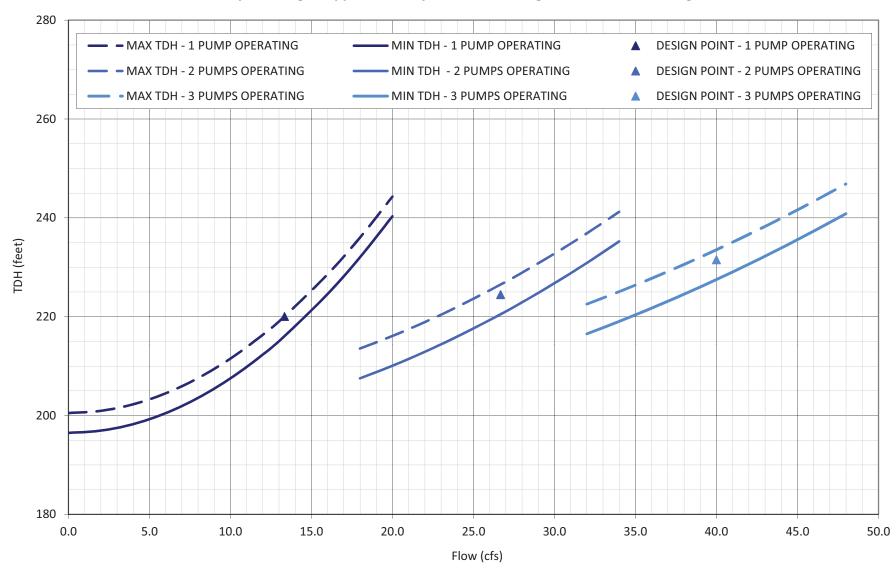
13-Sep-12

BY:

DATE:

TOTAI	TOTAL FLOW VELOCITIES			SUCTION	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	5 - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD) - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DYNAMIC HEAD - 3 PUMPS				
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	196.5	202.5	200.5	196.5	202.5	200.5	196.5	202.5	200.5
898	2.0	1.13	1.13		0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	196.9	202.9	200.9	196.6	202.6	200.6	196.6	202.6	200.6
1795	4.0	2.26	2.26		0.57			0.0	0.1	1.6	0.0	0.4	0.0	0.2	0.0	198.3	204.3	202.3	197.1	203.1	201.1	196.8	202.8	200.8
2693	6.0	3.39	3.39		0.85			0.1	0.2	3.6	0.1	0.9	0.0	0.4	0.0	200.5	206.5	204.5	197.8	203.8	201.8	197.3	203.3	201.3
3590	8.0	4.53	4.53		1.13			0.2	0.4	6.4	0.1	1.6	0.0	0.7	0.0	203.6	209.6	207.6	198.7	204.7	202.7	197.8	203.8	201.8
4488	10.0	5.66	5.66		1.41			0.3	0.6	9.9	0.2	2.5	0.0	1.1	0.0	207.5	213.5	211.5	200.0	206.0	204.0	198.6	204.6	202.6
5386	12.0	6.79	6.79		1.70			0.4	0.9	14.3	0.2	3.6	0.1	1.6	0.0	212.4	218.4	216.4	201.5	207.5	205.5	199.4	205.4	203.4
5985	13.3	7.55	7.55		1.89			0.6	1.1	17.7	0.3	4.4	0.1	2.0	0.0	216.1	222.1	220.1	202.6	208.6	206.6	200.1	206.1	204.1
7181	16.0	9.05	9.05		2.26			0.8	1.5	25.5	0.4	6.4	0.1	2.8	0.0	224.6	230.6	228.6	205.2	211.2	209.2	201.7	207.7	205.7
8078	18.0	10.18	10.18		2.55			1.0	1.9	32.2	0.5	8.1	0.1	3.6	0.1	232.0	238.0	236.0	207.5	213.5	211.5	203.0	209.0	207.0
8976	20.0	11.32	11.32		2.83			1.2	2.3	39.8	0.6	9.9	0.2	4.4	0.1	240.3	246.3	244.3	210.1	216.1	214.1	204.5	210.5	208.5
9874	22.0	12.45	12.45		3.11			1.5	2.7	48.1	0.7	12.0	0.2	5.3	0.1	249.5	255.5	253.5	212.9	218.9	216.9	206.1	212.1	210.1
10771	24.0	13.58	13.58		3.39			1.8	3.2	57.3	0.8	14.3	0.2	6.4	0.1	259.5	265.5	263.5	216.0	222.0	220.0	207.9	213.9	211.9
11970	26.7	15.09	15.09		3.77			2.2	3.8	70.7	0.9	17.7	0.3	7.9	0.1	274.2	280.2	278.2	220.5	226.5	224.5	210.5	216.5	214.5
12566	28.0	15.84	15.84		3.96			2.4	4.2	77.9	1.0	19.5	0.3	8.7	0.1	282.1	288.1	286.1	222.9	228.9	226.9	211.9	217.9	215.9
13464	30.0	16.97	16.97		4.24			2.8	4.8	89.5	1.2	22.4	0.3	9.9	0.2	294.7	300.7	298.7	226.8	232.8	230.8	214.2	220.2	218.2
14362	32.0	18.11	18.11		4.53			3.2	5.4	101.8	1.3	25.5	0.4	11.3	0.2	308.2	314.2	312.2	230.9	236.9	234.9	216.5	222.5	220.5
15259	34.0	19.24	19.24		4.81			3.6	6.0	114.9	1.5	28.7	0.4	12.8	0.2	322.5	328.5	326.5	235.2	241.2	239.2	219.1	225.1	223.1
16157	36.0	20.37	20.37		5.09			4.0	6.7	128.9	1.6	32.2	0.5	14.3	0.2	337.7	343.7	341.7	239.9	245.9	243.9	221.7	227.7	225.7
17054	38.0	21.50	21.50		5.38			4.5	7.4	143.6	1.8	35.9	0.5	16.0	0.2	353.7	359.7	357.7	244.8	250.8	248.8	224.6	230.6	228.6
17952	40.0	22.63	22.63		5.66			5.0	8.1	159.1	2.0	39.8	0.6	17.7	0.3	370.7	376.7	374.7	249.9	255.9	253.9	227.5	233.5	231.5
18850	42.0	23.76	23.76		5.94			5.5	8.9	175.4	2.2	43.8	0.6	19.5	0.3	388.4	394.4	392.4	255.3	261.3	259.3	230.6	236.6	234.6
19747	44.0	24.90	24.90		6.22			6.0	9.7	192.5	2.4	48.1	0.7	21.4	0.3	407.1	413.1	411.1	261.0	267.0	265.0	233.9	239.9	237.9
20645	46.0	26.03	26.03		6.51			6.6	10.5	210.4	2.6	52.6	0.7	23.4	0.3	426.5	432.5	430.5	266.9	272.9	270.9	237.3	243.3	241.3
21542	48.0	27.16	27.16		6.79			7.2	11.4	229.1	2.8	57.3	0.8	25.5	0.4	446.9	452.9	450.9	273.1	279.1	277.1	240.8	246.8	244.8
22440	50.0	28.29	28.29		7.07			7.8	12.3	248.6	3.0	62.1	0.8	27.6	0.4	468.1	474.1	472.1	279.5	285.5	283.5	244.5	250.5	248.5
23338	52.0	29.42	29.42		7.36			8.4	13.2	268.8	3.2	67.2	0.9	29.9	0.4	490.2	496.2	494.2	286.2	292.2	290.2	248.4	254.4	252.4
24235	54.0	30.55	30.55		7.64			9.1	14.1	289.9	3.5	72.5	1.0	32.2	0.5	513.1	519.1	517.1	293.1	299.1	297.1	252.4	258.4	256.4
25133	56.0	31.69	31.69		7.92			9.7	15.1	311.8	3.7	77.9	1.0	34.6	0.5	536.9	542.9	540.9	300.3	306.3	304.3	256.5	262.5	260.5
26030	58.0	32.82	32.82		8.20			10.5	16.1	334.5	4.0	83.6	1.1	37.2	0.5	561.5	567.5	565.5	307.8	313.8	311.8	260.8	266.8	264.8
26928	60.0	33.95	33.95		8.49			11.2	17.2	357.9	4.2	89.5	1.2	39.8	0.6	587.0	593.0	591.0	315.5	321.5	319.5	265.2	271.2	269.2

PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY



PID Pump Exchange - Appraisal Study - Alternative Alignment 4 - 40 CFS Design Flow

SYSTEM CURVE CALCULATION

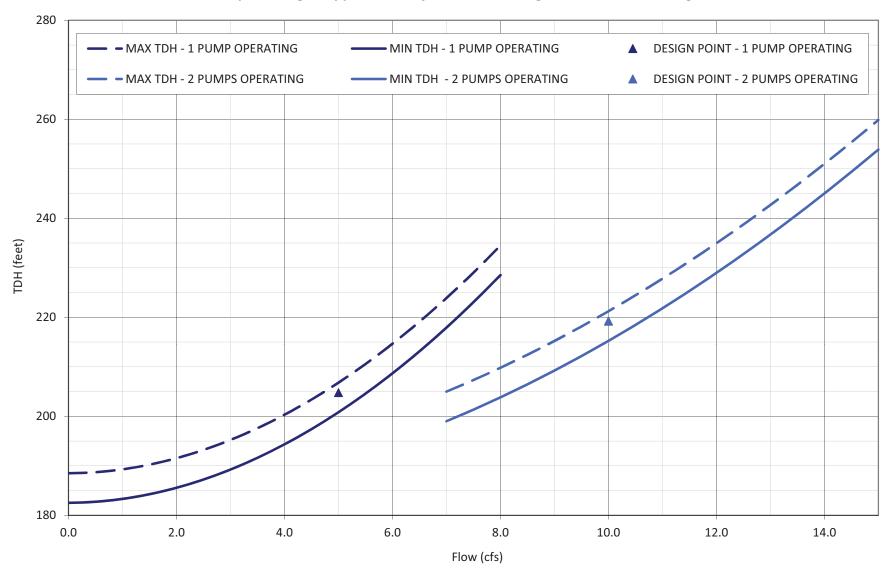
PROJECT: PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 5 - 10 CFS DESIGN FLOW

BY:	David Rice, P.E.
DATE:	13-Sep-12

UCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING		PS P	IPING		DISCHAR	GE PIPINO
ELEV	971.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS
HWL	976.0 feet	Wet Well (High River)	NOM. DIAM. (in)					12	12		20
LWL	972.0 feet	Wet Well (Low River)	O.D. (in)					12	12		20
			I.D. (in)					12	12		20
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1158.0 feet	Canal Bottom	C					110	110		130
HIGH	1160.5 feet	Canal (High Flow)	LENGTH (feet)					8	12		4,910
LWL	1158.5 feet	Canal (Low Flow)	K					10	10		10

ESIGN POIN	TS:		
FLOW	FLOW	TDH	POWER
(GPM)	(CFS)	(FT)	(HP)*
2244	5.0	205	165.9
4488	10.0	219	355.3
70%	Efficiency		
	FLOW (GPM) 2244 4488	(GPM) (CFS) 2244 5.0 4488 10.0	FLOW (GPM) FLOW (CFS) TDH (FT) 2244 5.0 205 4488 10.0 219

TOTA	FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	5 - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0			182.5	188.5	186.5	182.5	188.5	186.5			
224	0.5	0.64	0.64		0.23			0.0	0.1	0.1	0.0	0.0	0.0			182.7	188.7	186.7	182.6	188.6	186.6			
449	1.0	1.27	1.27		0.46			0.0	0.2	0.5	0.0	0.1	0.0			183.3	189.3	187.3	182.9	188.9	186.9			
673	1.5	1.91	1.91		0.69			0.1	0.5	1.1	0.0	0.3	0.0			184.2	190.2	188.2	183.4	189.4	187.4			
898	2.0	2.55	2.55		0.92			0.1	0.8	2.0	0.1	0.5	0.0			185.5	191.5	189.5	184.0	190.0	188.0			
1122	2.5	3.18	3.18		1.15			0.2	1.3	3.1	0.1	0.8	0.0			187.2	193.2	191.2	184.8	190.8	188.8			
1346	3.0	3.82	3.82		1.37			0.3	1.8	4.5	0.1	1.1	0.0			189.2	195.2	193.2	185.7	191.7	189.7			
1571	3.5	4.46	4.46		1.60			0.4	2.4	6.2	0.2	1.5	0.0			191.6	197.6	195.6	186.9	192.9	190.9			
1795	4.0	5.09	5.09		1.83			0.5	3.0	8.1	0.2	2.0	0.1			194.3	200.3	198.3	188.1	194.1	192.1			
2020	4.5	5.73	5.73		2.06			0.7	3.8	10.2	0.3	2.5	0.1			197.4	203.4	201.4	189.6	195.6	193.6			
2244	5.0	6.37	6.37		2.29			0.8	4.6	12.6	0.3	3.1	0.1			200.8	206.8	204.8	191.1	197.1	195.1			
2468	5.5	7.00	7.00		2.52			1.0	5.5	15.2	0.4	3.8	0.1			204.6	210.6	208.6	192.9	198.9	196.9			
2693	6.0	7.64	7.64		2.75			1.2	6.4	18.1	0.4	4.5	0.1			208.7	214.7	212.7	194.8	200.8	198.8			
2917	6.5	8.27	8.27		2.98			1.4	7.5	21.3	0.5	5.3	0.1			213.1	219.1	217.1	196.8	202.8	200.8			
3142	7.0	8.91	8.91		3.21			1.6	8.6	24.7	0.6	6.2	0.2			217.9	223.9	221.9	199.0	205.0	203.0			
3366	7.5	9.55	9.55		3.44			1.8	9.7	28.3	0.6	7.1	0.2			223.0	229.0	227.0	201.3	207.3	205.3			
3590	8.0	10.18	10.18		3.67			2.1	11.0	32.2	0.7	8.1	0.2			228.5	234.5	232.5	203.8	209.8	207.8			
3815	8.5	10.82	10.82		3.90			2.4	12.3	36.4	0.8	9.1	0.2			234.3	240.3	238.3	206.4	212.4	210.4			
4039	9.0	11.46	11.46		4.12			2.6	13.6	40.8	0.9	10.2	0.3			240.5	246.5	244.5	209.2	215.2	213.2			
4264	9.5	12.09	12.09		4.35			2.9	15.1	45.4	1.0	11.4	0.3			246.9	252.9	250.9	212.1	218.1	216.1			
4488	10.0	12.73	12.73		4.58			3.3	16.6	50.3	1.1	12.6	0.3			253.8	259.8	257.8	215.2	221.2	219.2			
4712	10.5	13.37	13.37		4.81			3.6	18.1	55.5	1.2	13.9	0.3			260.9	266.9	264.9	218.4	224.4	222.4			
4937	11.0	14.00	14.00		5.04			3.9	19.8	60.9	1.3	15.2	0.4			268.4	274.4	272.4	221.8	227.8	225.8			
5161	11.5	14.64	14.64		5.27			4.3	21.5	66.6	1.4	16.6	0.4			276.3	282.3	280.3	225.3	231.3	229.3			
5386	12.0	15.28	15.28		5.50			4.7	23.2	72.5	1.5	18.1	0.4			284.4	290.4	288.4	229.0	235.0	233.0			
5610	12.5	15.91	15.91		5.73			5.1	25.0	78.6	1.7	19.7	0.5			292.9	298.9	296.9	232.8	238.8	236.8			
5834	13.0	16.55	16.55		5.96			5.5	26.9	85.1	1.8	21.3	0.5			301.8	307.8	305.8	236.7	242.7	240.7			
6059	13.5	17.19	17.19		6.19			5.9	28.9	91.7	1.9	22.9	0.5			311.0	317.0	315.0	240.8	246.8	244.8			
6283	14.0	17.82	17.82		6.42			6.4	30.9	98.7	2.1	24.7	0.6			320.5	326.5	324.5	245.0	251.0	249.0			
6508	14.5	18.46	18.46		6.65			6.9	32.9	105.8	2.2	26.5	0.6			330.3	336.3	334.3	249.4	255.4	253.4			
6732	15.0	19.10	19.10		6.87			7.3	35.1	113.2	2.3	28.3	0.6			340.5	346.5	344.5	253.9	259.9	257.9			



PID Pump Exchange - Appraisal Study - Alternative Alignment 5 - 10 CFS Design Flow

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 5 - 20 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTIO	N PIPING		PS P	IPING		DISCHAR	GE PIPING
ELEV	971.0 feet	Invert at Diversion	7	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	976.0 feet	Wet Well (High River)		NOM. DIAM. (in)					14	14		30
LWL	972.0 feet	Wet Well (Low River)		O.D. (in)					14	14		30
				I.D. (in)					14	14		30
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE
ELEV	1158.0 feet	Canal Bottom		С					110	110		130
HIGH	1160.5 feet	Canal (High Flow)	7	LENGTH (feet)					8	12		4,910
LWL	1158.5 feet	Canal (Low Flow)	7	К			1		10	10		10

ROPOSED	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2992	6.7	200	216.3
2	5985	13.3	204	440.6
3	8976	20.0	210	679.6
Assumes	70%	Efficiency		

BY:

DATE:

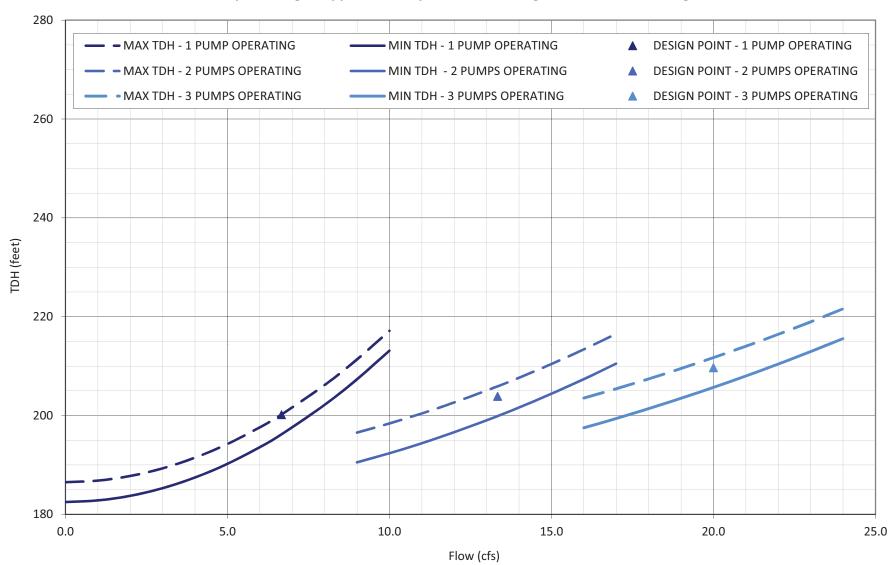
David Rice, P.E.

13-Sep-12

TOTAL	FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSE	S - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	D - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	0 - 3 PUMPS
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00	1		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	182.5	188.5	186.5	182.5	188.5	186.5	182.5	188.5	186.5
449	1.0	0.94	0.94		0.20			0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	182.8	188.8	186.8	182.6	188.6	186.6	182.6	188.6	186.6
898	2.0	1.87	1.87		0.41			0.0	0.1	1.1	0.0	0.3	0.0	0.1	0.0	183.8	189.8	187.8	182.9	188.9	186.9	182.8	188.8	186.8
1346	3.0	2.81	2.81		0.61			0.1	0.2	2.4	0.1	0.6	0.0	0.3	0.0	185.3	191.3	189.3	183.4	189.4	187.4	183.1	189.1	187.1
1795	4.0	3.74	3.74		0.81			0.1	0.4	4.3	0.1	1.1	0.0	0.5	0.0	187.5	193.5	191.5	184.1	190.1	188.1	183.5	189.5	187.5
2244	5.0	4.68	4.68		1.02			0.2	0.6	6.8	0.1	1.7	0.0	0.8	0.0	190.2	196.2	194.2	185.0	191.0	189.0	184.1	190.1	188.1
2693	6.0	5.61	5.61		1.22			0.2	0.9	9.8	0.2	2.4	0.1	1.1	0.0	193.6	199.6	197.6	186.1	192.1	190.1	184.7	190.7	188.7
2992	6.7	6.24	6.24		1.36			0.3	1.1	12.1	0.2	3.0	0.1	1.3	0.0	196.2	202.2	200.2	187.0	193.0	191.0	185.2	191.2	189.2
3590	8.0	7.48	7.48		1.63			0.4	1.5	17.4	0.3	4.3	0.1	1.9	0.0	202.2	208.2	206.2	188.9	194.9	192.9	186.4	192.4	190.4
4039	9.0	8.42	8.42		1.83			0.5	1.9	22.0	0.4	5.5	0.1	2.4	0.1	207.4	213.4	211.4	190.5	196.5	194.5	187.4	193.4	191.4
4488	10.0	9.35	9.35		2.04			0.6	2.3	27.2	0.5	6.8	0.1	3.0	0.1	213.1	219.1	217.1	192.4	198.4	196.4	188.5	194.5	192.5
4937	11.0	10.29	10.29		2.24			0.8	2.7	32.9	0.6	8.2	0.2	3.7	0.1	219.5	225.5	223.5	194.4	200.4	198.4	189.8	195.8	193.8
5386	12.0	11.22	11.22		2.44			0.9	3.2	39.1	0.7	9.8	0.2	4.3	0.1	226.5	232.5	230.5	196.6	202.6	200.6	191.1	197.1	195.1
5985	13.3	12.47	12.47		2.72			1.1	3.9	48.3	0.9	12.1	0.2	5.4	0.1	236.8	242.8	240.8	199.9	205.9	203.9	193.0	199.0	197.0
6283	14.0	13.09	13.09		2.85			1.3	4.3	53.2	1.0	13.3	0.3	5.9	0.1	242.3	248.3	246.3	201.6	207.6	205.6	194.1	200.1	198.1
6732	15.0	14.03	14.03		3.06			1.4	4.9	61.1	1.1	15.3	0.3	6.8	0.1	251.1	257.1	255.1	204.4	210.4	208.4	195.8	201.8	199.8
7181	16.0	14.97	14.97		3.26			1.6	5.5	69.6	1.2	17.4	0.3	7.7	0.2	260.4	266.4	264.4	207.4	213.4	211.4	197.5	203.5	201.5
7630	17.0	15.90	15.90		3.46			1.9	6.1	78.5	1.4	19.6	0.4	8.7	0.2	270.4	276.4	274.4	210.5	216.5	214.5	199.4	205.4	203.4
8078	18.0	16.84	16.84		3.67			2.1	6.8	88.0	1.5	22.0	0.4	9.8	0.2	281.0	287.0	285.0	213.8	219.8	217.8	201.4	207.4	205.4
8527	19.0	17.77	17.77		3.87			2.3	7.5	98.1	1.7	24.5	0.5	10.9	0.2	292.2	298.2	296.2	217.4	223.4	221.4	203.5	209.5	207.5
8976	20.0	18.71	18.71		4.07			2.6	8.3	108.7	1.9	27.2	0.5	12.1	0.2	303.9	309.9	307.9	221.1	227.1	225.1	205.7	211.7	209.7
9425	21.0	19.64	19.64		4.28			2.8	9.1	119.8	2.1	30.0	0.6	13.3	0.3	316.3	322.3	320.3	224.9	230.9	228.9	208.0	214.0	212.0
9874	22.0	20.58	20.58		4.48			3.1	9.9	131.5	2.2	32.9	0.6	14.6	0.3	329.2	335.2	333.2	229.0	235.0	233.0	210.4	216.4	214.4
10322	23.0	21.51	21.51		4.68			3.4	10.7	143.7	2.4	35.9	0.7	16.0	0.3	342.8	348.8	346.8	233.3	239.3	237.3	212.9	218.9	216.9
10771	24.0	22.45	22.45		4.89			3.7	11.6	156.5	2.6	39.1	0.7	17.4	0.3	357.0	363.0	361.0	237.7	243.7	241.7	215.6	221.6	219.6
11220	25.0	23.38	23.38		5.09			4.0	12.5	169.8	2.8	42.5	0.8	18.9	0.4	371.7	377.7	375.7	242.3	248.3	246.3	218.3	224.3	222.3
11669	26.0	24.32	24.32		5.30			4.4	13.5	183.7	3.1	45.9	0.8	20.4	0.4	387.0	393.0	391.0	247.1	253.1	251.1	221.1	227.1	225.1
12118	27.0	25.25	25.25		5.50			4.7	14.4	198.1	3.3	49.5	0.9	22.0	0.4	403.0	409.0	407.0	252.1	258.1	256.1	224.1	230.1	228.1
12566	28.0	26.19	26.19		5.70			5.1	15.5	213.0	3.5	53.2	1.0	23.7	0.5	419.5	425.5	423.5	257.2	263.2	261.2	227.1	233.1	231.1
13015	29.0	27.12	27.12		5.91			5.4	16.5	228.5	3.7	57.1	1.0	25.4	0.5	436.6	442.6	440.6	262.6	268.6	266.6	230.3	236.3	234.3
13464	30.0	28.06	28.06		6.11			5.8	17.6	244.5	4.0	61.1	1.1	27.2	0.5	454.4	460.4	458.4	268.1	274.1	272.1	233.5	239.5	237.5

PID Pump Exchange - Hydraulic Analysis - Alignment 5 - TO PID.xlsx

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PID Pump Exchange - Appraisal Study - Alternative Alignment 5 - 20 CFS Design Flow

SYSTEM CURVE CALCULATION

PROJECT: PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY ALTERNATIVE: ALTERNATIVE ALIGNMENT 5 - 40 CFS DESIGN FLOW

SUCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING		PS P	IPING		DISCHAR	GE PIPING
ELEV	971.0 feet	Invert at Diversion	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	976.0 feet	Wet Well (High River)	NOM. DIAM. (in)					18	18		36
LWL	972.0 feet	Wet Well (Low River)	O.D. (in)					18	18		36
			I.D. (in)					18	18		36
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1158.0 feet	Canal Bottom	С					110	110		130
HIGH	1160.5 feet	Canal (High Flow)	LENGTH (feet)					8	12		4,910
LWL	1158.5 feet	Canal (Low Flow)	K	1				10	10		10

ROPOSED	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	5985	13.3	207	446.5
2	11970	26.7	212	918.3
3	17952	40.0	222	1437.2
Assumes	70%	Efficiency		

BY:

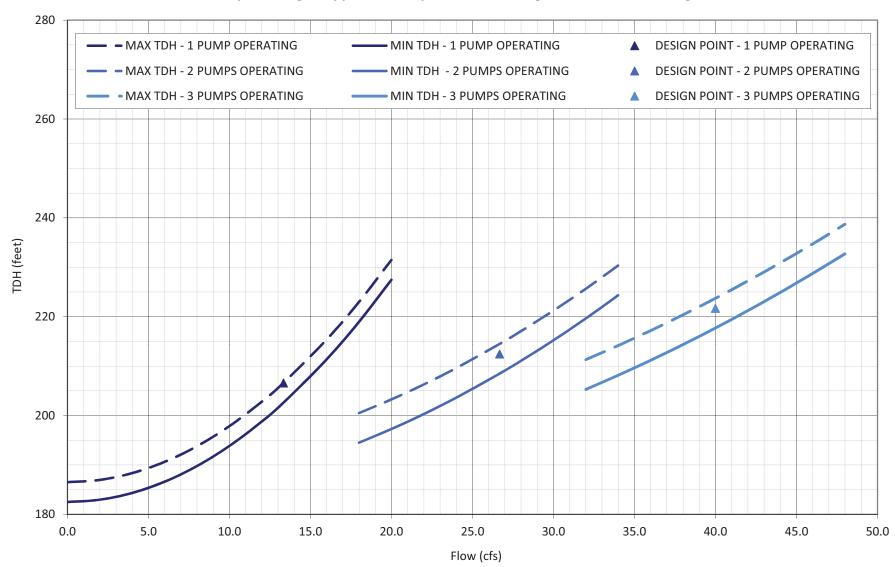
DATE:

David Rice, P.E.

13-Sep-12

TOTAL	FLOW		VELO	CITIES		SUCTION	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	S - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	D - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00	1		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	182.5	188.5	186.5	182.5	188.5	186.5	182.5	188.5	186.5
898	2.0	1.13	1.13		0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	183.0	189.0	187.0	182.7	188.7	186.7	182.6	188.6	186.6
1795	4.0	2.26	2.26		0.57			0.0	0.2	1.6	0.0	0.4	0.0	0.2	0.0	184.3	190.3	188.3	183.1	189.1	187.1	182.9	188.9	186.9
2693	6.0	3.39	3.39		0.85			0.1	0.4	3.6	0.1	0.9	0.0	0.4	0.0	186.6	192.6	190.6	183.9	189.9	187.9	183.4	189.4	187.4
3590	8.0	4.53	4.53		1.13	1		0.2	0.6	6.4	0.1	1.6	0.0	0.7	0.0	189.8	195.8	193.8	184.9	190.9	188.9	184.0	190.0	188.0
4488	10.0	5.66	5.66		1.41	1		0.3	0.9	9.9	0.2	2.5	0.0	1.1	0.0	193.9	199.9	197.9	186.3	192.3	190.3	184.9	190.9	188.9
5386	12.0	6.79	6.79		1.70	1		0.4	1.3	14.3	0.2	3.6	0.1	1.6	0.0	198.8	204.8	202.8	187.9	193.9	191.9	185.9	191.9	189.9
5985	13.3	7.55	7.55		1.89	1		0.6	1.6	17.7	0.3	4.4	0.1	2.0	0.0	202.6	208.6	206.6	189.2	195.2	193.2	186.7	192.7	190.7
7181	16.0	9.05	9.05		2.26	1		0.8	2.3	25.5	0.4	6.4	0.1	2.8	0.0	211.4	217.4	215.4	192.0	198.0	196.0	188.4	194.4	192.4
8078	18.0	10.18	10.18		2.55	1		1.0	2.8	32.2	0.5	8.1	0.1	3.6	0.1	219.0	225.0	223.0	194.5	200.5	198.5	190.0	196.0	194.0
8976	20.0	11.32	11.32		2.83	1		1.2	3.4	39.8	0.6	9.9	0.2	4.4	0.1	227.5	233.5	231.5	197.3	203.3	201.3	191.6	197.6	195.6
9874	22.0	12.45	12.45		3.11	1		1.5	4.1	48.1	0.7	12.0	0.2	5.3	0.1	236.9	242.9	240.9	200.3	206.3	204.3	193.5	199.5	197.5
10771	24.0	13.58	13.58		3.39			1.8	4.8	57.3	0.8	14.3	0.2	6.4	0.1	247.1	253.1	251.1	203.6	209.6	207.6	195.5	201.5	199.5
11970	26.7	15.09	15.09		3.77			2.2	5.8	70.7	0.9	17.7	0.3	7.9	0.1	262.2	268.2	266.2	208.5	214.5	212.5	198.5	204.5	202.5
12566	28.0	15.84	15.84		3.96	1		2.4	6.4	77.9	1.0	19.5	0.3	8.7	0.1	270.3	276.3	274.3	211.1	217.1	215.1	200.1	206.1	204.1
13464	30.0	16.97	16.97		4.24			2.8	7.2	89.5	1.2	22.4	0.3	9.9	0.2	283.2	289.2	287.2	215.2	221.2	219.2	202.6	208.6	206.6
14362	32.0	18.11	18.11		4.53			3.2	8.1	101.8	1.3	25.5	0.4	11.3	0.2	297.0	303.0	301.0	219.6	225.6	223.6	205.3	211.3	209.3
15259	34.0	19.24	19.24		4.81			3.6	9.1	114.9	1.5	28.7	0.4	12.8	0.2	311.6	317.6	315.6	224.3	230.3	228.3	208.2	214.2	212.2
16157	36.0	20.37	20.37		5.09			4.0	10.1	128.9	1.6	32.2	0.5	14.3	0.2	327.1	333.1	331.1	229.3	235.3	233.3	211.2	217.2	215.2
17054	38.0	21.50	21.50		5.38			4.5	11.2	143.6	1.8	35.9	0.5	16.0	0.2	343.6	349.6	347.6	234.6	240.6	238.6	214.4	220.4	218.4
17952	40.0	22.63	22.63		5.66			5.0	12.3	159.1	2.0	39.8	0.6	17.7	0.3	360.8	366.8	364.8	240.1	246.1	244.1	217.7	223.7	221.7
18850	42.0	23.76	23.76		5.94			5.5	13.5	175.4	2.2	43.8	0.6	19.5	0.3	379.0	385.0	383.0	245.9	251.9	249.9	221.2	227.2	225.2
19747	44.0	24.90	24.90		6.22			6.0	14.7	192.5	2.4	48.1	0.7	21.4	0.3	398.0	404.0	402.0	252.0	258.0	256.0	224.9	230.9	228.9
20645	46.0	26.03	26.03		6.51			6.6	15.9	210.4	2.6	52.6	0.7	23.4	0.3	418.0	424.0	422.0	258.3	264.3	262.3	228.7	234.7	232.7
21542	48.0	27.16	27.16		6.79			7.2	17.2	229.1	2.8	57.3	0.8	25.5	0.4	438.8	444.8	442.8	264.9	270.9	268.9	232.7	238.7	236.7
22440	50.0	28.29	28.29		7.07			7.8	18.6	248.6	3.0	62.1	0.8	27.6	0.4	460.4	466.4	464.4	271.8	277.8	275.8	236.9	242.9	240.9
23338	52.0	29.42	29.42		7.36			8.4	20.0	268.8	3.2	67.2	0.9	29.9	0.4	483.0	489.0	487.0	279.0	285.0	283.0	241.2	247.2	245.2
24235	54.0	30.55	30.55		7.64			9.1	21.4	289.9	3.5	72.5	1.0	32.2	0.5	506.4	512.4	510.4	286.4	292.4	290.4	245.7	251.7	249.7
25133	56.0	31.69	31.69		7.92			9.7	22.9	311.8	3.7	77.9	1.0	34.6	0.5	530.7	536.7	534.7	294.1	300.1	298.1	250.3	256.3	254.3
26030	58.0	32.82	32.82		8.20			10.5	24.5	334.5	4.0	83.6	1.1	37.2	0.5	555.8	561.8	559.8	302.1	308.1	306.1	255.1	261.1	259.1
26928	60.0	33.95	33.95		8.49			11.2	26.0	357.9	4.2	89.5	1.2	39.8	0.6	581.9	587.9	585.9	310.4	316.4	314.4	260.0	266.0	264.0

Anchor QEA, LLC



PID Pump Exchange - Appraisal Study - Alternative Alignment 5 - 40 CFS Design Flow

APPENDIX D FISH SCREENING CRITERIA AND CALCULATIONS

PRELIMINARY FISH SCREEN EVALUATION

PROJECT: PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

APPLICABLE CRITERIA:

National Marine Fisheries Science Anadromous Salmonid Passage Facility Design Chapeter 11 - Fish Screen Bypass Facilities

Washington State Department of Fish and Wildlife Fish Protection Screen Guidelines

Location and Orientation:

-Where possible, screen should be constructed at point of diversion with screen face parallel to river flow. -For screens constructed at the bankline, the screen face must be aligned with the adjacent bankline. -Screen facilities must be designed to function properly within the full range of stream hydraulic conditions.

Approach Velocity:

V _{approach} =	0.4 fps max. (active screens)
	0.2 fps max. (passive screens)

Effective Screen Area:

 $A_{screen min.} = Q_{max}/V_{approach}$

Sweeping Velocity:

V_{sweep} =

0.8 ft/s min. (or > V_{approach} if screen is 6' or longer) 3 ft/s max.

Screen Material:

A _{open min.} =	0.087 inches (wire mesh, 14 gauge max.)
A _{open min.} =	1.75 mm (profile bar, slotted)
A _{open min.} =	0.097 inches (circular openings, perforated plate)
A _{open min.} =	0.097 inches on diagonal (square openings)
A _{open} =	27% X A _{screen}

Submergence:

D_{screen} = 1 radius (for end of pipe screen, below water surface)

Clearance:

L_{clear} = 1 radius (for end of pipe screen, around screen)

SCREEN SIZING:

Assumptions:

Screen Type = Fixed Plate, Active (Self-Cleaning), at Angle of 45° F.O.S. = 27%

Q _{max} (CFS)	V _{approach} (FPS)	A _{screen min.} (SF)	A _{screen with F.O.S.} (SF)	H _{screen proposed} (FT)	L _{screen min.} (FT)
5	0.4	12.50	15.88	4.0	4.0
10	0.4	25.00	31.75	4.0	7.9
15	0.4	37.50	47.63	4.0	11.9
20	0.4	50.00	63.50	4.0	15.9
25	0.4	62.50	79.38	4.0	19.8
30	0.4	75.00	95.25	4.0	23.8
35	0.4	87.50	111.13	4.0	27.8
40	0.4	100.00	127.00	4.0	31.8

Assumptions:

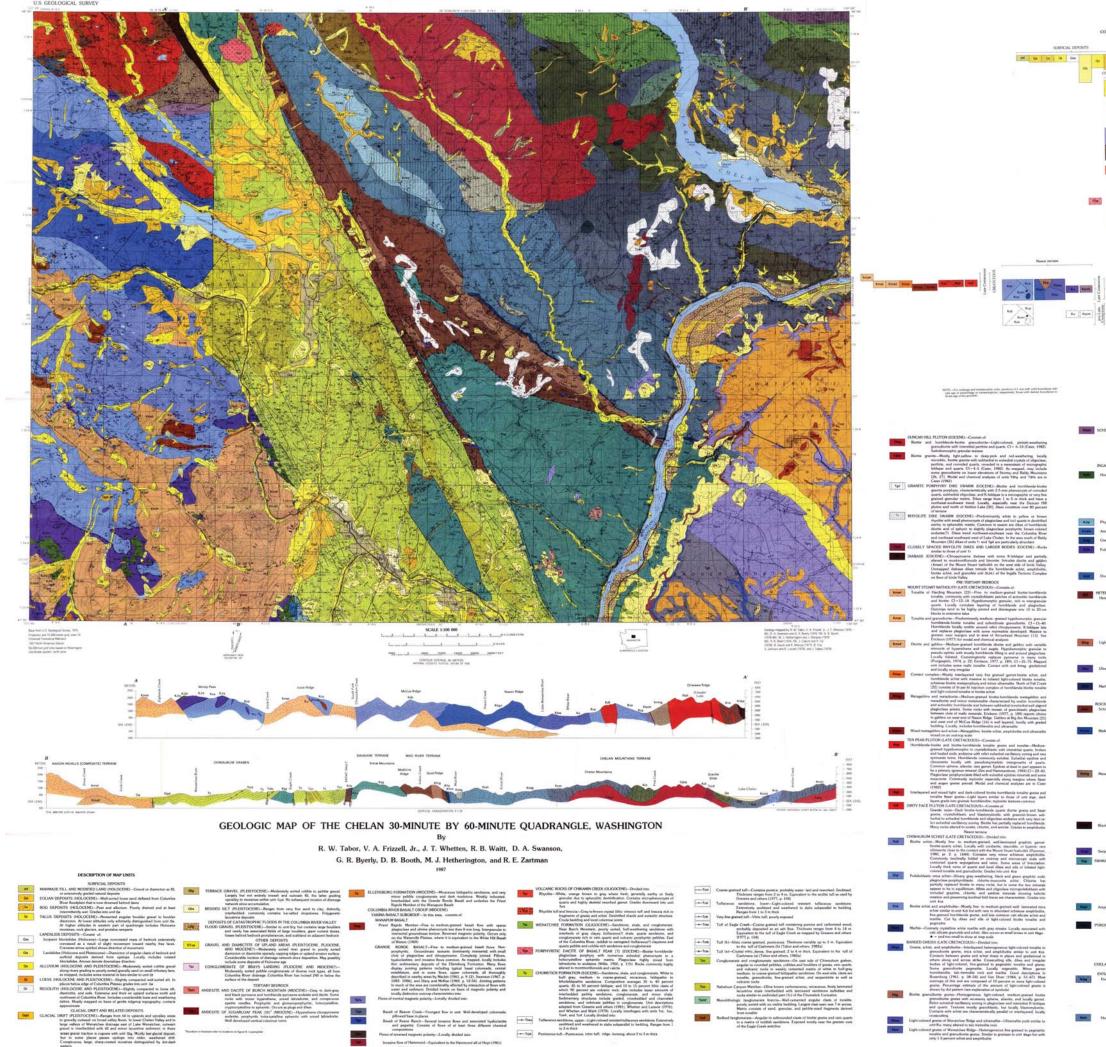
Screen Type = Cylinder Screen, End of Pipe, Passive 27%

F.O.S. =

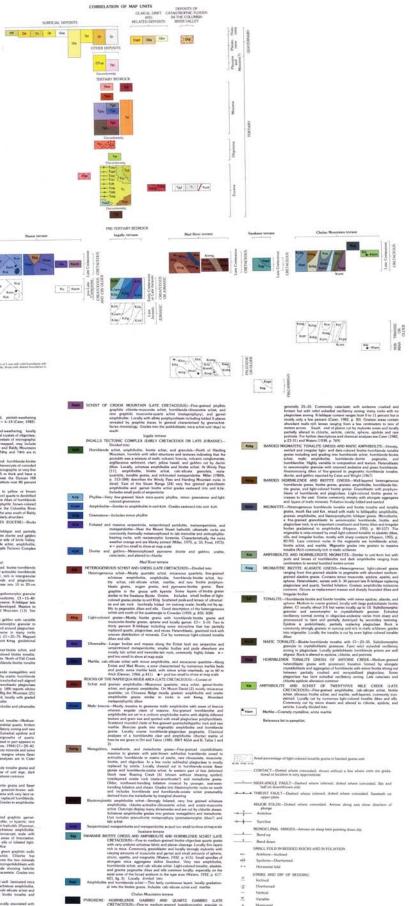
Q _{max} (CFS)	V _{approach} (FPS)	A _{screen min.} (SF)	A _{screen with F.O.S.} (SF)	D _{screen proposed} (IN)	L _{screen min.} (FT)
5	0.2	25.00	31.75	12.0	10.1
10	0.2	50.00	63.50	12.0	20.2
15	0.2	75.00	95.25	12.0	30.3
20	0.2	100.00	127.00	18.0	27.0
25	0.2	125.00	158.75	18.0	33.7
30	0.2	150.00	190.50	24.0	30.3
35	0.2	175.00	222.25	24.0	35.4
40	0.2	200.00	254.00	24.0	40.4

APPENDIX E GEOLOGIC REVIEW DATA





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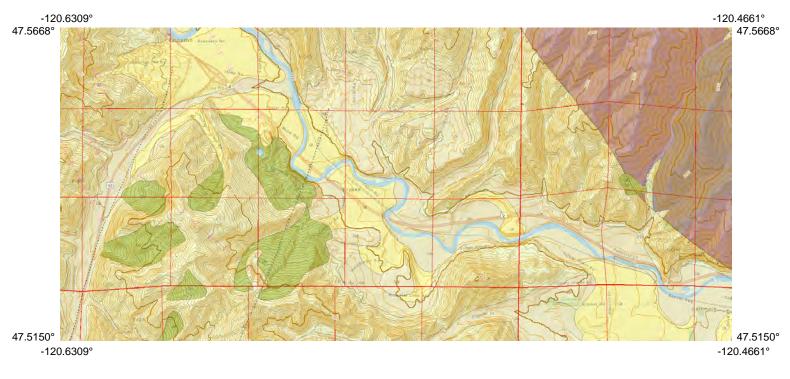
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[1] Sana J. (Schlassification, Nac. Sciences, Inv. Nat. Astrophys. J. Science, 7(1):44211

Peshastin Irrigation District Geologic Map



Map Scale 1:74045





Division of Geology and Earth Resources http://www.dnr.wa.gov/ResearchScience/GeologyEarthSciences/

Map Legend

Base Layers

Contours, 40 foot

Contours 40 foot

Public Land Survey (Sections)

Public Land Survey (Sections)

Public Land Survey (Township/Range)



Public Land Survey (Township/Range)

30x60-minute Quadrangle Boundaries



30x60-minute Quadrangle Boundaries

Landslides

Landslides, 1:100,000 Scale



Landslides, 1:100,000 Scale

Landslides, 1:24,000 Scale



Surface Geology Geologic Unit Points 100K

Geologic Unit Points 100K

Dikes 100K



- Dike Identity and existence certain, location accurate Dike - Identity or existence certain,
- location concealed

Contacts 100K



Contact - Identity and existence certain, location accurate

Shoreline

Faults 100K

	Fault, unknown offset - Identity and existence certain, location concealed
. 	Right-lateral strike-slip fault - Identity and existence certain, location concealed. Arrows show relative motion
	Normal fault - Identity and existence certain, location accurate. Bar and ball on downthrown block
•••]••••	Normal fault - Identity and existence certain, location concealed. Bar and ball on downthrown block
- <u>=</u>	Right-lateral strike-slip fault - Identity and existence certain, location inferred. Arrows show relative motion

Folds 100K

	Anticline - Identity and existence certain, location accurate
•	Anticline - Identity and existence certain, location concealed
+	Syncline - Identity and existence certain, location accurate
· ‡· · · ·	Syncline - Identity and existence certain, location concealed

Linear Geologic Features 100K

 Linear geologic feature, other than dike - Identity and existence certain, location accurate
 Linear geologic feature, other than dike - Identity and existence certain, location concealed
 Continental ice limit, late Wisconsinan
 Continental ice limit, pre-late Wisconsinan

Geologic Units 100K

Unconsolidated Sediments



Quaternary alluvium, dune sand, loess, and artificial fill Quaternary alluvial fans, beach deposits, undifferentiated sedimentary deposits, lacustrine deposits, landslides, peat, terraced deposits, and talus Pleistocene continental glacial, glaciolacustrine, and outburst flood deposits, Fraser-age

Sedimentary Rocks and Deposits



Quaternary–Miocene continental sedimentary rocks

Eocene continental sedimentary rocks

Metamorphic Rocks (Amphibolite Facies and Higher)

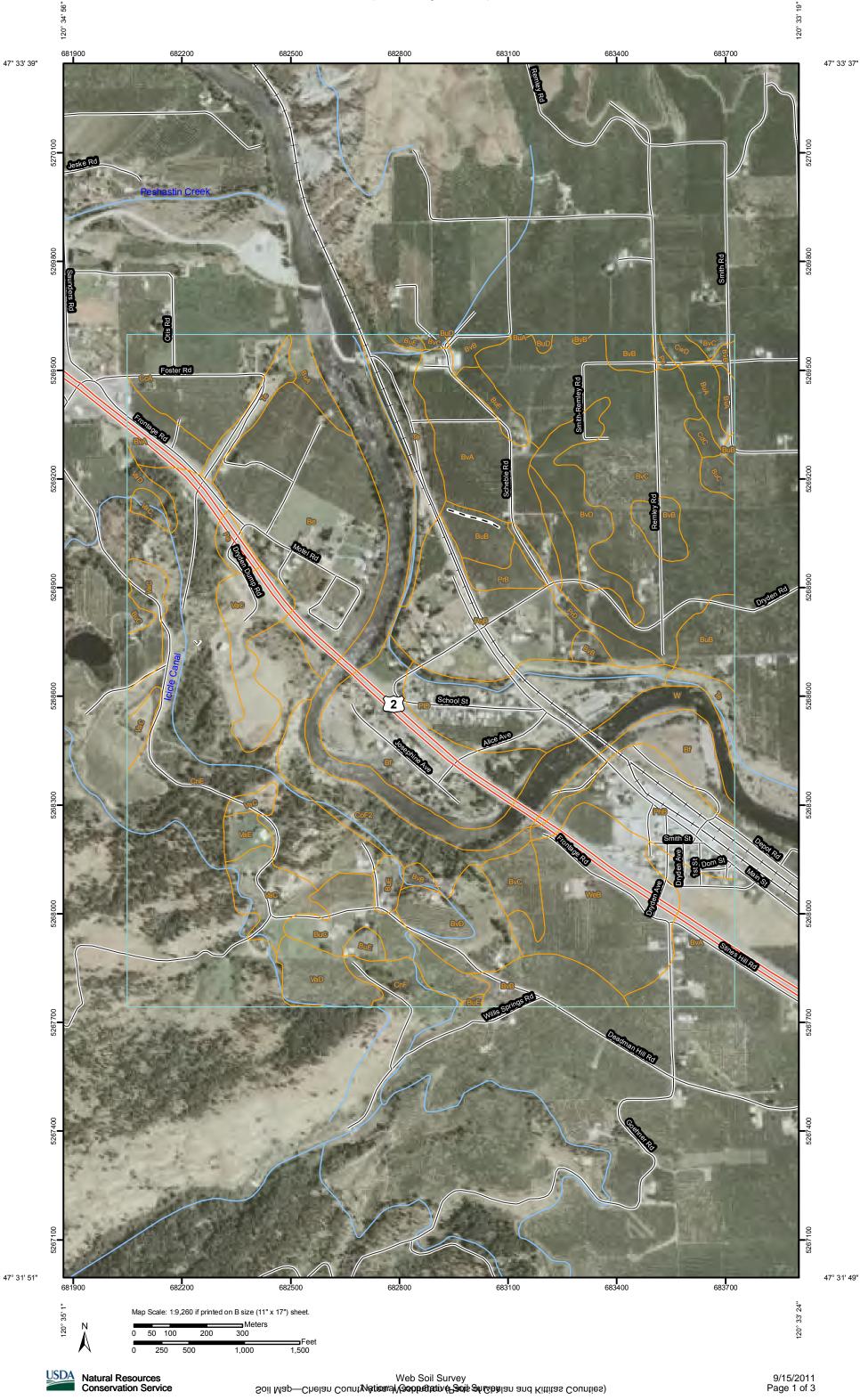


Precambrian metamorphic rocks

Other Geologic Units or Features







120° 33' 19"

Area of Interest (AOI) Image: Constraint of Constraints Very Stony Spot Image: Area of Interest (AOI) Image: Constraint of Constraints Wet Spot Soils Image: Constraint of Constraints Other Special Point Features Soilly
Soils Other Special Line Features
Soil Map Units Special Line Features
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Map Unit Legend

Ch	elan County Area, Washington (Parts of Chelan a	nd Kittitas Counties) (W	/A607)
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Ве	Beverly fine sandy loam	55.5	7.2%
Bf	Beverly gravelly fine sandy loam	46.4	6.0%
BuA	Burch fine sandy loam, 0 to 3 percent slopes	7.9	1.0%
BuB	Burch fine sandy loam, 3 to 8 percent slopes	13.8	1.8%
BuC	Burch fine sandy loam, 8 to 15 percent slopes	18.6	2.4%
BuD	Burch fine sandy loam, 15 to 25 percent slopes	0.6	0.1%
BuE	Burch fine sandy loam, 25 to 45 percent slopes	9.3	1.2%
BvA	Burch loam, 0 to 3 percent slopes	52.9	6.9%
BvB	Burch loam, 3 to 8 percent slopes	25.4	3.3%
BvC	Burch loam, 8 to 15 percent slopes	83.5	10.9%
BvD	Burch loam, 15 to 25 percent slopes	28.4	3.7%
CcA	Cashmont sandy loam, 0 to 3 percent slopes	22.4	2.9%
CdC	Cashmont gravelly sandy loam, 8 to 15 percent slopes	1.6	0.2%
CnE	Cle Elum silt loam, 25 to 45 percent slopes	120.8	15.7%
CnF	Cle Elum silt loam, 45 to 65 percent slopes	6.1	0.8%
CoF2	Cle Elum-Rock outcrop complex, 25 to 65 percent slopes	18.5	2.4%
CwD	Cowiche silt loam, 15 to 25 percent slopes	1.8	0.2%
Pe	Peoh silt loam	3.8	0.5%
PhB	Peshastin loam, 3 to 8 percent slopes	22.3	2.9%
PID	Peshastin stony loam, 0 to 25 percent slopes	30.8	4.0%
РоВ	Pogue fine sandy loam, 3 to 8 percent slopes	30.3	3.9%
PrB	Pogue gravelly fine sandy loam, 3 to 8 percent slopes	3.8	0.5%
PrD	Pogue gravelly fine sandy loam, 15 to 25 percent slopes	5.3	0.7%
Те	Terrace escarpments	23.2	3.0%
VaC	Varelum silt loam, 3 to 15 percent slopes	36.3	4.7%
VaD	Varelum silt loam, 15 to 25 percent slopes	11.1	1.4%
VaE	Varelum silt loam, 25 to 45 percent slopes	3.2	0.4%
W	Water	48.0	6.2%
WeB	Wenatchee silt loam, 3 to 8 percent slopes	37.5	4.9%
Totals for Area of Inter	est	768.9	100.0%

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quivers and the kind and nature of the , with at least one entry for each ches	materi	formation
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Gumbo CLAY	1	7
RAUEL + CLAY	7	29
MEPTED SAND +	10	
RNUEL	29	36
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ER'S STATEMENT:	,	
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(Well Driller)	1. S	
1749 Date 44	72	
	7	445 <u>(4</u> 7)
	1249 Date 4	1249 Date 45 7

The Department of Ecology does NOT Warranty the Data and/or the Information on this Well Report.

File Original and First Copy with	
the Division of Water Management	
Second Copy — Owner's Copy	
Third Copy - Driller's Copy	

WATER WELL REPORT STATE OF WASHINGTON

Permit	No.	 	•••••	

OWNER: Name R. A. Grohne, dbs RIVERVIEL OR	Harman Rt 1. Cashmere. Wa. 98815
'ng and distance from section or subdivision corner 2281.5' E an	nel 352'S of the NN Cor of Sec 27
	(10) WELL LOG:
- 1	
	Formation: Describe by color, character, size of material and structure, an show thickness of aquifers and the kind and nature of the material in ea strutum penetrated, with at least one entry for each change of formation
(if more than one)	MATERIAL FROM TO
Deepened : Cable Driven	·
Reconditioned 🗌 Rotary 🗌 Jetted 🗌	
DIMENSIONS: Diameter of well 4 inches. Drilled 20 ft. Depth of completed well 34 ft.	
CONSTRUCTION DETAILS.	Deepened by drilling and installing
	4" casing below dug section during
	1955-56 period. Further information
Welded [] The many from	not available
Perforations: Vet D No CV	
Type of perforator used	R. A. Grohne, present owner.
SIZE of perforations	
perforations from ft. to ft. to ft. to	
Screens: Ver M No D	
Manufacturer's Name. No. rscord available	· · · · · · · · · · · · · · · · · · ·
Type	
	RECHIVED
	MAR 3 0 1971
Did any strata contain unusable water? Yes No	DEPARTMENT OF ECOLOGY
Type of water?	
PUMP: Manufacturer's Name Pacific - Jet Type: JN3 14x3/4 HP	· · · · · · · · · · · · · · · · · · ·
WATER LEVELS: Land-surface elevation 970 ft.	
c level	
(Cap, valve, etc.)	
WELL TESTS: Drawdown is amount water level is iowared below static level	
a pump test made? Yes 🗌 No 📋 If yes, by whom?	Work started
i: gal./min. with ft. drawdown after hrs.	WELL DRILLER'S STATEMENT:
	This well was drilled under my jurisdiction and this report . true to the best of my knowledge and belief.
very data (time taken as zero when pump turned off) (water level	
neasured from well top to water level) me Water Level Time Water Level Time Water Level	NAME
	(Person, firm, or corporation) (Type or print)
	Address
ate of test	[Signed]
dan flow	
persture of water	Date
	New vell Image: Second the seco

bird Copy - Driller's Copy	Application Application Application	- (<u> </u>
(1) OWNER: Name Charan Co. Dat. of Public Works	Address Chaken Co. Court house 1 terest cha	Wa.g	19801
LOCATION OF WELL: County CHELAN	_ SE 14 NW 14 Sec 27 TZ		
bearing and distance from section or subdivision corner			
(3) PROPOSED USE: Domestic 🗶 Industrial 🗆 Municipal 🗋	(10) WELL LOG:		
Irrigation 🗌 Test Well 📋 Other 🔲	Formation : Describe by color character, size of materia	al and stru	CTATE. 67
b) TYPE OF WORK: Owner's number of well	show thickness of aquifers and the kind and nature of i stratum pensivated, with at least one entry for each c	the materi	
New well Method: Dug Bored	MATERIAL	FROM	то
Deepened 🗌 Cable 🗌 Drivan 🗍	BROWN CLAY		_33
Reconditioned Rotary Jetted	SANDY CLAY WB	33	36
ONS: Diameter of well Sinches	Shout cent wa	-00-	
7	BROWN CLAY	36	49
SUCTION DETAILS:			
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Diam. from ft. to ft.	SANDSTONE	52	्रम
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d: Yes D No Size of gravel;		<u>├</u>	
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ethod of sealing strate off			
" Manufagurer's Name BERKLEY		├ ─── ↓	
SUBMERSIBLE HP 3		├────┤	
LEVELS: Land-surface elevation above mean sea level			
tt. below top of well Date 8-26-87	OCTANTMENT OF ECOLOGY		
tesian water is controlled by	PENTRAL REGION OFFICE	<u> </u>	
		<u> </u>	
TESTS: Drawdown is amount water level is lowered below static level	Work started 8 - 25 19 87 Completed	8-26	
made? Yes D No k If yes, by whom?	WELL DRILLER'S STATEMENT:		
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10 n n	true to the best of my knowledge and belief.		eport is
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5.5 gal/min. with ft. drawdown after /2 he.	(Well Griller)	·	••••••••••••••••••••••••••••••••••••••
are of water	Lionse No. 1249 Date 8.	- 28	1.87

(USE ADDITIONAL SHEETS IF NECESSARY)

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IER: Name Della NOCL ATION OF WELL: County CHELAN EET ADDDRESS OF WELL (or nearest address) POSED USE: Domestic Irrigation Irrigation Test Well Municipal POSED USE: Domestic Irrigation Irrigation Test Well Municipal E OF WORK: Owner's number of well (If more than one) Test Well Other Boned New well Method: Dug Bored Driven Boned Reconditioned Rotary Jetted Inches. Installed: ' Diam. from ft. to T. to ft. to '' Diam. from ft. to ft. to ft. to ft. to '' Diam. from ft. to ft. to ft. to ft. to	Formation: Desc thickness of aquife	Water Right Permit No 9003 Motel I SE & NW OG or ABANDONM ribe by color, character, era and the kind and nature ntry for each index of inf MATERIAL LORMY - 1 ROCKS -	K Sec 27 T.2 ENT PROCEDUI	RE DESC	RIPTIO
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TER LEVELS: Land-surface elevation above mean eaa leval level B12 ft. below top of well Date4-/1-88				L OUT	
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Artesian water is controlled by(Cap. valve, etc.))		11-11-09	4	-77-3	88-
L TESTS: Drawdown is amount water level is lowered below static level	Work started		Completed		
pump lest mede? Yes No Hyse, by whom?	WELL CONS	STRUCTOR CERTIF			
gal,/min. with fi. drawdown after hrs.	I construct	ted and/or accept res ompliance with all We	Distant for con		
	Materials (used and the informati	on reported above	are true t	to my b
ery data (time taken as zero when pump turned off) (water level measured rell top to water level)	Knowledge	and delief.	0		
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	Piegistratign		4~	12	·
lest gal./min. with ft. dravedown after hra.	[] ~~} 0,∞9 				
	ii top to water level) Water Level Time Water Level	by data (triffe laken as zero when point in red on) (water level NAME Water Level Time Water Level Water Level Time Water Level Date of test	It top to water level) Time Water Level Time Water Level NAME TWMUD A 7ER Water Level Time Water Level Image: Mater Level NAME Image: Mater Level Date of test	If op to water level Time Water Level Time Water Level Water Level Time Water Level Time Water Level NAME Tume Water Level Time Water Level Date of test	If top to water level Time Water Level Time Water Level NAME Tumu A 7 E R D R3 LL3N L Water Level Time Water Level Time Water Level NAME Tumu A 7 E R D R3 LL3N L Date of test

The Department of Ecology does NOT Warranty the Data and/or the Information on this Well Report.

	S		-	
		original and First Copy with Startment of Ecology With	ELL REPORT F Start Card No. 🖉	16192
		cond Conv—Owner's Conv	WASHINGTON Water Right Permit No.	
ť	/41	OWNER: Name DENTS S. KNISHRA	Address Kok 56 , DRYDEN , WA	, 98831
Report	(2)	LOCATION OF WELL: County CHELAN	<u>SE & NW & Sec</u> 27 ;	24 . /0
	(24) STREET ADDDRESS OF WELL (or meanest eddress) 8990 MOTE		AT#/145
We	(3)	PROPOSED USE: Domestic Industrial D Municipal D	(10) WELL LOG or ABANDONMENT PROCEDU	RE DESCRIPTION
		DeWater Test Well Other TYPE OF WORK: Owner's number of well	Formation: Describe by color, character, size of material an thickness of aquiters and the kind and nature of the material in er	d structure, and show sch stratum penetrated
this	(-/	Abandoned New well Method: Dug Bored	with at least one entry for each change of information. MATERIAL	FROM TO
on		Deepened D Cable Driven Reconditioned Rotary Jetted	BLACK TOPSOIL	03
the Information	(5)	DIMENSIONS: Diameter of well6	GRAVEL, SAND + ROCKS	3 39
lati		Drilled feet. Depth of completed well 28/2ft.		
E	(6)			
Ĕ		Casing installed: Diam. from Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model Model		
ē		Threaded Diam. fromft. to ft.		
		Type of perforator used		
20		SIZE of perforations in. by in.		
and/or		perforations from ft. to ft.		
		perforations fromfl. toft.		
Data		Screens: Yes _ No		
the		Sype Model No		
		Diam Slot sizefromft. toft. Diam Slot sizefromft. toft.		
rranty		Gravel packed: Yes No Size of gravel		
arr		Gravel placed fromft. toft.		
2		Surface seal: Yee Not To what depth?f.		
0		Did any strate contain unusable water? Yes No	TERENNE T	
Σ s		Type of water?		
oes	(7)	PUMP: Manufacturer's Name	Alig 2 4 988	
ĕ ≻		Type:H.P		
Ecology	(8)	WATER LEVELS: Land-surface elevation above mean sea level	DEPARTMENT OF THE DEPARTMENT	
<u> </u>		Artesian pressure lbs. per square inch Date Artesian water is controlled by	CENTRAL REGION TO LEE	
оt	(9)	(Cap, valve, etc.))	Work started 8-12, 19. Completed 8	
	\•,	Was a pump test made? Yes No Kit yes, by whom?	WELL CONSTRUCTOR CERTIFICATION:	,
Jer		Yield: gal./min. with ft. drawdown after hrs.	I constructed and/or accept responsibility for constr	uction of this well,
partment		Recovery data (time taken as zero when pump turned off) (water leve) measured	and its compliance with all Washington well const Materials used and the information reported above a knowledge and belief.	niction_standarda.
epa		from well top to water level) Time Water Level Time Water Level Time Water Level		T
Ō			NAME LUMULATER DRALLAM	
lhe			ADDREADERTH, (L)AS	<u>.</u>
	<u></u>	Baller test gal./min. with ft. drawdown after hrm.	(Bigned Kastt Katter Dicense No	<u>/249</u>
		Airtest 45 gal./min. with stem set at 3.7 flufor 1 hm.	Contractor's (WELL DRILLING	
		Artesian flow g.p.m. Date G-12 - 58	No. 14 Philip A DI - 1330C. Date 8-	<u>12 18</u>
57	~ ~~~	Temperature of water Was a chemical analysis made? Yes No	(USE ADDITIONAL SHEETS IF NECESS)	ARY)

Dep	Original and First Copy with write water of Ecology With		<u>9112</u> 911
Sec Thi	ong Copy — Dwner's Copy	WASHINGTON Weter Right Permit No.	<u></u>
	OWNER: Name DENNIS KNISHKA	POBOX 56 ORYDEN, WA 988	21
(20) (~~)	LOCATION OF WELL: COUNTY CHELPO	SE 1/4 NW 1/4 Sec 27 124 N. 1	<u>18Ew</u>
	STREET ADDRESS OF WELL (or nearost address) 8900 MOTEL	RD. DRYDEN, WA.	
(3)	PROPOSED USE: Oomestic Industrial I Municipal I	(10) WELL LOG or ABANDONMENT PROCEDURE DESCRIP	
	DeWater Test Well DOther	Formation: Describe by color, character, size of material and structure, and show thick and the kind and nature of the material in each stratum penetrated, with at least on	mess of aquite e entry for eac
(4)	TYPE OF WORK: Owner's number of well (If more than one)	change of information.	TO
	Abandoned 🗌 New well 📜 Method: Dug 🗆 Bored 🗍 Despened 🗍 Cable 🖵 Driven 🗌	LLACK SANDY JOPSOZI O	2
(5)	DIMENSIONS: Diameter of weil inches Drilled feet. Depth of completed weil 381/ ft	BLACK GRAVEL, COBBLES 2	13
(6)	CONSTRUCTION DETAILS:	BLACK GRAVEL + SAND 13	16
	Casing installed: Diam. from _+112t. to 381/2 tt	WATER BEARTUL	
	Welded Diam, from tt. to tt. to tt.	HARD PACKED RUNCK	-
	Threaded Diam. from ft. to ft.	BROKEN GRAVEL, CLAY, Rad 16	28
	Perforations: Yes No X		
	SIZE of perforations in. by in.	WATER BEARTHL	317
	ft. toft.		
	perforations from ft. to ft.	Ⅰ 1	
	Screens: Yes No X		
	Manufacturer's Name		
	Type Model No		
	Gravel packed: Yes No Kize of gravel		+
	Gravel placed fromft. toft.		
	Surface seal: Yes No D To what depth? ft.	+6	
	Material used in seal <u>SENTONITE</u> Did any strate contain unusable water? Yes No X		
	Did any strata contain unusable water? Yes 🗌 No 🔀 Type of water? Depth of strata		
	Method of sealing strata off		
7)	PUMP: Manufecturer's Name	DEPARTMENT OF ECCORAT	
<u>́</u>	Type: H.P	CENTRAL REGION OFFICE	┦──
3)	WATER LEVELS: Land-surface elevation	Work Started 8-2 19. Completed 8-2	
	Static level ft. below top of well DateY	WELL CONSTRUCTOR CERTIFICATION:	
	Artesian water is controlled by	I constructed and/or accept responsibility for construction of this w	منا ممر الم
	(Cap, valve, stc.)	compliance with all Washington well construction standards. Material the information reported above are true to my best knowledge and bell	I used end
"	WELL TESTS: Drawdown is amount water level is lowered below static level Was a pump test made? Yes No X If yes, by whom?		
	Yield: gal./min. with ft. drawdown after hra.	NAME TUMINAZER DRZLLITUL	4rc.
	n n n n	Address LEAVENWORTH, UPALH.	
	" " Recovery data (time taken as zero when pump turned off) (water level measured from well	(Signed) R TH V License No.	249
	top to water level) me Water Level Time Water Level Time Water Level	(WELL DRILLER)	┍╌╶┥╋┈╴
		Contractor's Registration	~1
		No. TUMIONDE 330C Date 8 3	_ 19 24
	Date of test	(USE ADDITIONAL SHEETS IF NECESSARY)	
	Bailer test gal./min. with ft. drawdown after hra. Airtest Qgel./min. with stern set at 38_ ft. jgr hra.	Ecology is an Equal Opportunity and Affirmative Action employer.	For and
	Artesian flow g.p.m. Date 9-2-9Y	ciel accommodation needs, contact the Water Resources Program	n at (206)
	Temperature of water Was a chemical analysis made? Yes 🗶 No 🗌	407-6600. The TDD number is (206) 407-6006.	-

The Department of Ecology does NOT Warranty the Data and/or the Information on this Well Report.

Seco	Driginal and First Copy with rtment of Ecology nd Copy—Owner's Copy Copy—Driller's Copy STATE OF 1	WASHINGTON		
- • •	OWNER: NEMO_STANLEY D. Price	Hater Right Pormit No	leta.	9887
(2)	LOCATION OF WELL: County CHELAN	<u>SE NW sec 27 1</u>		
	STREET ADDDRESS OF WELL (or searest address; 899)	Motel Rd. Onuda like 988.	21 21	2 ,18 .w. Y
	PROPOSED USE: Pomestic Industrial Municipal	(10) WELL LOG OF ABANDONMENT PROCEDU		
,	Irrigation Industriat Multicipat DeWater Test Well Other	Formation: Describe by color, character, size of material an		
(4)	TYPE OF WORK: Owner's number of well (if more than one)	thickness of aquifers and the kind and nature of the material in e with at least one entry for each change of information.	ach stratu	m penetrat
	Abandoned Diversitian one)	MATERIAL	FROM	то
	Deepened Cable Driven C Reconditioned Rotary X Jetted C	BLACK TOPSOIL	0	
5)	DIMENSIONS: Diameter of well inches.	ROCKS, GRAVEL, SAND, CUA	1	39
	Drilled 39 feet. Depth of completed well 38 It.			
6)	CONSTRUCTION DETAILS:			
	Casing installed: Diam. from/Z_ft. to T			
	Welded * Diam. fromft. toft.			
	Threaded * Diam. fromft. toft. Perforations: Yes No X			
	Type of perforator used			-
	SIZE of perforations in. by in.			
	perforations fromft, toft, toft,			
			ł	
	Manufacturer's Name			
	Type Model No		ļ	+
	Dism Slot size from ft. to ft.		<u> </u>	
(Gravet packed: Yes No Size of gravel		ļ	
(-+
	Surface seal: Yes No To what depth?			ļ
	Meterial used in seal <u>SEN7ANA7E</u> Did any strata contain unusable water? Yes NdX			
	Type of water?Depth of strate			+
	Method of acaling strata off			
7)	PUMP: Manufacturer's Name			
	WATER LEVELS: Land-surface elevation		····-	
•	Static level			
,	Artesian pressure Ibs. per square inch. Date			
	Artesian water is controlled by(Cap. valve, etc.))		2-8	
9)	WELL TESTS: Drawdown is amount water level is lowered below static level Nas a pump test made? Yes No K II yes, by whom?	Work started 12 - 1	4-0	19
	field: gel./min. with ft. drawdown efter hre.	WELL CONSTRUCTOR CERTIFICATION:		
		I constructed and/or accept responsibility for const and its compliance with all Washington well cons	struction	standard
	Recovery data (time taken as zero when pump turned off) (water level measured	Materials used and the information reported above knowledge and belief.	are irve i	to my bee
	rom well top to water level) Time Water Level Time Water Level Time Water Level	NAME TUMWATER DRILLA	, D	lun
		(PERSON, FIRM, OR CORPORATION)	(TYPE (DR PRINT)
		Address LEAVENWORTH WI	HZA	
	Date of test		ŀ	วปจ
	Saller test gal./min. with fi, drawdown after hra.	(Signed) (WELL DRALER) (WELL DRALER)	10£	~1/
	Nintest gel./min. with stem set at ft. for hrs.	Registration No. TUMWADZ-1330C Date (2-	9	1086
	rtesian flow g.p.m. Date		1	, I VLU

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WATER WELL REPORT

Start Card No. # 047936 Unique Well I.D. # NOME Water Right Permit No.

STATE OF W	Unique Well I.D. # MOME ASHINGTON Water Right Permit No.	
1) OWNER: Name PROSCH, DAVID Address 8896 (DEAD MAN HILL RD. CASHINERE, WA 98815-	
2) LOCATION OF WELL: County CHELAN 2a) STREET ADDRESS OF WELL (or nearest address) SAME,		
3) PROPOSED USE: DONESTIC	(10) WELL LOG	
4) TYPE OF WORK: Owner's Number of well (If more than one) NEW WELL Nethod: ROTARY	Formation: Describe by color, character, size of mater and structure, and show thickness of aquifers and the and nature of the material in each stratum penetrated, at least one entry for each change in formation.	kind
5) DINERSIONS: Diameter of well 6 inches Drilled 311 ft. Depth of completed well 302 ft.	NATERIAL FROM	10
6) CONSTRUCTION DETAILS: Casing installed: 6 * Dia. from +1.5 ft. to 133 ft. WELDED W/LINER 4 * Dia. from -129 ft. to 302 ft. " Dia. from ft. to ft.	CRAY SUALE 32	32 34 38 68 82 84 113
Perforations: YES Type of perforator used SKILL SAW SIZE of perforations 7 in. by .125 in. 136 perforations from 227 ft. to 302 ft. perforations from ft. to ft. perforations from ft. to ft.	GRAY CLAY SHALE SANDSTONE 84 WHITE CLAY 113 BROKEN SEALE BROWN CLAY 129 BRU. SANDSTONE 133 DICAYED BRU. SANDSTONE CLAY 137 BROKEN SEALE CLAY 157	113 129 133 137 157 165 176
Screens: NO Manufacturer's Name Type Model No. Diam. slot size from ft. to ft. Diam. slot size from ft. to ft.	BEN. SANDSTONE 165 SHALE BROWN CLAY 176 BEN. SANDSTONE 192 VOID CAVING 208.5 BROWN CLAY BRN. SANDSTONE SHALE 211 MATER BRANING 270 DBCAYED BRN. SANDSTONE 282	1/0 192 208 211 282 275 302
Gravel packed: NO Size of gravel Gravel placed from ft. to ft.	NATER BEARING 287 CAVING BROKEN BRIL. SANDSTONE 302	291 311
Surface seal: YES To what depth? 19 ft. Material used in seal BERTOWITE Did any strata contain unusable water? NO Type of water? Depth of strata ft. Nethod of sealing strata off SEAL NETHOD 1	AUG 2 page	
7) POMP: Manufacturer's Name Type SUBMERSIBLE H.P.		
8) WATER LEVELS: Static level 239 ft. below top of well Date 07/13/95 Artesian Pressure 1bs. per square inch Date Artesian water controlled by CAP	Bark started 07/11/05	
9) WELL TESTS: Drawdown is amount water level is lowered below static level. Was a pump test made? MO If yes, by whom? Yield: gal./min with ft. drawdown after hrs.	Work started 07/11/95 WELL CONSTRUCTOR CERTIFICATION: I constructed and/or accept responsibility for con- struction of this well, and its compliance with all Washington well construction standards. Naterials and the information reported above are true to my b knowledge and belief.	used
Recovery data Time Water Level Time Water Level Time Water Level	HAME TUMMATER DEILLING, INC. (Person, firm, or corporation) (Type or print)	
Date of test / / Bailer test gal/min. ft. drawdown after hrs. Air test 20 gal/min. w/ stem set at 302 ft. for 1.5 hrs. Artesian flow g.p.m. Date	ADDRESS P.Q.BOI 777 [SIGHED] AT License No. 1249 Contractor's	
Temperature of water Was a chemical analysis made? NO	Registration No. TUNNADI 1330C Date 07/15/95	

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File Original and First Copy with Department of Ecology
Second Copy — Owner's Copy Third Copy — Driller's Copy

WATER WELL REPORT

STATE OF WASHINGTON

Start Card No. ___ UNIQUE WELL I.D. # _

W057204 ABQ 443

This	d Copy — Driller's Copy STATE OF W	VASHINGTON Water Right Permit No
ਦ [‴]	OWNER: Name Don Hansen Add	6969 Stinehill Rd. Cashmere, WA. 98815
<u>o</u> (z)	LOCATION OF WELL: CountyChelan	<u>- SW 1/4 SE 1/4 Sec 27 t. 24 N. R 18E WM</u>
(<u>)</u>	STREET ADDRESS OF WELL (grown atten) 8807 . Tosephin	e Ave. Dryden, WA. 98821
= ⁽³⁾		(10) WELL LOG or ABANDONMENT PROCEDURE DESCRIPTION
Š.	DeWater Test Weil DOther	Formation: Describe by color, character, size of material and structure, and show thickness of aquilers and the fund and nature of the material in each stratum penetrated, with at least one entry for each
ω ⁽⁴⁾	TYPE OF WORK: Owner's number of well (If more than one)	change of information.
th th	Abandoned D New well 2 Method: Dug D Bored D	MATERIAL FROM TO silty gravels 0 3
с	Despensed 🗔 Cable 🗔 Driven 🗌 Reconditioned 🗆 Rotary 🍕 Jetted 🗆	silty gravels 0 3 gravel, cobbles & boulders 3 15
0 _ (5)	DIMENSIONS: Diameter of well 6 inches.	sand & gravel, coarse w/water 15 28
<u>5</u> (Drilled 80 feet. Depth of completed well 80 ft.	sandstone, brown w/lenses of
<u> </u>	CONSTRUCTION DETAILS:	shale 28 70
Information	Casing installed: <u>6</u> Diam. from <u>+2</u> ft. to <u>27</u> ft.	w/water 7075
ò	Weided 4	sandstone, brown/grey 75 80
Ξ	Threadedt.	
ene 	Perforations: Yes No	
	Type of perforator used skill saw	
2	SIZE of perforations 1/4 in. by 6 in.	
<u>a</u>	<u>35</u> perforations from <u>60</u>	
and/or	perforations from t. to t.	
<u>س م</u>	Screens: Yes No M	
Data 	Manufacturer's Name	
	Type Model No	
the	Diam. Slot size from ft. to ft.	
	DiamStot sizeft. toft.	
warranty	Gravel packed: Yes No 🕅 Size of gravel	
Ľ		
Va	Surface seal: Yes X No To what depth? 18 ft. Material used in seal bentonite	0EC 1 2 1921
2	Did any strata contain unusable water? Yes No 🕅	
ō	Type of water? Depth of strata	
Z	Method of sealing strata off	
a B B B B B B B B B B B B B B B B B B B	PUMP: Manufacturer's Name	
<u>8</u>	Туре: Н.Р	
>(8)	WATER LEVELS: Land-surface elevation above mean sea level ft.	Work Started 10/6/94 19. Completed 10/7/94 19
	Static level 14 R. below top of well Date 10/7/94	WELL CONSTRUCTOR CERTIFICATION:
ō	Artesian pressure lbs. per square inch Date Artesian water is controlled by	
<u>й</u>	(Cap, valve, etc.)	I constructed and/or accept responsibility for construction of this well, and its compliance with all Washington well construction standards. Materials used and
с (9)	WELL TESTS: Drawdown is amount water level is lowered below static level	the information reported above are true to my best knowledge and belief.
	Was a pump test made? Yes No [3] If yes, by whom? Yield:	NAMEFOGLE_PUMP & SUPPLY_INC
6		
Ē	······································	Address <u>316 W. 5th Colville, Wa. 99114</u>
Ц —	Recovery data (time taken as zero when pump turned off) (water level measured from well	(Signed) Mile Kolain in for License No. 1544
epartment	top to water level) Time Water Level Time Water Level Time Water Level	· ·
ے د		Contractor's Registration
 De	······································	No FOGLEPS095L4 Date 10/7/94 19
<u>_</u> _	Date of test	(USE ADDITIONAL SHEETS IF NECESSARY)
	Baller teet gel./min. with ft. drawdown after hrs.	
	Airteetgel./min. with stern set atft. for hrs.	Ecology is an Equal Opportunity and Affirmative Action employer. For spe- cial accommodation needs, contact the Water Resources Program at (206)
	Ariseian flow g.p.m. Date Temperature of water Wee a chemical analysis made? Yes 🗔 No 🕅	407-6606. The TDD number is (206) 407-6006.
	- /	
ECY	050-1-20 (993) **† · · · · · · · · · · · · · · · · · ·	·

		LL REPORT Start Card No
	OWNER: Name LARRY APILIAN	Address 8892 DEND MAND HALL RD, CASHINE
- (2) (2a)	LOCATION OF WELL: County CHELAN STREET ADDDRESS OF WELL (or nearest address) (SAME)	SW SE 1 Sec 27 1 24 N. R/8 W.M
	PROPOSED USE: Comeatic Industrial Municipal	(10) WELL LOG or ABANDONMENT PROCEDURE DESCRIPTION
(4)	DeWater Test Well Other TYPE OF WORK: Owner's number of well	Formation: Describe by color, character, size of material and structure, and show thickness of aquilers and the kind and nature of the material in each stratum penetrated with at least one entry for each change of information.
	Abandoned Deepened Decable Driven	ELISTER 4 12" HAND PUG O 12
(5)	Reconditioned Rotary 54 Jetted DIMENSIONS: Diameter of well	BRODD STLTY CLAY +
	Drilled Yofeet, Depth of completed well H. CONSTRUCTION DETAILS:	(RE4 CLEY + SHALE 24 40
	Casing installed: <u>6</u> · Diam. from <u>+11/2</u> ft. to <u>40</u> ft. Welded . Diam. from <u>+11/2</u> ft. to <u>- ft.</u> Liner installed	LREY CLAY & SHALE 24 40
	Threaded* Diam, fromfi. tofi. Perforations: Yes A No ROTARY MILLS Type of perforator used ROTARY MILLS	
	SIZE of perforations	
	Manufacturer's Name Model No	
	Diam Slot aizefromft. toft. Diam Slot aizefromft. toft.	
	Gravel packed: Yes No Size of gravel	
	Surface seal: Yes X No To what depth? 18 11. Material ward in seal BEN70N17E	
	Did any strate contain unusable water? Yes No 🕱 Type of water?Depth of strate	
	PUMP: Manufacturer's Name	
	Type:	
	Static level ft. below top of well Date Artesian pressure lbs. per equare inch Date Artesian water is controlled by	
(9)	(Cap. valve. etc.)) WELL TESTS: Drawdown is amount water level is lowered below static level	Work started 4-27 19. Completed 4-27 19.9
	Was a pump test made? Yes No K If yes, by whom? Yield: gal./min. with ft. drawdown after hrs.	WELL CONSTRUCTOR CERTIFICATION: I constructed and/or accept responsibility for construction of this well, and its compliance with all Washington well construction standards.
	Recovery data (time taken as zero when pump turned off) (water level measured	Materials used and the information reported above are true to my best knowledge and belief.
	from wail top to water level) Time Water Level Time Water Level Time Water Level	NAME THIM ATER DRILLINL TWIC OF PRINTS
		ADDRONG LEAVENWORTH, WASH.
	Date of test gel, / min. with ft, drawdown after hrs.	(Signed) Long (Well DRallers
	Airlest gal./min. with stem set et H, for hrs. Artesian flow g.p.m. Date A7293 Temperature of water Was a chemical analysis made? Yes No	No. [WWWADZ 1350LDate 4-28 10 97

ECY 050-1-20	(10/87)	- 1329-
ECY 050-1-20	(10/87)	-1329-

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	Jepa Secol	la copy — Owner's Capy	ELL REPORT	Application No.	\mathcal{O}^{-}
	(1)				
ئد	<u>(1)</u>		Address P.D. BOX 332	, Deyde	n wa
E	(OCATION OF WELL: County Chilan	-SW SE SE	<u>。</u> 2 ″ _т 2 4 _N	RI BWM
g	Bear	ing and distance from section or subdivision corner 20+	10 Um Banc Flat	+ {	
Re	(3)	PROPOSED USE: Domestic X Industrial [] Municipal []	(10) WELL LOG:		<u></u>
	(8)				
ſell		Irrigation 🗋 Test Well 🗌 Other 🛄	Formation: Describe by color, character, so show thickness of aquifers and the kind a	nd nature of the mo	sterial in each
Ś	(4)	TYPE OF WORK: Owner's number of well (if more than one)	stratum penetrated, with at least one entr	· FRC	
<u>s</u>		New well 🔲 Method: Dug 🔲 Bored 🗋	Acad 2 da Sc	/ FAC	M TO ア クラ
this		Deepened 🕅 Cable 🗗 Driven 🗍	go av y + Sam		
ç		Reconditioned 🗌 Rotary 🗌 Jetted 🗌	- Saud Stonk		7 30
0	(5)	DIMENSIONS: Diameter of well inches.			
5		Drilledft. Depth of completed wellft.			
Information	(6)	CONSTRUCTION DETAILS:			
na					
Ĕ		Casing installed: 6 Diam. from - 8 tt. to 27 tt.			
Ę		Threaded Diam. from ft. to ft. Welded Diam. from ft. to ft.			
<u>_</u>					
the		Perforations: Yes 🗆 No 📈			1
Ŧ		Type of perforator used			all the is a
and/or		SIZE of perforations			
þ		perforations from	l <u> </u>		
an		perforations from ft. to ft.			
		Screens: Yes 🗋 No 😿			
Data		Manufacturer's Name			
		Type			
the		Diam. Slot size from ft. to ft.			
		Diam		<u>+</u>	<u> </u>
Warranty		Gravel packed: Yes 🗆 No 🕱 Size of gravel;			
an		Gravel placed from			
Ľ		Surface seal: Yes 🗌 No 🛒 To what depth? ft.			
Sa		Material used in seal			
>		Did any strata contain unusable water? Yes 🗌 No 🗌			
5		Type of water?			
ž		Method of sealing strata off			
ŝ	(7)	PUMP: Manufacturer's Name			<u>}</u>
0e		Туре: Н.Р.			
Ō	(8)	WATER LEVELS: Land-surface elevation above mean sea level		i987	<u>+</u> + <u>+</u>
<u>9</u>	•	level 17 tt. below top of well Date 114.3/8.6.			₩ <u>+</u>
<u>0</u>		ian pressure lbs. per square inch Date			1
00		Artesian water is controlled by	CENTRAL N	EGION CAPICE	
ш	<u> </u>				
ď	•	iowered below static level	Work started	mpleted 11/3	19 SL
ų,	Was : Yield	a pump test made? Yes D No D If yes, by whom?	WELL DRILLER'S STATEMEN	275	
en l		. Bandebrer arter sei alla gant arter tita.			
rtmen		n n n	This well was drilled under my ju true to the best of my knowledge as	nd belief.	nis report is
	Recov	very data (time taken as zero when pump turned off) (water level		11 -	
pa	n: Tin	reasured from well top to water level) ne Water Level Time Water Level Time Water Level	NAME GIRSSHEN DVI	Iling 1	k.C
De			(Person, firm, or corpora	tion) (Type o	r print)
ы в			Address Buy 7174	E. Weni	tchee
The	• •		I D OAL) .	
-		ate of test	[Signed] Wlsten Il	essin	
		r test 15 gal/min. with 4 ft. drawdown after hrs.	(Well	Driller)	_
		erature of water	License No. 015 4 D	ate 11/3	
			بال المراجع (المراجع المراجع (المراجع (الم		Ψ-

(USE ADDITIONAL SHIETS IF NECESSARY)

1 3

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	To a name	17~h	And fill		- 10 - 10 - 10 - 10 - 10 - 10 - 10 - 10		· • · · •
File Original and First Copy with Department of Ecology Second Copy—Owner's Copy Third Copy—Driller's Copy	WATER	WE		PORT Water Right Pen	Start Card No	255	34
) OWNER: Name Craig Carson 12	ake side Brot	hards	Address	9230 Dec	damis Hill Rd. 1	estast	in Wa 98
(2) LOCATION OF WELL: County CHE (2a) STREET ADDDRESS OF WELL (or nearest						204	18- N
	striel 🗆 Municij	·	(10) WELL L	OG or ABAND	ONMENT PROCEDU	RE DES	CRIPTION
(4) TYPE OF WORK- Owner's number of well	Well Other		thickness of squife	ribe by color, char ers and the kind and ntry for each change	acter, size of material a nature of the material in of information.	and structur sach stratur	e, and show n penetrated,
Abandoned 🗆 New well 🕅 Method:	Dug 🗆 Bore	•d 🗆		MATERIA		FROM	TO
Deepened	Cable 🗌 🛛 Drive	ren 🗆	BROWN	6LA4		0	68
5) DIMENSIONS: Diameter of well Drilled ユニュー feet. Depth of complete	1 <i>aQ</i>	inches.	HZTISH	fares .	CLINY	68	87
6) CONSTRUCTION DETAILS:		<u> </u>	OK GRE	it in	\Y	87	150
Casing installed: Diam. from Welded Vz. Difference	11/2 n. 10 21	8 n.	RROWN	CLAY		/50	/60
Threaded Diam. from Veak No	ft.to	<u>n.</u>	(REEL JOL	YGREY S	TWOY INBOUCH	1 160	163
Type of perforator usedSK2L	SAW 7	[i qin.	CRPY - BI SALIDSIO		AT WIST	/63	193
perforations from	ft. to	t. [t.]	EROWL	PACKED	SANDY CLAY	193	212
Screens: Yes No No Manufacturer's Name			BROWN	Gumizo	CLAY	212	214
Type Slot size from	Model No	n. [/	LAE GAN	MIBIS CLILL	214	217
DiamStot aizefrom Gravel packed: Yee No Size of gravel	H. to	<u> </u>	GREY .	LIALE I	1/2000021627	217	278
Gravel placed from ft. to Surface seal: Yes X No To what depit			BROKEN	N JUSTICE	STADSTORE	278	28)
Material used in seal <u><u>REN7404</u> Did any strata contain unusable water? Yes </u>			BROWN	SHUDS7	ONE	281	299
Type of water? (ALC) FTLL DRATHA Method of sealing strate offCASED	EDepth of strata	ĥ	WATER	· · · · · · · · · · · · · · · · · · ·			AMED
7) PUMP: Manufacturer's Name			<u>70 5721</u> GARBAGE		FALL NEHR		De_
Туре:		[BREALUA				
B) WATER LEVELS: Land-surface elevation above mean sea level Static level It, below top of we	11 Date 12-20	<u>-</u>					ļ
Artesian pressure Ibs. per square		[······				<u></u>
Artesian water is controlled by	(Cap. velve, etc.))			17.1-			0.00
WELL TESTS: Drawdown is amount water leve Was a pump test made? Yes No K H yes, by	i is lowered below static	ic level	Work started	1-15	19. Completed /	<u>2~ 7</u>	<u>0, 188</u>
Yield: gsl./min. with ft, dra	wilden after	hre.		TRUCTOR CER			
	P.	**	and its con	npliance with all	responsibility for cons Washington well con	struction :	standarda.
Recovery data (time taken as zero when pump turned	off) (water level meester 0 1989 weterL	E	Materiala us knowledge s	led and the inform and belief.	nation reported above	are true t	o my best
·			`	(PERSON, FIRM. OR		TVPE O	A PRINT)
Date of test			$\mathbf{\nabla}$	ADENMON		,	249
	9B n. tor/	Z.m.	(Signed)	WHELL DRILLER	,		<u> </u>
Artesian flow g.p.m. De Temperature of water Was a chemical analysis		x			Date 12 -		_, 19_90

The Denartment of Ecology does NOT Warranty the Data and/or the Information on this Well Report

WATER WELL REPORT

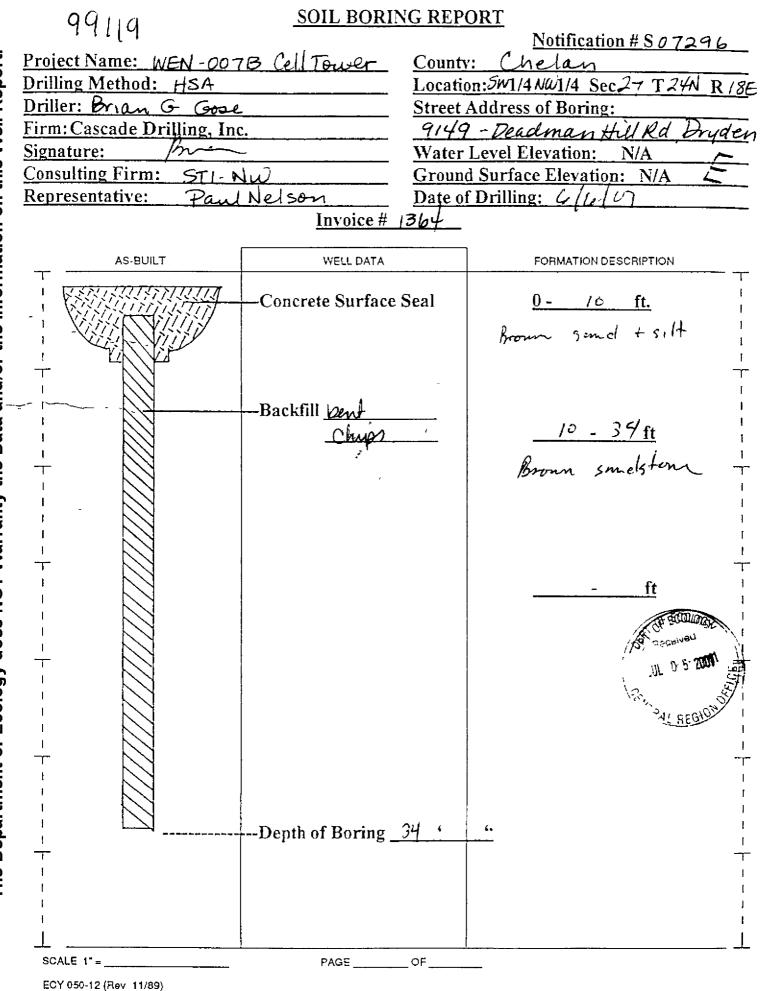
		WATER S	STATE OF W	ASKINGTON	Start Card No. Unique Well I.D. # 1 Water Right Permit 1	No.	
(1) OWNER: Name RB	MAN, WALTER L.	Address	3 P.O.B	DX 26 DRYDEN, WA 98821-	0026		
(2) LOCATION OF WH (2a) STREET ADDRESS	LL: County CHELAN OF WELL (or nearest	address) 8722 S(CHOOL ST.,	OX 26 DRYDEN, WA 98821-0 - א א 1/4 א 1/4 Sec DRYDEN Sec	27 T.24 N., R18B	₩ ***********	4====
				(10) WELL LOG			
4) TYPE OF WORK: NEW WELL	Owner's Numb (If more that Method: ROTA	er of well n one) RY		Formation: Describe by co and structure, and show th and nature of the materia at least one entry for eac	lor, character, size hickness of aquifers l in each stratum p	of mater: and the l enetrated,	ial kind wit
Drilled 69 f	t. Depth of com	ter of well 6 pleted well 59	ft.	MATERIAL LOBMY BROWN CLAY		FROM	T0
6) CONSTRUCTION DF Casing installe WELDED W/TUE	TAILS: d: 6 " Dia. fr X " Dia. fr " Dia. fr " Dia. fr	om +1 ft. to 5 om ft. to om ft. to	59 ft. ft. ft.	SANDY GRAVEL BROWN CLAY ROCKY GRAVEL B BROWN GRAVEL WATER BEARING BLACE BROWN DACK (B)	OULDER (S) G	4 7 31 39 44	7 31 39
Perforations: 1 Type of perf SIZE of perf perfor perfor perfor perfor	0 orator used orations ations from ft ations from ft ations from ft	in.by .to ft. .to ft. .to ft.	in.	BROWN CLAY GRAVEL BLACK BROKEN ROCK (C) BROWN SANDY GRAVEL WATER I	BEARING	48 53	48 53 60
Screens: NO Manufacturer Type Diam. Diam.	's Name Model slot size from slot size from	No. ft. to ft. to	ft. ft.		W =====		
Gravel place	d from ft. to	ize of gravel ft. To what depth? 1					
Material use Did any stra Type of wate Method of se	d in seal BENTUNITE ta Contain unusable r? aling strata off SEA	water? NO Depth of strata L MRTHOD 1	ft.	PEN CEN EGI-M			
7) PUMP: Manufactu	rer's Name Type SUBMERSIB	LE H.P.					
 WATER LEVELS: Static level Artesian Pressu 	Land-surf above mea 31.5 ft. below t re lbs. per sg	ace elevation	ft.				
Artesian water	controlled by CAP			Work started 10/28/99	Completed 3	10/28/99	
9) WELL TESTS: Dra sta Was a pump test n	wdown is amount wate tic level. ade? NO If yes, b /min with ft.	r level is lowere y whom?		WELL CONSTRUCTOR CERTIFIC I constructed and/or a struction of this well Washington well constru- and the information rej knowledge and belief.	ccept responsibility , and its compliance uction standards.	Materials	usec
Recovery data Time Water Le	vel Ti me Water L	evel Time Wat	ter Level	NAME TUNNATER DRILLING, IN (Person, firm, or co)	NC. rporation) (Type o	r print)	
Date of test Bailer test Air test 54 qa	gal/min. ft. ll/min. w/ stem set a g.p.m.	drawdown after t 58 ft. for 1	hrs. 1 hrs.	ADDRESS P. Q. BOX 777 [SIGNED]	ttcense No	. 1249	

The Department of Ecology does NOT Warranty the Data and/or the Information on this Well Report.

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Control Copy — Control Copy	LL REPORT Application	Ion No	
(1) OWNER: Name Lakeside Orchand	Addres PD. Box 357 Portastin	Wa. 98	847
LOCATION OF WELL: County Chele		-24 x pl	2
Bearing and distance from section or subdivision corner		A	
3) PROPOSED USE: Domestic 🖌 Industrial 🗆 Municipal	(10) WELL LOG:		
Irrigation 🗌 Test Well 📋 Other	Formation: Describe by color, character, size of ma	terial and strue	ture, and
4) TYPE OF WORK: Owner's number of well	show thickness of aquifers and the kind and nature stratum penetrated, with at least one entry for ea	of the materia ch change of f	ormation
New well Method: Dug [] Bored []	MATERIAL	FROM	TO
Deepened Cable Driven	BROWN AUMBO CLAY	r _0	38
5) DIMENSIONS: Diameter of well inches.	GRAY CLAY	28	41
Drilled 187 ft. Depth of completed well 187 ft.	SAOWH ARATTY CLAY	97	110
6) CONSTRUCTION DETAILS: Casing installed: 6." Diam. from 0. ft. 10 185 ft.	BAST-ORANAE CLAY + LAP	UEL 110	116
Threaded [] " Diam. from	SANDY GRAY CLAY		/49
Perforations: Yes I No	SILLE-GRAY QUICKSAN	0 1-9	163
SIZE of perforations in. by in,	GRAY GUMBO CLAS	1 763	/68
perforations from ft. to ft. perforations from ft. to ft. perforations from ft. to ft.	BLUE SALDY CLAY	168	185
Screens: Yes D No	GRAY SANDSTONE	185	187
Manufacturer's NameModel No			
Diam. Slot size from ft. to ft. Diam. Slot size from ft. to ft.			
Gravel packed: Yes No Size of gravel: Gravel placed from fi. to fi. Surface seal: Yes No To what depth? ft. Material used in seal Did any strata contain unuable water? Yes NoX Type of water? Depth of strata Method of sealing strata of			
7) PUMP: Manufacturer's Name			
Туре:НР			
(8) WATER LEVELS: Land-surface elevation above mean sea level	- 		
static level 2. ft. below top of well Date 1-7-53			
Artesian water is controlled by			
		-+	
(9) WELL TESTS: Drawdown is amount water level is lowered below static level	Work started // - 1 19 95 Completed	1-7	284
Was a pump test made? Yes I No K If yes, by whom?	WELL DRILLER'S STATEMENT:		
	This well was drilled under my jurisdicti	on and this .	co ort is
0 U U U	true to the best of my knowledge and belie	ť.	
tecovery data (time taken as zero when pump turned off) (water level measured from well top to water level) Time Water Level Time Water Level Time Water Level	NAME TUMWAPPER DR34	HL T	NC
······	Address BR. Box B3c LE		HCR
		T -	
Date of test beller test. 39-1 gal/min. with f. G. st. drawdown after	[Signed]		
emperature of water Was a chumical analysis made? Yes 🕤 Re-	Louis No. 249 Date	£-•	, 10
		çr	
(USE ADDITIONAL (II)	TEREST NECESSARY)	2-16-84	<u>منتحمت</u> ،



File Original and First Copy with Department of Ecology Second Copy — Owner's Copy Third Copy — Driller's Copy	WATER WE State of W	LL REPORT		Application M		
(1) OWNER: Name TOM 400	Arwa	Addres 809	6th Ave Con	1 co Dau	Wa	994
OCATION OF WELL: County CH	ELAD		SWH NW H S		J _{N R}	18 w x
Bearing and distance from section or subdivision cor	ner					
(3) PROPOSED USE: Domestic 🗶 Indust	riel 🗌 Municipal 🗋	(10) WELL LO	G:			
Irrigation [] Test V		Formation: Describe show thickness of ag	by color, character, a wifers and the kind o	nze of materia and nature of t	l and stru he materi	cture, and al in each
(4) TYPE OF WORK: Owner's number of v (if more than one)			with at least one ent	Ty Jor Back Ci	FROM	TO
New well 🚺 Method: I Deepened 📋 💦	Dug 🔲 Bored 📋 Cable 🗌 Driven 🗍	RROWN	SAUDSTO	ONE	Ð	6
Reconditioned []	Rotary Jetted	H CKOOSI				
(5) DIMENSIONS: Diameter of well	6 jaches	SAUTE	JOHE RO	22	6	70
Drilled b ft. Depth of completed	weil <u>65 rt.</u>					
(6) CONSTRUCTION DETAILS:		RROWU S	OFT MOIS	<u>1 2499</u>	78	121_
Casing installed: 6 Diam. from +	12n to 121_n	HARD RD	OUND SALD	+CLAY	121	743
Welded	(.)	w sho	dstone (AYERS		
Perforations: Yes No		GRADEL			143	145
Type of perforator used SIAP	SKIL SAN	BROWN	CANDS70	NE	146	149
SIZE of perforations in, t	n w 1/0 n					
19 perforations from 133	ft. to	SHALE	W Arren			<u>KO</u>
perforations from	n. 10 n.	BROWN	SAUDSTON	5E	150	156
Screens: yes D No						
Type. Mod						
Diam. Slot size from						
Construction of the second sec						
Gravel placed from	gravel: ft.					
Surface seal: Yes M No C To what de	enth? 18 e					
Material used in seal		· · · · · · · · · · · · · · · · · · ·				
Did any strata contain unusable water? Type of water?	Yes Nopes					<u> </u>
Method of sealing strate off						<u> </u>
(7) PUMP: Manufacturer's Name				<i>y</i>]		
Туре:	НР					·
(8) WATER LEVELS: Land-surface elevat above mean sea lev	ו אם ראס		231987			
Static level				ฦ]		
Artesian water is controlled by (Ca	p, valve, etc.)		HEGICAL OF YOL	-		
(9) WELL TESTS Drawdown is amount	water level is					
lowered below static	level om?	Work started 6.	25 <u>19</u> 87 co	mpleted	- 30	<u>, 1987</u>
field: gal./min. with ft. drawdown	n after hrs.		B'S STATEME			
· · · ·		This well was o true to the best of	irilled under my j f my knowledge a	urisdiction a nd belief.	nd this r	eport is
Recovery data (time taken as zero when pump turn measured from well top to water level)	ned off) (water level	_	7			٦.
	ime Water Level	NAME TUMU	DATER			. 412 C .
		1	ENNOUT		ACU	/
·····				<u>' 1</u>		N.a
Date of test		[Signed]	-ull'	14		
Bailer test. 20 gal/min. with			(Well)	Driller	- 0-	
Comperature of water		License No	LA41 D	ate. VO	<u> </u>	19

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epartment of Ecology econd Copy — Owner's Copy hird Copy — Driller's Copy		ELL REPORT WASHINGTON	Application No.	L	
1) OWNER: Name Tom	600DWZN	the lath Aug. 1		9911	6
OCATION OF WELL: County			W 1/4 Sec 2.7 r 24	N D	18
earing and distance from section or subdivisi			TT	N., K	
3) PROPOSED USE: Domestic	Industrial [] Municipal []	(10) WELL LOG:			
Irrigation []		Formation: Describe by color, cha show thickness of aquifers and th	racter, size of material an	nd struc	ture, and
4) TYPE OF WORK: Owner's numb (if more than		stratum penetrated, with at least	one entry for each chan	on of t	ormetion
New well 😿 Me	thod: Dug 📋 Bored 🗍	BROWN CLA		TROM CD	10 ?
Deepened	Cable 🛛 Driven 🗗 Rotary 🗙 Jetted 🗖				
5) DIMENSIONS: Diameter	of well 6 inches.	KRONDU SHINU:	STONE	3	6
Drilled AS 7 ft. Depth of completed well. AS 6. ft.		RROWN CLAY		6	- 47
6) CONSTRUCTION DETAILS:					
Casing installed: Diam. fro		SANDSTONE	SHALE 4	u7	-774
	om ft. to ft. om ft. to ft.				
		RROKEN SAMOS	TONE & CLAY	114	127
Type of perforator medR	MARY STAR	BROKEN BROWN	SANDSTONE	27	132
SIZE of perforations	B n to AL n				
perforations from	ft. to ft.	CREY SANDST	ione 1	132	_157
Compared by Second Seco					
Screens: Yes D No X Manufacturer's Name					
Type					
	om ft, to ft.				
Gravel packed: Yes 🗆 No 💢 s	Size of gravel;				
Gravel placed from	ft. to ft.				
Material used in seal. No D To v	what depth? 30 m.				
Material used in seal					
Type of water?					
7) PUMP: Manufacturer's Home	··	MEGE			_
Type:		U			
8) WATER LEVELS: Land-surface			┩──╫┼┼╢ ────┼──		
tatic level	of well Date 6-25-8				
rtesian pressure	re Inch Date	DEPARTMENT OF E.	SLOGT		
b) While a note lowered below		Work started 6- 24 . 19	87 Completed 6-	<u>25</u>	10.B.
	, by whom?	WELL DRILLER'S STAT	EMENT:		
	** PF	This well was drilled under	er my jurisdiction and	this r	eport is
ecovery data (time taken as zero when pun		true to the best of my know.	Nouse and Dener.	~ ~	,
measured from well top to water level) Time Water Level Time Water Lev	el Time Water Level	NAME UMWATER	URZLIZHE	<u> </u>	PC.
		Address LEAUELING		• or pri c 11	
				÷13	
"Date of test	1 .	In the course	1400		Ma rina da Antonio da Marina da
ailer test	irawdown after		(Well Driller)	ال	المتعقد الم

File Original and First Copy with Department of Ecology Second Copy — Owner's Copy Third Copy — Driller's Copy WATER WELL REPORT Application No. ... STATE OF WASHINGTON Permit No. 508 6th Ale Coalec Den We 95A6 (1) OWNER: Name TOM GODWIN Addres LOCATION OF WELL: County CHELAN SW 1/4 NW 1/4 Sec 27 T 24 N. R / SWM Bearing and distance from section or subdivision corner (10) WELL LOG: (3) PROPOSED USE: Domestic 👗 Industrial 📋 Municipal 🗋 Irrigation [] Test Well [] Other Formation: Describe by color, character, size of material and structure, and show thickness of aquifers and the kind and nature of the material in each strutum penetrated, with at least one entry for each change of formation. Owner's number of well (if more than one)...... Method: Dug (4) TYPE OF WORK: MATERIAL TROM TO New well Method: Dug Bored [1 ROUD CLAY 47 D Deepened D Cable 🖸 Driven Reconditioned Rotary 🗶 Jetted HARD RRWD CLAY + PBBLES 47 63 (5) DIMENSIONS: Diameter of well baches. Depth of completed well 324 ft. BROWN SANDY CLAY 63 8/ (6) CONSTRUCTION DETAILS: Casing installed: 6 " Diam from +12 n. to 133 n. Threaded _____ Diam from the to 133 n. 3<u>n</u>. SANDSTONE DECOMPOSED e/ 100 RR OARK CLAY + SHALE VIEAN 100 120 Welded Perforations: Yes No SYAL SAN Type of perforations ______ B____ in_by SIZE of perforations from 275 ft to 325 ft. 6/______ perforations from 765 ft to 325 ft. ROUD CLAY **200422** なっ 120 774 SAUDSTONE 22 BROWN YUCIAZ CLAY たび 129 perforations from ft. to ft. Screens: Yes D No SANDS70DE 127 ハマ HARD SHALLE 134 136 Diam. Slot size from ft. to ft. **USCOWD** CLAY 136 ाज Gravel packed: Yes D No X Size of gravel: KROOU HARD CLAY 767 184 Surface seal: Yes No D To what depths 2768 **R**. HARD CLAY PEA GRAVEL 194 216 NoX Did any strata contain unusable water? Yes 🗇 Type of water?...... Depth of strate CREY SANDSIDNE 716<u>|</u> 222 Method of sealing strata off. KROWN CLAY (7) PUMP: Manufacturer's Name..... Type: HP. BLACK CRADEL WR 241 232 (8) WATER LEVELS: Land-surface elevation above mean sea level.... Static level 201 tt. below top of well Date 6-24-87 BRONOU TURAJEI **CLR4** 241 222 Artesian water is controlled by..... BRAWN SANDA <u>2191</u> (Cap, valve, etc.) 327 337 (9) WELL TESTS: Drawdown is amount water level is lowered below static level Was a pump test made? Yes No I if yes, by whom? In Mulater, A Yield: 7 gal/min. with (CAVED JU) 19 87 Completed 6 14, 187 Work started 6 NEICII WELL DRILLER'S STATEMENT: hrs. **İIW**E •• This well was drilled under my jurisdiction and this report is Recovery data (time taken as terr when nume terr measured from well top to the reveal of the second second to the second true to the best of my knowledge and belief. ... lawal NAME UMWATER URZLIZE the, (Type or print) (Person, firm, or corporation) Time CENTRAL HEGKON OFFICE WASH EAVENWOR Address zate of test 6-24-87 Bailer test 3 gal/min, with ft drawdown after 11/2 (Bigood) Artesian flow 3 er.

File Original and First Copy with Department of Ecology Second Copy—Owner's Copy Third Copy—Driller's Copy

WATER WELL REPORT

8360 Start Card No. 212618

(1)		Water Right Permit No		1_		
. 7	OWNER: Name Marvin Landon	Address Deadman Hill, Dryde	n, WA	. 98		
	LOCATION OF WELL: County Chelan STREET ADDDRESS OF WELL (or nearest address)	SW corner xx SW _{% Sec} 27 t	24 _{N., R}	18 _w		
		T				
(3)	PROPOSED USE: Domestic Industrial D Municipal	(10) WELL LOG or ABANDONMENT PROCEDU	RE DESC	RIPTIC		
	DeWater Test Well Other	Formation: Describe by color, character, size of material a	nd atructure.	, end eh		
(4)	TYPE OF WORK: Owner's number of well (if more than one)	thickness of aquifers and the kind and nature of the material in each stratum po with at least one entry for each change of information.				
	Abandoned 🗌 New well 🛛 Method: Dug 📋 Bored 🗌	MATERIAL	FROM	τọ		
	Deepened 🗌 Cable 🗋 Driven 🗌 Reconditioned 💭 Rotary 🕰 Jetted 🗌	Overburden	0	3		
		Sand & gravel	3	<u>15</u>		
(5)	DIMENSIONS: Diameter of well 6 inches.	Sandstone, med., brown	15	75		
	Drilled 200 feet. Depth of completed well 200 ft.	Broken rock, grey	75	80		
(6)	CONSTRUCTION DETAILS:	Sandstone, med., brown	80	95		
	Casing installed: <u>6</u> · Diam. from +1 ft. to 74 ft.	Broken rock, grey	95	115		
	Welded 2 _5 Diam. from <u>+2</u> ft. to <u>115</u> ft.	sandstone, brown, med.	.1.15	200		
	Threaded Dimension Threaded Threaded Threaded Threaded Threaded Threaded Threaded Threaded Threader Th					
	Perforations: Yes No		† †			
	Type of perforator used		<u> </u>	••••		
	SIZE of perforations in, by in.		<u>├</u> ···-	_		
	perforations from ft. to ft.					
	perforations from ft. to ft.		†	<u> </u>		
	perforations from ft. to ft.		•			
	Screens: Yes No 🛣					
	Manufacturer's Name			_		
1	Type Model No		311			
	DiamSlot sizefromft. toft.					
	DiamSlot sizefromft, toft.			L		
	Gravel packed: Yes No Size of gravel		1			
	Gravel placed fromft.					
	Surface seal: Yes No To what depth? 20 tt. Material used in seal Bentonite		7			
	Did any strata contain unusable water? Yes No		· · · · ·			
	Type of water?Depth of strata					
	Aethod of seeling strate off					
	PUMP: Manufacturer's Name					
	ype:HPHP			•		
•	WATCH LEVELS: above mean sea level ft.					
	itatic leve) ft. below top of well. Date					
^	utesian pressure Ibs. per square inch_Date Artesian water is controlled by					
	(Cap, valve, etc.))	Work started 2/11 IB Completed 2	718 ···			
נ (נ	WELL TESTS: Drawdown is amount water level is lowered below static level Vas a pump test made? Yes No tryes, by whom?	Work started 2/11, 19. Completed2	/10	_, ₁₉ 9 (
	Cantal and the second second	WELL CONSTRUCTOR CERTIFICATION:				
	ield: gel/min. with H. drawdown after hra.	I constructed and/or accept responsibility for const	ruction of t	his well		
	u u u u	and its compliance with all Washington well cona Materials used and the information reported above a	truction at	andarda my be-		
R	acovery data (time taken as zero when pump turned off) (water level measured om well top to water level)	knowledge and belief.		iny Dea		
	ime Water Level Time Water Level Time Water Level	NAME FOGLE PUMP & SUPPLY, IN(-			
		(PERSON, FIRM, OR CORPORATION)	(TYPE OR	PRINT)		
		Address 316 W. 5th Colville.	μa	0014		
	Date of test	Constant of the Jell COTATIE		221		
			<u> </u>			
	niler test gel./min. with ft. drawdown after hra.	(Signed) Mike BOGLE DRULER) License N (WELL DRULER)				
Ai	niler test gel./min. with ft. drawdown after hrs. intest gal./min. with stem set at ft. for hrs. rtesian flow g.p.m. Date	(Signed) ILLER DOLL / License N (WELL DRALER) Registration No. EDGLEPS0951.4 Date 2/19				

			STI	ATE OF WA	Unique Well I.D. W SHINGTON Water Right Permit		

(1)	OWNER: Name LANDON, MA		Address				
	LOCATION OF WELL. Cou STREET ADDRESS OF WEL	inty CHELAN LL (or nearest addre	ss) 9060 D ea i	DMAN HILL	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	M N	<u> </u>
	PROPOSED USE: DOMESTIC				(10) WELL LOG	r '	
()) == 91	PROPOSED USE. DOMESTIC						 - ·
(4)	TYPE OF WORK: New Well	Owner's Number of (If more than one) Method: ROTARY	2	1	Formation: Describe by color, character, sig and structure, and show thickness of aquifer and nature of the material in each stratum p at least one entry for each change in format	s and the enetrated.	kind
= = = = : (5.)	DIMENSIONS:	Diameter of	well 6	inches			
1.27	Drilled 84 ft.	Depth of completed			MATERIAL	FROM	TO
						0	21
(6)	CONSTRUCTION DETAILS:	5 " Dia. from +2		f = 1	CLAY BROWN GRAVEL CLAY BROWN MEDIUM	32	48
	Casing installed: (b "Dia. from +∡ "Dia. from	ft. to		CLAY BROWN GRAVEL	j 48	55
	HELDED	" Dia from	ft. to	-		55	74
					SOFT	74	1
	Perforations: NO			ł	BROKEN BASALT W/WATER	84	84
	Type of perforator					64	1
	SIZE of perforation			1n.		i	i
	perforations perforations		10 . 	ĺ		1	1
	perforations		Éc			I	
			 .				
	Screens: NO						
	Manufacturer's Name	e Model No.		I			ì
	Type Diam, slotis:		ft. ta	ft		İ	i i
	Diam. slot s		ft. LO	ft.		I	
	Gravel packed: NO	Size of				γ 	
	Gravel placed from	ft. to	ft.				i
	Surface seal: YES		at depth? 20	ft.		i i	i
	Material used in se					l	
	Did any strata con	tain unusable water?				1	
	Type of water?	-	of strata	ft.		1	
	Method of sealing :						i
(7)	PUMP: Manufacturer's					Ì	Ι
		Type NONE	Н.Р.			I	ļ
				*==****			
(8)	WATER LEVELS:	Land-surface el		£		1	I
	Static level 58	above mean sea ft. below top of		ft. 4/15/98	•	i	Ì
	Artesian Pressure			_,,		I.	ł
	Artesian water contro	lled by CAP			 Work started 04/15/98 Completed	n4/15/98	1
					WELL CONSTRUCTOR CERTIFICATION:		
.,,	static le				I constructed and/or accept responsibili	ty for con	1-
	as a pump test made? N				struction of this well, and its complian	ce with al	
Y	ield: gal./min w	ith ft drawdo	own after	hrs.	Washington well construction standards. and the information reported above are t	Materials	best
					and the information reported above are t knowledge and belief.	Lue co my	
F	ecovery data						
	Time Water Level	Time Water Level	Time Wate	r Level	NAME FOGLE PUMP & SUPPLY, INC. (Person, firm, or corporation) (Type	or print)	
					ADDRESS 316 W. STH COLVILLE, WA		
	Date of test /	1					
£		, nin. ft. drawdd	own aft⊚r	hrs.	(SIGNED) Jon Ricard Jon License N	0.2341	
		w/ stem set at 78	ft. for 1.				
	-	р, т.	Date		Contractor's Registration No. FOGLEPS095L4 Date	04/23/98	
	Cemperature of water						

.4444

ርሞኒምድ ሰድ	LL REPORT Start Card No. Unique Well I.D. # AC WASHINGTON Water Right Permit No	W 087015 B377
(1) OWNER: Name MYRICK, LARRY & KAY Address P.O.		- =====================================
	- NE 1/4 SW 1/4 Sec 27 T 24 N., R 18E W , DRYDEN	
3) PROPUSED USE: DOBESTIC	(10) WELL LOG	=============
(4) TYPE OF WORK: Owner's Number of well (If more than one) NEW WELL Nethod: ROTARY	Formation: Describe by color, character, size (and structure, and show thickness of aquifers a and nature of the material in each stratum peop	and the kind
5) DIMENSIONS: Diameter of well 6 inches Drilled 40 ft. Depth of completed well 39 ft.	MATERIAL	
Casing installed: 6 " Dia. from +1.5 ft. to 39 ft. WKLDED " Dia. from ft. to ft. " Dia. from ft. to ft.	BROWN SANDY GRAVEL COBBLES WATER BEARING	PRON TO 0 4 4 16 16 31 31 40
Perforations: BO Type of perforator used SIZE of perforations in. by in. perforations from ft. to ft. perforations from ft. to ft. perforations from ft. to ft.		
Screens: NO Manufacturer's Name Type Nodel No. Diam. slot size from ft. to ft. Diam. slot size from ft. to ft.	AFR R 1005	
Gravel packed: NO Size of gravel Gravel placed from ft. to ft.	A R KOON	
Surface seal: YES To what depth? 19 ft. Material used in seal BENTONITE Did any strata contain unusable water? YES Type of water? FIRST WATER Depth of strata 12 ft. Nethod of sealing strata off SEAL METHOD 1		
) POMP: Manufacturer's Name		
Type SUBMERSIBLE H.P. WATER LEVELS: Land-surface elevation above mean sea level ft. Static level 5 ft. below top of well Date 02/19/96 Artesian Pressure Artesian water controlled by CAP		
	Work started 02/19/96 Completed 02/1	9/96
) WELL TESTS: Drawdown is amount water level is lowered below static level. Was a pump test made? NO If yes, by whom? Yield: gal./min with ft. drawdown after hrs. Recovery data	WELL CONSTRUCTOR CERTIFICATION: I constructed and/or accept responsibility fo struction of this well, and its compliance wi Washington well construction standards. Mate and the information reported above are true t knowledge and belief.	r con- th all rials used
Time Water Level Time Water Level Time Water Level	NAME TUNNATER DRILLING, INC. (Person, firm, or corporation) (Type or pr	int)
Date of test / / ailer test gal/min. ft. drawdown after hrs. ir test 954 gal/min. W/ stem set at 38 ft. for 1 hrs. rtesian flow g.p.m.	ADDRESS P.O.BOX 177, LEADENWORTH [SIGNED] License No. 12	49
emperature of water Was a chemical analysis made? NO	Contractor's Registration No. TUNNADI 1330C Date 02/21	/96

The Department of Ecology does NOT Warranty the Data and/or the Information on this Well Report.

Seper Secor	nd Copy-Owner's Copy		525	
fhird .	Copy-Driller's Copy (Bill 14/11/15)	Water Right Permit No.		
(1)				
	002/		N., R	<u>/8- w.</u>
	PROPOSED USE: Comestic Industriel Municipal	(10) WELL LOG or ABANDONMENT PROCEDURE	DESC	RIPTIO
	STATE OF WASHINGTON WILL GON DATE: CONC. OWNER: Name OWNER: Name DOWNER: Name OWNER: Name DOWNER: Name	and sho penetrate		
		MATERIAL	FROM	то
	Deepened 🗍 🛛 Cable 💭 Driven 🖬 🖞	CRAVEL + ROCKS	0	39
5)		SAND	34	-44
		SRAUEL + ROCKS	<u>44</u>	67
÷.	Casing installed: Diam. from1/12_ H. to8H.	SAND	62	71
		ROCIES	21	26
	Perforations: Yes No X		-76	78
	SIZE of perforations in. by in.			
	-			
	Diam. Slot eize trom tt. to tt.			
	Gravel packed: Yes Ng Size of gravel	M O DEX		
	Gravel placed from H. to H.	SIL DRIVE SHOE		
	Surface seal: Yes A Nej To what depth? 70 the			
		<u>, p) is is is wis</u> jn)[-		
7)				-
	Land audean algustian			
-	WATER LEVELS: above mean sea level	DEMATIMENT OF ECOLOGY		
	Artegien water is controlled by	CENTRAL REGION OFFICE		
<u></u>	(Cap, valve, etc.))	Work started	<u>- 4</u>	<u>19</u>
	Was a pump test made? Yes No 🕰 If yes, by whom?	WELL CONSTRUCTOR CERTIFICATION:		
	·	Materials used and the information reported above ar		
	from well top to water level)			
		(PERSON, FIRM, OR CORPORATION)		a Print)
.,	Date of test			
	Bailer test gal, / min. with ft. drawplown after hrs.	(Signed) ALA (WELL DRULLER) NO.		249
	Airtest gal, / min. with stem set at h. for hrs.	Registration No. 1144 WADZ-BSOC Date 21-12	2	19
	Artesian flow g.p.m. Date Temperature of water Was a chemical analysis made? Yes No		-	., 18 <u>9</u>

The Denartment of Ecology does NOT Warranty the Data and/or the Information on this Well Report

econd Copy — Owner's Copy hird Copy — Driller's Copy	STATE OF WASHINGTON	Permit No.	t	
1) OWNER: Name ALKSMITH				
) LOCATION OF WELL: County	CHELAN	5E 1/ 5W 1/ Sec 22 TZ	$4 \text{ N} \text{ R}^{1}$	8 w.
searing and distance from section or subdivision corner				
3) PROPOSED USE: Domestic 🕱 Industrial				
Irrigation [] Test Well	i show thickness of day	iters and the kind and nature of	The materia	14 IN CO
(4) TYPE OF WORK: Owner's number of well (if groze than one)	strutum penetrateo, a	MATERIAL	FROM	то
New well Method: Dug	Bored Briven BROWN	LOAM	0	77
L	Jetted 🛛	Deals		-17-
(5) DIMENSIONS: Diameter of well	6 inches GRADEL	SUD + KOCK?	/4	7.
Drilled	39			
CONSTRUCTION DETAILS.				
· · · · · · · · · · · · · · · · · · ·				
-	· ·			
Welder	ft. to ft.			
Perforations: Yes 🗆 No 🗶				
Type of perforator used				
			-+	
perforations from	to ft.			
perforations from ft.	to			
Screens: yes 🗆 No 🗶				
				-
Diam. Slot size from	ft. to <u>ft.</u>			
Surface seal: Yes No D DTo what depth	, <i>1</i> 8 ,			
Material used in seal. BEN770				
Did any strata contain unusable water?	WATER WELL REPORT STATE OF WARENORD State OF WARENORD Nome Address 90/0 Faster D. Colores Nome Address 90/0 Faster D. Colores Nome CHRLADN SE Not State OF MARENORD ED USE: Dometic by Industry D. Municipal IP WORK: Other of the Well Other FWORK: Other of the Well Other Perpended Reconditioned or and the kind and the red of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mode of the mod		·	
(7) PIMP. GRUDE	OS INTELO			
Manuf Children Con Con Con			_	
(8) WATER LEVELS: Land-surface elevation				
Artesian pressure	ate	ENT OF ECOLOGY		
Artesian water is controlled by	valve, etc.)	- 8201011 UF 904		
(9) WELL TESTS: Drawdown is amount wa		19	11-20	· · · ·
	?			, 19
<u></u>				
······································	This well was	arillea under my jurisdiction f my knowledge and belief.	n and this	report
Recovery data (time taken as zero when pump turned	·····	N	7	
measured from well top to water level)	NAME WWW	UHTER URALLA	(Type or p	rint)
· · · · · · · · · · · · · · · · · · ·	/ ED			-
	Address		<u></u>	
Late of test	retenati IK	the Vila	-	
Baller test	after hread and a start and a start and a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start a start	(Well Officer)		i t
Artesian flow		/2.47 //	'- 27	19.

••••

epartment of Ecology econd Copy — Owner's Copy pird Copy — Driller's Copy		ELL REPO	r	Perm	it No.	
1) OWNER: Name PAUL H	ERRON	Address 9	014 Fos	sterRol. Cast	increwa.	18815
LOCATION OF WELL: County	CHELAN		_SE	SW 1, Sec 22	T.24 N. I	18 W.
earing and distance from section or subdivis						
3) PROPOSED USE: Domestic 🕱	Industrial 🔲 Municipal (1 (10) WEL	L LOG:	<u>_</u>		· · · · ·
		- Example 1	scribe by color	r, character, size of	material and st	ructure, a
		_ i show thickness	s of actuiters as	nd the kind and nat least one entry for	cure of the mate	гланты сан
4) TYPE OF WORK: Owner's num (if more than	1 one)		MATE	RIAL	PROM	-
New well OK Mo Despended	ethod: Dug 📋 Bored Cable 🗋 Driven		GRAVE	L+ CORBLE	5795 0	33
Reconditioned	Rotary X Jetted			<u> </u>		
5) DIMENSIONS: Diameter		GRADE		Rocks	33	<u>> 67</u>
Drilled 64 ft. Depth of cor	of well	•.				
	-					
6) CONSTRUCTION DETAILS:	14 17					
	om +172rt to 63 t					
- -	-om ft. to f rom ft. to f					
<u>_</u>						
Type of perforator used		·				
SIZE of perforations						+
perforations from	ft. to i	t.				
perforations from perforations from						
				•		
Screens: Yes D No						
Manufacturer's Name						<u> </u>
Diam Slot size fi	rom ft. to i	r.				
Diam Slot size fi	rom ft. to	a.				
Gravel packed: Yes 🗆 No 🗶	Size of gravel:					
Gravel placed from	ft. to 1	<u> </u>				
Surface seal: Yes 🖌 No 🗆 _ To	what depth? 18	rt.				
Material used in seal						
Did any strata contain unusable Type of water? I						
Method of sealing strata off			205	MED		
(7) PUMP: Manufacturer's Name U	L LOALDE					<u> </u>
Type: SURMERS	BLE HP YZ	<u>_ }4</u>	<u>.</u>	1986		_ _
(8) WATER LEVELS: Land-surfa-	ce elevation	_ 		1.500		<u> </u>
itatic level 46 tt. below top	of well Date 8-27-5					-
rtesian pressure	are inch Date	3A I AC	ARTMENT O	ON GEHEN		
Artesian water is controlled by	(Cap, valve, etc.)					
(9) WELL TESTS: Drawdown is lowered belo	s amount water level is					
	ow static level es, by whom?	Work started.	8-26		ted 0-A	
	drawdown after h	THEFT T THE	ILLER'S S	STATEMEN'T:		
······································		This wel	l was drilled	l under my jurisc	diction and th	is report
о н			best of my	knowledge and b	ellei.	
Recovery data (time taken as zero when pu measured from well top to water level)		INAME (M	(MU)A7	ER DR	TURNO	INC
Time Water Level Time Water Le	_	•	(Person, fi	irm, or corporation,) (Type or	print)
· · · · · · · · · · · · · · · · · · ·		Address	FAVE	UWORTH,	Was	H
-1		- C	> -+-	DIT		
Date of test		2 (Maned)	Snell	(¥e Drill		المترجية المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد المحمد الم
Artesian fow	drawdown afjinger af in the		้ เว้า	C INCOURT		1979 (A. 1971 - 1971 (A.
remperature of water	· · · · · · · · · · · · · · · · · · ·		<u> </u>	Date		5. 19 ^C

(USE ADDITIONAL SHEETS IF NECESSARY)

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6 1-7392 Application No. File Original and First Copy with Department of Ecology Second Copy — Owner's Copy Third Copy — Driller's Copy WATER WELL REPORT Spring STATE OF WARDINGTON Permit No. llow FlatsLeRL Oralon (rrffith Jank (1) OWNER: Name ine. Address 14 SW 4 Sec / Z TZY N. R / Q WM (*) LOCATION OF WELL: County 4.4. ig and distance from section or subdivision corner (10) WELL LOG: (3) PROPOSED USE: Domestic 📑 Industrial 📄 Municipal 🔂 Irrigation 😰 Test Well 🗆 Other Formation: Describe by color, character, size of material and structure, and show thickness of aquifers and the kind and nature of the material in each stratum penetrated, with at least one entry for each change of formation. п Owner's number of well (if more than one).... (4) TYPE OF WORK: MATERIAL FROM то Method: Dug Bored 📋 New well Journ _ over lay ß 6 Driven D Deepened Cable 🗗 12 ille rock and 6 Jetted Rotary 🗌 Reconditioned gurel 12 33 ted Diameter of well lange (5) **DIMENSIONS**: inches. 35 40 Depth of completed well 41. ... ft.ft. Drilled 40 .4 un und (6) CONSTRUCTION DETAILS: 8 " Diam. from . O. ft. to 2 Casing installed: ft ... ft. to ft. Threaded 🗋 " Diam. from ft. to ft. Welded 🛃 Perforations: Yes D No 🖆 Type of perforator used 20 SIZE of perforations in. by in. perforations from ft. to ft. Screens: Yes D No Manufacturer's Name Type Model No..... Diam. Slot size from ft. to ft. Slot size from ft, to ft. Diam. Gravel packed: Yes 🔲 No 🗹 Size of gravel: ... 18 ace seal: Yes of Non Townat depth? 18 Material used in seal Bintonice Surface seal: Yes 🗹 ft. Did any strata contain unusable water? Yes 📋 No 🖨 Type of water?..... Depth of strata..... Matty Method of sealing strata off. (7) PUMP: Manufacturer's Name НР..... Type: (8) WATER LEVELS: Land-surface elevation above mean sea level..... Static level 22 tt. below top of well Date 4-25-80 1 23 23 Drawdown is amount water level is lowered below static level (9) WELL TESTS: 1000 Work started 4-24 180 Completed 4-25-Was a pump test made? Yes 🗋 No 🚰 If yes, by whom?.... WELL DRILLER'S STATEMENT: ft. drawdown after hrs. Yield: gal/min. with .. •• ... This well was drilled under my jurisdiction and this report is true to the best of my knowledge and belief. Recovery data (time taken as zero when pump turned off) (water level measured from well top to water level) NAME UMWales Dulling . (Person, firm, or corporation) (Type or print) Time Water Level | Time Water Level | Time Water Level • Leavenworth with Address RT / Box 133C ate of test Artesian flow _______ g.p.m. Date Temperature of water ______ Was a chemical analysis made? Yes [] No _____ 50 Date. a No DRIF . All La (USE ADDITIONAL SHEETS IF NECESSARY) ECY 050-1-20

WATER WELL REPORT
Original & 1 st copy – Ecology, 2 nd copy – owner, 3 rd copy – driller
DEPARTMENT OF
ECOLOGY Construction/Decommission ("x" in circle)
\boxtimes Construction 417429
Decommission ORIGINAL INSTALLATION
Notice of Intent Number
PROPOSED USE: Image: Domestic Industrial Image: Municipal Image: DeWater Image: Image: Image: Domestic and the second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second seco
TYPE OF WORK: Owner's number of well (if more than one) Image: New well Reconditioned Method : Dug Bored Driven
Deepened
DIMENSIONS: Diameter of well 6 inches, drilled 259 ft.
Depth of completed well 259ft.
CONSTRUCTION DETAILS
Casing \boxtimes Welded $\underline{6}^{"}$ Diam. from $\underline{+1 \ 1/2}$ ft. to $\underline{46 \ 1/2}$ ft.Installed: \boxtimes Liner installed $\underline{4}^{"}$ Diam. from $\underline{-25}$ ft. to $\underline{259}$ ft.
Threaded Diam. From ft. to ft.
Perforations: 🛛 Yes 🗌 No
Type of perforator used Saw cut & rotary mills knife***
SIZE of perfs <u>1/8</u> in, by <u>9</u> in, and no. of perfs <u>38</u> from <u>225</u> ft. to <u>259</u> ft.
Screens: Yes X No K-Pac Location
Manufacturer's Name
Type Model No
Diam. Slot size from ft. to ft. Diam. Slot size from ft. to ft.
Gravel/Filter packed: Yes X No Size of gravel/sand Materials placed from ft. to ft.
Surface Seal: Xes D No To what depth? <u>19</u> ft.
Material used in seal Bentonite
Did any strata contain unusable water?
Type of water? Depth of strata
Method of sealing strata off
PUMP: Manufacturer's Name
Туре: Н.Р
WATER LEVELS: Land-surface elevation above mean sea level 1018 ft.
Static level 64 ft. below top of well Date 06-06-11
Artesian pressure lbs. per square inch Date
Artesian water is controlled by (cap, valve, etc.)
WELL TESTS: Drawdown is amount water level is lowered below static level
Was a pump test made? Ves No if yes, by whom?
Yield:gal /min. withft. drawdown after hrs.
Yield:gal./min. withft. drawdown afterhrs.
Yield:gal./min. withft. drawdown afterhrs.
Recovery data (time taken as zero when pump turned off) (water level measured from well top to water level)
Time Water Level Time Water Level Time Water Level
Date of test
Bailer testgal./min. withft. drawdown afterhrs.
Airtest <u>37</u> gal./min. with stem set at <u>258</u> ft. for <u>1</u> hrs.
Artesian flow g p.m. Date 06-06-11
Temperature of water Was a chemical analysis made? Types 🕅 No

CURRENT

Notice of Intent No. <u>W 301427</u>
Unique Ecology Well ID Tag No. <u>BCF 501</u>
Water Right Permit No
Property Owner Name Pearmont Inc.
Well Street Address 8343 Pine Flats Loop Rd.
City Cashmere County Chelan
Location SWI/4-1/4 NW1/4 Sec 35 Twn 24 R 18 EWM ⊠ (s, t, r Still REQUIRED) Or Or WM □
Lat/Long Lat Deg N47 Lat Min/Sec 32.059

Lat/Long	Lai Deg	<u>IN 47</u>	Lat will/Sec	<u>32.059</u>
	Long Deg	5 <u>W 120</u>	Long Min/Se	c <u>33.419</u>
Tax Parcel No.	(Required	l) <u>24183</u>	5 230 050	

CONSTRUCTION OF Formation: Describe by color, charact nature of the material in each stratum	er, size of material and penetrated, with at leas	structure, and t t one entry for	the kind and
of information. (USE ADDITIONAL	SHEETS IF NECESS.		······································
MATERIAL		FROM	TO
Brown clay		0	8
Brown clay, gravel, rocks		8	13
Brown sand		13	16
Brown sandy clay, cobbles	· · · ·	16	29
Brown sand, gravel		29	35
Brown sandy clay, gravel,			
rocks WB		35	46
Brown sandstone		46	53/
Gray soft sandstone		53	93
Black shale		93	97
Gray sandstone w/ shale		97	123
Lt. gray sandstone		123	151
Gray sandstone / shale		151	160
Shale		160	172
Lt. gray sandstone		172	202
Black shale		202	205
Lt. gray sandstone		205	214
Shale		214	216
Gray sandstone		216	248
1'5+ gpm @ 235-238'			1
Crumbly black shale WB	· · · · · · · · · · · · · · · · · · ·	248	252
Gray sandstone	· · · · · · · · · · · · · · · · · · ·	252	259
	···· - · · ·		
*** Casing perferated @			+
38-45' w/ static of 36 =3 gpm			
Cascading after finding water			
down at 235-252'. Placed	RECE	IVED	1
shale packer on PVC liner at		1	
65' with bentonite to	JUN 2	4 2014	
separate the two waters.		•	
	DEPARTMENT OF ECOLOGY - C	NTDAL DEGLOV	
		HERIONAL OF	FICE
Start Date 06-03-11	_ Completed Da	te <u>06-06-1</u>	1

WELL CONSTRUCTION CERTIFICATION: I constructed and/or accept responsibility for construction of this well, and its compliance with all Washington well construction standards. Materials used and the information reported above are true to my best knowledge and belief.

Driller Engineer Trainee Name (Print) Brett Phythian	Drilling Company Tumwater Drilling & Pump	Inc.
Driller/Engineer/Trainee Signature	Address P.O.Box 777 / 9290 Hwy 2	
Driller or trainee License No. 1249	City, State, Zip Leavenworth	, WA, 98826
IF TRAINEE: Driller's License No:	Contractor's	
Driller's Signature:	Registration No. TUMWADIP011LZ	Date <u>06-07-2011</u>

ECY 050-1-20 (Rev 02/10) If you need this document in an alternate format, please call the Water Resources Program at 360-407-6872. Persons with hearing loss can call 711 for Washington Relay Service. Persons with a speech disability can call 877-833-634/.

(1)	OWNER: Mane TALLEY, ROBERT D.		O BOY 202			r Right Per	ACISSS THE NO.	2222
101			0.BOX 302 - SE 1	/4_SW 1/-		r 24 N. P		CZQ:
31	PROPOSED USE: DOMESTIC	AND AND AND AND AND AND AND AND AND AND	≝LLL XD., DX ≈======	YD KA ====================================				\sim
=			(10) WE	LL LOG		╺┑╸╸╸╸╸╸		Ť
4)	TYPE OF WORK: Owner's Number of well (If more than one) Nethod: ROTARY DIMENSIONS:		Pormati and str	on: Describ ucture, and	be by color, I show thickn	character,	Size of mate	erial
'	Deillad 640 at Didmeter OF Well	6 inche	2]	ire of the one entry	material in for each ch	each strati ange in foi	size of mata ifers and the mation.	d, vi
6)	CONSTRUCTION DETAILS: Casing installed: 6 * Dia. from +1.5 ft. WELDED W/LINER 4 * Dia. from -99 ft. Dia. from ft.	171223333888	BROWN SI BROWN CI GRAY CL BROWN CI	YA TIDA CIYA			PROM 0 3 25 63 57 71 77	326
	Perforations: YES Type of perforator used SKILL SAW SIZE of perforations .125 in. by 7 27 perforations from 469 ft. to 489 ft 27 perforations from 569 ft. to 589 ft 72 perforations from 609 ft. to 649 ft Screens: NO		BROWN CT	AY DSTONE Ay DSTONE (LA) ASALT Y SHALE	Y LAYER (S)		88 92 104 172	74 77 88 92 10 17 18
	Manufacturer's Name Type Nodel No. Diam. slot size from ft. to Diam. slot size from ft. to	ft.	GKAY SAN	D GRAY SAM DSTONE	ASTONE SEALE		180 186 211 237 242 261 297 303 313	18 21 23 24 26 29 30
٥	Gravel packed: NO Size of gravel Gravel placed from ft. to ft. Surface seal: YES To what depth Naterial used in seal BENTONITE Did any strata contain unusable water? NO Type of water? Depth of strat Nethod of sealing strata off SEAL METHOD 1	? 20 ft.	GRAY SAM SHALE GRAY SAM SHALE GRAY SAM	ISTORE	DEPARTMEN CENTRAL RI	0 1997 T OF ECOLOGY EGION OFFICE	318 334 350 390 417 454	31 31 33 35 35 39 41 45 45 48
P	ONP: Manufacturer's Name GRUNFOS Type SUBMERSIBLE H.I	2. 1.5	= SHALE GRAY SAND SHALE GRA	STORE			460 489 496	480 496 519 536
SI Al	tatic level 129 ft. below top of well ha	# 6	SHALE WAT GRAY SAND	STONE ER BRARING STONE SHALL			519 536 562 573 NAL FORMATIO	562 573 592
===			Work star	ed 11/11/9	7	Completed	1 11/14/97	
as iel	CLL TESTS: Drawdown is amount water level is low static level. a pump test made? NO If yes, by whom? d: gal./min with ft. drawdown after wery data me Water Level Time Water Level Time W		and the knowled	ructed and on of this ton well c informati ge and bel	on reported . ief.	esponsibili ts complian standards	ity for con- ice with all Materials u rue to my be	
	Date of test / /	MART DEART		on, firm, (NG, INC. or corporation LEAVENCETH	on) (Type	or print)	
r t	er test gal/min. ft. drawdown after test 5 gal/min. w/ stem set at 648 ft. for sian flow g.p.m. Dat	hrs. 4.5 hrs.	[SIGNED]		TA	Lizense N	0. 1 249	

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WÀTER NELL STATE OF WASHI CONTINUATION S	INGTON	
(1 MA	0) WELL LOG TERIAL SHALE GRAY CLAY GRAY SANDSTONE SHALE BLACK SHALE GRAY SANDSTONE GRAY SANDSTONE SHALE GRAY SANDSTONE	FROM TO 592 603 609 618 620 623 636 636 644 644

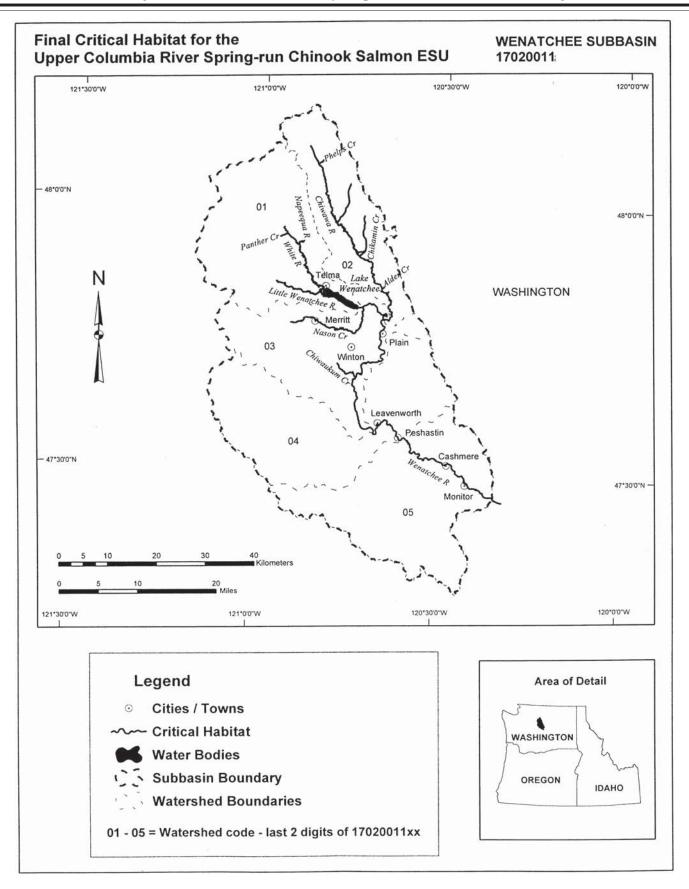
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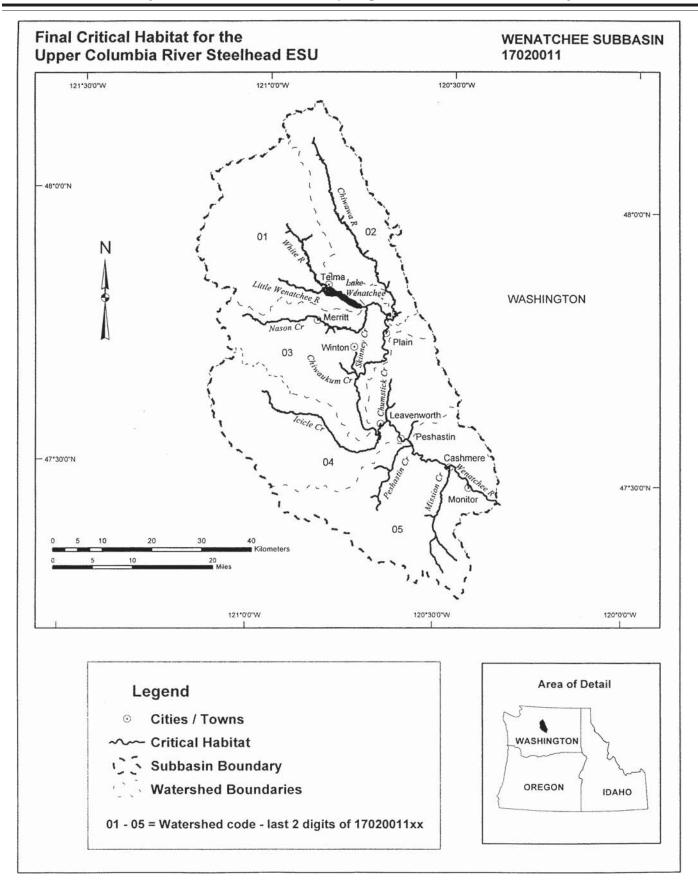
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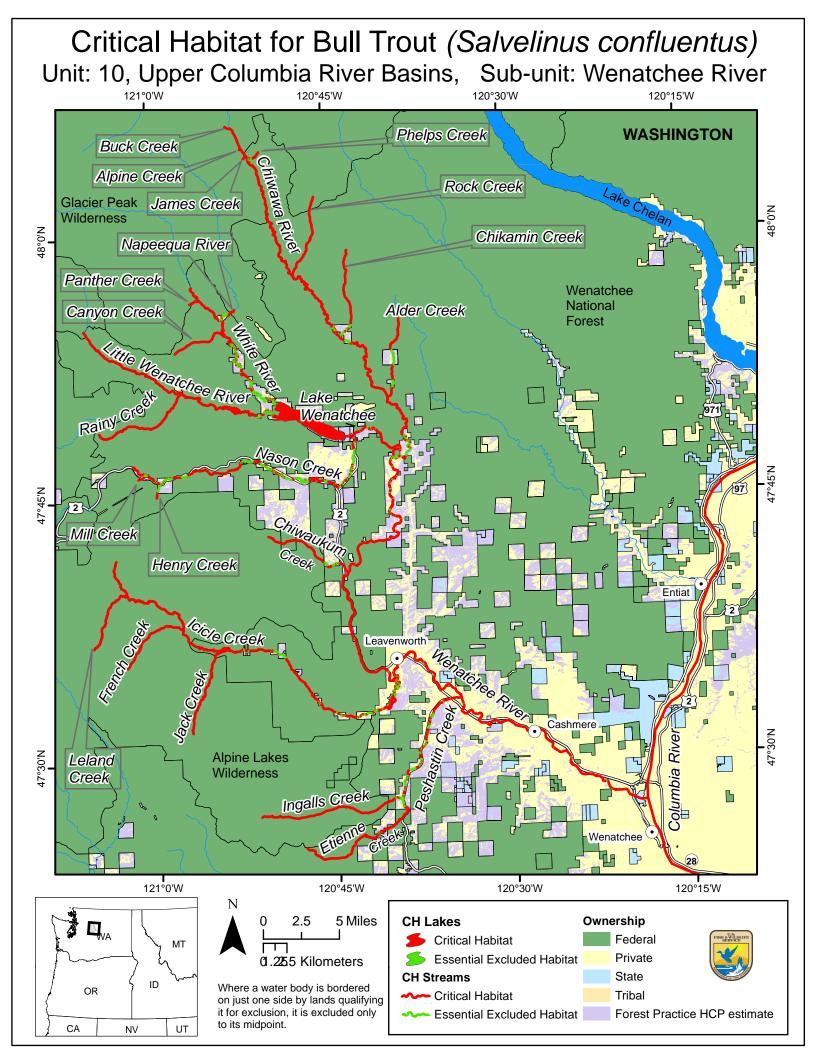
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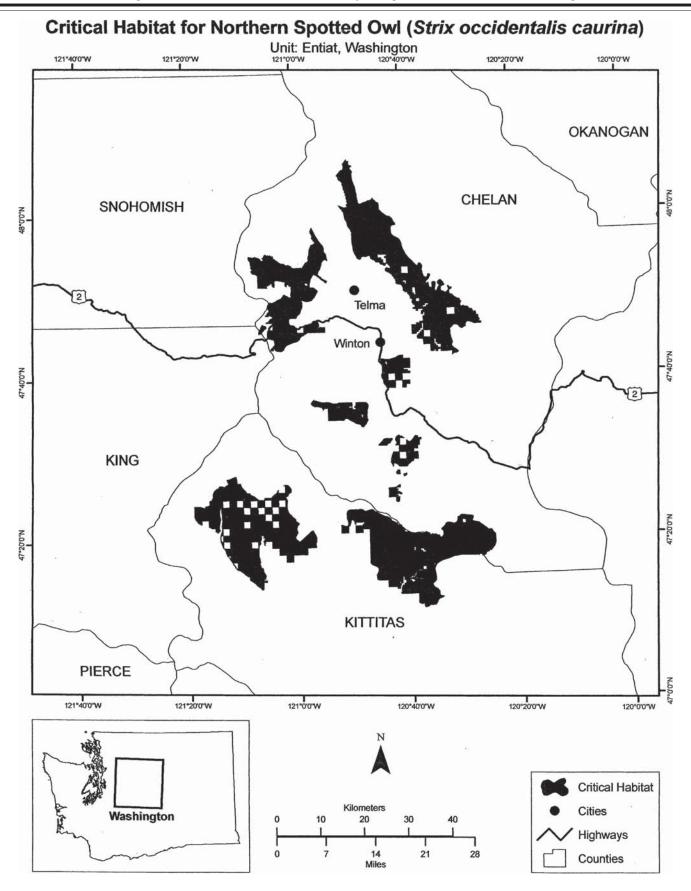
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APPENDIX F ENVIRONMENTAL REVIEW DATA









APPENDIX G OPINIONS OF PROBABLE COST

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Costs Design Flow Rate = 10 CFS

			ALTER	NATIVE 1	ALTER	NATIVE 2	ALTER	NATIVE 3	ALTERNATIVE 4		ALTER	NATIVE 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST
Micellaneous Site Work												
Diversion and care of water	LS	VARIES	1	\$30,000	1	\$30,000	1	\$30,000	1	\$30,000	1	\$30,000
Temporary & permanent access	LS	VARIES	1	\$40,000	1	\$18,000	1	\$30,000	1	\$20,000	1	\$15,000
Erosion and sediment control	LS	VARIES	1	\$8,000	1	\$12,000	1	\$8,000	1	\$14,000	1	\$12,000
Clearing and grubbing	AC	\$3,500	0.9	\$2,989	1.0	\$3,668	0.9	\$3,053	1.8	\$6,248	0.2	\$592
Subtotal - Miscellaneous Site Work				\$80,989		\$63,668		\$71,053		\$70,248		\$57,592
Earthwork												
Excavation and stockpile, soil	CY	\$6.00	902	\$5,412	1,148	\$6,889	1,014	\$6,084	1,797	\$10,783	2,545	\$15,268
Excavation and stockpile, rock	CY	\$15.00	601	\$9,021	765	\$11,482	676	\$10,140	1,198	\$17,972	1,696	\$25,446
Backfill (imported material)	CY	\$30.00	383	\$11,504	548	\$16,451	458	\$13,752	983	3 \$29,493	1,484	\$44,514
Backfill (native material)	CY	\$8.00	581	\$4,647	774	\$6,196	669	\$5,351	1,285	\$10,277	1,872	\$14,978
Waste/Disposal of excess material	CY	\$5.00	923	\$4,613	1,139	\$5,696	1,021	\$5,105	1,711	\$8,553	2,369	\$11,844
Subtotal - Earthwork				\$35,197		\$46,714		\$40,432		\$77,079		\$112,050
Pump Station and Intake Facility												
Reinforced Concrete Structure	LS	\$60,000	1	\$60,000	1	\$60,000	1	\$60,000	1	\$60,000	1	\$60,000
Steel Supports for Screen	LS	\$25,000	1	\$25,000	1	\$25,000	1	\$25,000	1	\$25,000	1	\$25,000
Wedge Wire or Profile Bar Screen	SF	\$300	32	\$9,600	32	\$9,600	32	\$9,600	32	\$9,600	32	\$9,600
Air Blast or Other Screen Self-Cleaning System	LS	\$90,000	1	\$90,000	1	\$90,000	1	\$90,000	1	\$90,000	1	\$90,000
Vertical Turbine Pumps - 175-200 HP, VFD	EA	\$140,000	2	\$280,000	2	\$280,000	2	\$280,000	2	\$280,000	2	\$280,000
3-Phase Power Extension	LS	VARIES	1	\$110,000	1	\$50,000	1	\$70,000	1	\$50,000	1	\$25,000
Electrical and Controls	LS	\$110,000	1	\$110,000	1	\$110,000	1	\$110,000	1	\$110,000	1	\$110,000
Piping, Valves, Meter and Appurtenances	LS	\$40,000	1	\$40,000	1	\$40,000	1	\$40,000	1	\$40,000	1	\$40,000
Subtotal - Pump Station and Intake Facility				\$724,600		\$664,600		\$684,600		\$664,600		\$639,600
Delivery Pipeline												
20" CL 52 D.I. Pipe	LF	\$115.00	1,391	\$159,965	2,607	\$299,805	1,654	\$190,210	3,086	\$354,890	4,876	\$560,740
Fittings and Appurtenances	LF	\$15.00	1,391	\$20,865	2,607	\$39,105	1,654	\$24,810	3,086	\$46,290	4,876	\$73,140
U.S. Highway 2 Crossing	LS	VARIES	0	\$0	1	\$50,000	0	\$0	1	\$300,000	1	\$300,000
Subtotal - Delivery Pipeline				\$180,830		\$388,910		\$215,020		\$701,180		\$933,880
Outlet Structure												
Reinforced Concrete Outlet Structure	LS	\$5,000.00	1	\$5,000	1	\$5,000	1	\$5,000	1	\$5,000	1	\$5,000
Subtotal - Outlet Structure				\$5,000		\$5,000		\$5,000		\$5,000		\$5,000
Construction Subtotal				\$1,027,000		\$1,169,000		\$1,016,000		\$1,518,000		\$1,748,000
Mobilization / Demobilization	10.0%			\$102,700		\$116,900		\$101,600		\$151,800		\$174,800
Contingency	30.0%			\$338,910		\$385,770		\$335,280		\$500,940		\$576 <i>,</i> 840
Engineering, Permitting and Administration	20.0%			\$225,940		\$257,180		\$223,520		\$333,960		\$384,560
Sales Tax	8.0%			\$135,564		\$154,308		\$134,112		\$200,376		\$230,736
Allowance for Land Acquisition	LS	\$50,000.00	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000
Total Project Cost				\$1,880,000		\$2,133,000		\$1,861,000		\$2,755,000		\$3,165,000

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Operating Costs Design Flow Rate = 10 CFS

			ALIGN	MENT 1	ALIGN	IMENT 2	ALIGN	IMENT 3	ALIGN	IMENT 4	ALIGN	IMENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST
Annual Operations and Maintenance Cost ¹				\$11,920		\$12,820		\$11,920		\$14,720		\$16,120
Pumping Power Costs ²												
Monthly Basic Charge (3-Phase Power)	/EA/MO	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50
Monthly Demand Charge	/HP/MO	\$3.52	395	\$1,390.40	400	\$1,408.00	387	\$1,362.24	369	\$1,298.88	355	\$1,249.60
Energy Charge												
2-Week Annual Pumping Duration	kWH	\$0.0165	99,009	\$1,633.65	100,262	\$1,654.33	97,004	\$1,600.56	92,492	\$1,526.12	88,983	\$1,468.22
4-Week Annual Pumping Duration	kWH	\$0.0165	198,018	\$3,267.30	200,525	\$3,308.66	194,008	\$3,201.13	184,984	\$3,052.24	177,966	\$2,936.44
6-Week Annual Pumping Duration	kWH	\$0.0165	297,027	\$4,900.95	300,787	\$4,962.99	291,012	\$4,801.69	277,476	\$4,578.36	266,949	\$4,404.65
8-Week Annual Pumping Duration	kWH	\$0.0165	396,036	\$6,534.60	401,050	\$6,617.32	388,015	\$6,402.26	369,968	\$6,104.48	355,932	\$5,872.87
Total Annual Pumping Costs												
2-Week Annual Pumping Duration				\$8,658		\$8,767		\$8,484		\$8,093		\$7,789
4-Week Annual Pumping Duration				\$10,292		\$10,421		\$10,085		\$9,619		\$9,257
6-Week Annual Pumping Duration				\$11,925		\$12,075		\$11,685		\$11,145		\$10,725
8-Week Annual Pumping Duration				\$13,559		\$13,730		\$13,286		\$12,671		\$12,193
Total Annual Operating Costs ³												
2-Week Annual Pumping Duration				\$20,600		\$21,600		\$20,400		\$22,800		\$23,900
4-Week Annual Pumping Duration				\$22,200		\$23,200		\$22,000		\$24,300		\$25,400
6-Week Annual Pumping Duration				\$23,800		\$24,900		\$23,600		\$25,900		\$26,800
8-Week Annual Pumping Duration				\$25,500		\$26,500		\$25,200		\$27,400		\$28,300

Notes:

1) Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance, and accounting, in 2012 dollars.

2) Pumping power costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service), and are applied May through June, in 2012 dollars.

3) Rounded to nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Costs Design Flow Rate = 20 CFS

			ALIGI	NMENT 1	ALIGN	NMENT 2	ALIGN	NMENT 3	ALIGNMENT 4		ALIGN	NMENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST
Micellaneous Site Work												
Diversion and care of water	LS	VARIES	1	\$30,000	1	\$30,000	1	\$30,000	1	\$30,000	1	\$30,000
Temporary & permanent access	LS	VARIES	1	\$42,000	1	\$20,000	1	\$32,000	1	\$22,000	1	\$17,000
Erosion and sediment control	LS	VARIES	1	\$9,000	1	\$13,000	1	\$9,000	1	\$15,000	1	\$13,000
Clearing and grubbing	AC	\$3,500	0.9	\$2,989	1.0	\$3,668	0.9	\$3,053	1.8	\$6,248	0.2	\$592
Subtotal - Miscellaneous Site Work				\$83,989		\$66,668		\$74,053		\$73,248		\$60,592
Earthwork												
Excavation and stockpile, soil	CY	\$6.00	1,279	\$7,674	1,631	\$9,784	1,439	\$8,633	2,558	\$15,348	3,626	\$21,756
Excavation and stockpile, rock	CY	\$15.00	853	\$12,789	1,087	\$16,307	959	\$14,388	1,705	\$25,580	2,417	\$36,259
Backfill (imported material)	CY	\$30.00	549	\$16,464	784	\$23,507	656	\$19,665	1,403		2,115	\$63,464
Backfill (native material)	CY	\$8.00	738	\$5,906	976	\$7,808	846	\$6,770	1,602	\$12,820	2,324	\$18,593
Waste/Disposal of excess material	CY	\$5.00	1,393	\$6,966	1,742	\$8,709	1,552	\$7,759	2,661	\$13,304	3,719	\$18,596
Subtotal - Earthwork				\$49,799		\$66,115		\$57,216	·	\$109,128	· · ·	\$158,668
Pump Station and Intake Facility												
Reinforced Concrete Structure	LS	\$80,000	1	\$80,000	1	\$80,000	1	\$80,000	1	\$80,000	1	\$80,000
Steel Supports for Screen	LS	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000
Wedge Wire or Profile Bar Screen	SF	\$300	64	\$19,200	64	\$19,200	64	\$19,200	64	\$19,200	64	\$19,200
Air Blast or Other Screen Self-Cleaning System	LS	\$115,000	1	\$115,000	1	\$115,000	1	\$115,000	1	\$115,000	1	\$115,000
Vertical Turbine Pumps - 220-260 HP, VFD	EA	\$160,000	3	\$480,000	3	\$480,000	3	\$480,000	3	\$480,000	3	\$480,000
3-Phase Power Extension	LS	VARIES	1	\$130,000	1	\$60,000	1	\$80,000	1	\$60,000	1	\$30,000
Electrical and Controls	LS	\$160,000	1	\$160,000	1	\$160,000	1	\$160,000	1	\$160,000	1	\$160,000
Piping, Valves, Meter and Appurtenances	LS	\$60,000	1	\$60,000	1	\$60,000	1	\$60,000	1	\$60,000	1	\$60,000
Subtotal - Pump Station and Intake Facility				\$1,094,200		\$1,024,200		\$1,044,200		\$1,024,200		\$994,200
Delivery Pipeline												
30" CL 52 D.I. Pipe	LF	\$160.00	1,391	\$222,560	2,607	\$417,120	1,654	\$264,640	3,086	\$493,760	4,876	\$780,160
Fittings and Appurtenances	LF	\$17.00	1,391	\$23,647	2,607	\$44,319	1,654	\$28,118	3,086	\$52,462	4,876	\$82,892
U.S. Highway 2 Crossing	LS	VARIES	0	\$0	1	\$60,000	0	\$0	1	\$360,000	1	\$360,000
Subtotal - Delivery Pipeline				\$246,207		\$521,439		\$292,758		\$906,222		\$1,223,052
Outlet Structure												
Reinforced Concrete Outlet Structure	LS	\$6,000.00	1	\$6,000	1	\$6,000	1	\$6,000	1	\$6,000	1	\$6,000
Subtotal - Outlet Structure				\$6,000		\$6,000		\$6,000		\$6,000		\$6,000
Construction Subtotal				\$1,480,000		\$1,684,000		\$1,474,000		\$2,119,000		\$2,443,000
Mobilization / Demobilization	10.0%			\$148,000		\$168,400		\$147,400		\$211,900		\$244,300
Contingency	30.0%			\$488,400		\$555,720		\$486,420		\$699,270		\$806,190
Engineering, Permitting and Administration	20.0%			\$325,600		\$370,480		\$324,280		\$466,180		\$537,460
Sales Tax	8.0%			\$195,360		\$222,288		\$194,568		\$279,708		\$322,476
Allowance for Land Acquisition	LS	\$50,000.00	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000
Total Project Cost				\$2,687,000		\$3,051,000		\$2,677,000		\$3,826,000		\$4,403,000

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Operating Costs Design Flow Rate = 20 CFS

			ALIG	NMENT 1	ALIGN	IMENT 2	ALIGNMENT 3		ALIGN	ALIGNMENT 4		NMENT 5	
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST	
Annual Operations and Maintenance Cost ¹				\$14,220		\$15,320		\$14,120		\$17,620		\$19,320	
Pumping Power Costs ²													
Monthly Basic Charge (3-Phase Power)	/EA/MO	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	
Monthly Demand Charge	/HP/MO	\$3.52	780	\$2,745.60	786	\$2,766.72	761	\$2,678.72	716	\$2,520.32	680	\$2,393.60	
Energy Charge													
2-Week Annual Pumping Duration	kWH	\$0.0165	195,512	\$3,225.94	197,016	\$3,250.76	190,749	\$3,147.36	179,470	\$2,961.25	170,446	\$2,812.36	
4-Week Annual Pumping Duration	kWH	\$0.0165	391,023	\$6,451.89	394,031	\$6,501.52	381,498	\$6,294.72	358,939	\$5,922.50	340,892	\$5,624.72	
6-Week Annual Pumping Duration	kWH	\$0.0165	586,535	\$9,677.83	591,047	\$9,752.27	572,248	\$9,442.09	538,409	\$8,883.75	511,338	\$8,437.08	
8-Week Annual Pumping Duration	kWH	\$0.0165	782,047	\$12,903.77	788,062	\$13,003.03	762,997	\$12,589.45	717,879	\$11,845.00	681,784	\$11,249.44	
Total Annual Pumping Costs													
2-Week Annual Pumping Duration				\$17,026		\$17,157		\$16,613		\$15,635		\$14,853	
4-Week Annual Pumping Duration				\$20,252		\$20,408		\$19,761		\$18,597		\$17,665	
6-Week Annual Pumping Duration				\$23,478		\$23,658		\$22,908		\$21,558		\$20,478	
8-Week Annual Pumping Duration				\$26,704		\$26,909		\$26,056		\$24,519		\$23,290	
Total Annual Operating Costs ³													
2-Week Annual Pumping Duration				\$31,200		\$32,500		\$30,700		\$33,300		\$34,200	
4-Week Annual Pumping Duration				\$34,500		\$35,700		\$33,900		\$36,200		\$37,000	
6-Week Annual Pumping Duration				\$37,700		\$39,000		\$37,000		\$39,200		\$39,800	
8-Week Annual Pumping Duration				\$40,900		\$42,200		\$40,200		\$42,100		\$42,600	

Notes:

1) Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance, and accounting, in 2012 dollars.

2) Pumping power costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service), and are applied May through June, in 2012 dollars.

3) Rounded to nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Costs Design Flow Rate = 40 CFS

			ALIGN	IMENT 1	ALIGN	IMENT 2	ALIGN	IMENT 3	ALIGN	MENT 4	ALIGN	IMENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST	QTY	COST
Micellaneous Site Work												
Diversion and care of water	LS	VARIES	1	\$30,000	1	\$30,000	1	\$30,000	1	\$30,000	1	\$30,000
Temporary & permanent access	LS	VARIES	1	\$18,000	1	\$22,000	1	\$18,000	1	\$24,000	1	\$31,000
Erosion and sediment control	LS	VARIES	1	\$12,000	1	\$15,000	1	\$12,000	1	\$18,000	1	\$15,000
Clearing and grubbing	AC	\$3,500	0.9	\$2,989	1.0	\$3,668	0.9	\$3,053	1.8	\$6,248	0.2	\$592
Subtotal - Miscellaneous Site Work				\$62,989		\$70,668		\$63,053		\$78,248		\$76,592
Earthwork												
Excavation and stockpile, soil	CY	\$6.00	1,721	\$10,326	2,146	\$12,873	1,914	\$11,484	3,265	\$19,590	4,554	\$27,326
Excavation and stockpile, rock	CY	\$15.00	1,147	\$17,209	1,430	\$21,456	1,276	\$19,140	2,177	\$32,650	3,036	\$45,543
Backfill (imported material)	CY	\$30.00	667	\$20,010	924	\$27,719	771	\$23,132	1,663		2,514	\$75,424
Backfill (native material)	CY	\$8.00	914	\$7,314	1,179	\$9,429	1,034	\$8,276	1,876	\$15,005	2,678	\$21,428
Waste/Disposal of excess material	CY	\$5.00	1,954	\$9,770	2,397	\$11,986	2,155	\$10,777	3,566	\$17,830	4,912	\$24,560
Subtotal - Earthwork				\$64,629		\$83,464		\$72,808		\$134,965		\$194,280
Pump Station and Intake Facility												
Reinforced Concrete Structure	LS	\$120,000	1	\$120,000	1	\$120,000	1	\$120,000	1	\$120,000	1	\$120,000
Steel Supports for Screen	LS	\$95,000	1	\$95,000	1	\$95,000	1	\$95,000	1	\$95,000	1	\$95,000
Wedge Wire or Profile Bar Screen	SF	\$300	128	\$38,400	128	\$38,400	128	\$38,400	128	\$38,400	128	\$38,400
Air Blast or Other Screen Self-Cleaning System	LS	\$185,000	1	\$185,000	1	\$185,000	1	\$185,000	1	\$185,000	1	\$185,000
Vertical Turbine Pumps - 455-535 HP, VFD	EA	\$250,000	3	\$750,000	3	\$750,000	3	\$750,000	3	\$750,000	3	\$750,000
3-Phase Power Extension	LS	VARIES	1	\$150,000	1	\$70,000	1	\$120,000	1	\$70,000	1	\$40,000
Electrical and Controls	LS	\$250,000	1	\$250,000	1	\$250,000	1	\$250,000	1	\$250,000	1	\$250,000
Piping, Valves, Meter and Appurtenances	LS	\$110,000	1	\$110,000	1	\$110,000	1	\$110,000	1	\$110,000	1	\$110,000
Subtotal - Pump Station and Intake Facility				\$1,698,400		\$1,618,400		\$1,668,400		\$1,618,400		\$1,588,400
Delivery Pipeline												
36" CL 52 D.I. Pipe	LF	\$210.00	1,391	\$292,110	2,607	\$547,470	1,654	\$347,340	3,086	\$648,060	4,876	\$1,023,960
Fittings and Appurtenances	LF	\$23.00	1,391	\$31,993	2,607	\$59,961	1,654	\$38,042	3,086	\$70,978	4,876	\$112,148
U.S. Highway 2 Crossing	LS	VARIES	0	\$0	1	\$70,000	0	\$0	1	\$420,000	1	\$420,000
Subtotal - Delivery Pipeline				\$324,103		\$677,431		\$385,382		\$1,139,038		\$1,556,108
Outlet Structure												
Reinforced Concrete Outlet Structure	LS	\$10,000.00	1	\$10,000	1	\$10,000	1	\$10,000	1	\$10,000	1	\$10,000
Subtotal - Outlet Structure				\$10,000		\$10,000		\$10,000		\$10,000		\$10,000
Construction Subtotal				\$2,160,000		\$2,460,000		\$2,200,000		\$2,981,000		\$3,425,000
Mobilization / Demobilization	10.0%			\$216,000		\$246,000		\$220,000		\$298,100		\$342,500
Contingency	30.0%			\$712,800		\$811,800		\$726,000		\$983,730		\$1,130,250
Engineering, Permitting and Administration	20.0%			\$475,200		\$541,200		\$484,000		\$655 <i>,</i> 820		\$753,500
Sales Tax	8.0%			\$285,120		\$324,720		\$290,400		\$393,492		\$452,100
Allowance for Land Acquisition	LS	\$50,000.00	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000	1	\$50,000
Total Project Cost				\$3,899,000		\$4,434,000		\$3,970,000		\$5,362,000		\$6,153,000

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Operating Costs Design Flow Rate = 40 CFS

	ALIGNMENT 1 ALIGNMENT 2				IMENT 2	ALIGN	IMENT 3	ALIGNMENT 4		ALIGN	MENT 5		
ITEM	UNIT	IIT UNIT COST QTY COST QTY		QTY	COST	QTY	COST	QTY	COST	QTY	COST		
Annual Operations and Maintenance Cost ¹				\$17,820		\$19,420		\$18,020		\$22,220		\$24,620	
Pumping Power Costs ²													
Monthly Basic Charge (3-Phase Power)	/EA/MO	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	1	\$14.50	
Monthly Demand Charge	/HP/MO	\$3.52	1,617	\$5,691.84	1,633	\$5,748.16	1,583	\$5,572.16	1,501	\$5,283.52	1,437	\$5,058.24	
Energy Charge													
2-Week Annual Pumping Duration	kWH	\$0.0165	405,311	\$6,687.63	409,321	\$6,753.80	396,788	\$6,547.01	376,235	\$6,207.87	360,193	\$5,943.18	
4-Week Annual Pumping Duration	kWH	\$0.0165	810,622	\$13,375.25	818,642	\$13,507.60	793,577	\$13,094.02	752,469	\$12,415.74	720,385	\$11,886.36	
6-Week Annual Pumping Duration	kWH	\$0.0165	1,215,932	\$20,062.88	1,227,964	\$20,261.40 1,190,365		\$19,641.03 1,128,704		\$18,623.62	1,080,578	\$17,829.54	
8-Week Annual Pumping Duration	kWH	\$0.0165	1,621,243	\$26,750.51	1,637,285	\$27,015.20	1,587,154	\$26,188.04	1,504,939	\$24,831.49	1,440,771	\$23,772.72	
Total Annual Pumping Costs													
2-Week Annual Pumping Duration				\$35,219		\$35,567		\$34,480		\$32,698		\$31,307	
4-Week Annual Pumping Duration				\$41,907		\$42,321		\$41,027		\$38,906		\$37,250	
6-Week Annual Pumping Duration				\$48,595		\$49,075		\$47,574		\$45,114		\$43,193	
8-Week Annual Pumping Duration				\$55,282		\$55,829		\$54,121		\$51,322		\$49,136	
Total Annual Operating Costs ³													
2-Week Annual Pumping Duration				\$53,000		\$55,000		\$52,500		\$54,900		\$55,900	
4-Week Annual Pumping Duration				\$59,700		\$61,700		\$59,000		\$61,100		\$61,900	
6-Week Annual Pumping Duration				\$66,400		\$68,500		\$65,600		\$67,300		\$67,800	
8-Week Annual Pumping Duration			\$73			\$75,200	\$75,200			\$73,500		\$73,800	

Notes:

1) Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance, and accounting, in 2012 dollars.

2) Pumping power costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service), and are applied May through June, in 2012 dollars.

3) Rounded to nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 1, Design Flow Rate = 10 CFS

FUTURE COST
60,436
20,872
41,743
52,607
.05,214
10,427
13,043

\$1,928,000

\$1,880,000

REPLACEMENT I	UND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$18,733	
To Replace	50%	After Life of Project	\$37,466	
To Replace	100%	After Life of Project	\$74,933	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$9,929	
To Replace	50%	After Life of Project	\$19,858	
To Replace	100%	After Life of Project	\$39,717	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$20,184	
To Replace	50%	After Life of Project	\$40,368	
To Replace	100%	After Life of Project	\$80,736	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$42,261	
To Replace	50%	After Life of Project	\$84,522	
To Replace	100%	After Life of Project	\$169,043	

Input Cells - Assumed or Given Values
Input Cells - Adjust Using Goal Seek Tool to Make Account
5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:												
(PRESENT VALUE OF LONG-TERM	Replacment											
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL								
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:									
25% Replacement	\$482,000	\$596,000	\$432,908	\$1,510,908								
50% Replacement	\$964,000	\$596,000	\$432,908	\$1,992,908								
100% Replacement	\$1,928,000	\$596,000	\$432,908	\$2,956,908								
Assuming the Pumping Power Costs for a 4-week Annual Operating Duration:												
25% Replacement	\$482,000	\$596,000	\$514,590	\$1,592,590								
50% Replacement	\$964,000	\$596,000	\$514,590	\$2,074,590								
100% Replacement	\$1,928,000	\$596,000	\$514,590	\$3,038,590								
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:									
25% Replacement	\$482,000	\$596,000	\$596,273	\$1,674,273								
50% Replacement	\$964,000	\$596,000	\$596,273	\$2,156,273								
100% Replacement	\$1,928,000	\$596,000	\$596,273	\$3,120,273								
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:									
25% Replacement	\$482,000	\$596,000	\$677,955	\$1,755,955								
50% Replacement	\$964,000	\$596,000	\$677,955	\$2,237,955								
100% Replacement	\$1,928,000	\$596,000	\$677,955	\$3,201,955								

LIFE CYCLE COSTS:

r																						
	Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

Replacement Fund (For Funding Replacement of 25% of System):

 To Replace
 50%
 After Life of Project

 To Replace
 100%
 After Life of Project

	cement of 25% of 5	bystem).																			
Deposits		\$9,929	\$10,227	\$10,534	\$10,850	\$11,175	\$11,511	\$11,856	\$12,212	\$12,578	\$12,955	\$13,344	\$13,744	\$14,157	\$14,581	\$15,019	\$15,469	\$15,933	\$16,411	\$16,904	
Interest		\$0	\$298	\$614	\$948	\$1,302	\$1,676	\$2,072	\$2,490	\$2,931	\$3,396	\$3,887	\$4,404	\$4,948	\$5,521	\$6,124	\$6,758	\$7,425	\$8,126	\$8,862	
End of Year Balance		\$9,929	\$20,454	\$31,602	\$43,400	\$55,877	\$69,064	\$82,992	\$97,693	\$113,202	\$129,554	\$146,784	\$164,932	\$184,037	\$204,139	\$225,282	\$247,510	\$270,869	\$295,406	\$321,172	:
Replacement Fund (For Funding Repla	cement of 50% of S																				
Deposits		\$19,858	\$20,454	\$21,068	\$21,700	\$22,351	\$23,021	\$23,712	\$24,423	\$25,156	\$25,911	\$26,688	\$27,489	\$28,313	\$29,163	\$30,038	\$30,939	\$31,867	\$32,823	\$33,808	
Interest		\$0		\$1,227	\$1,896	\$2,604	\$3,353	\$4,144	\$4,980	\$5,862	\$6,792	\$7,773	\$8,807	\$9,896	\$11,042	\$12,248	\$13,517	\$14,851	\$16,252	\$17,724	
End of Year Balance		\$19,858	\$40,908	\$63,203	\$86,799	\$111,754	\$138,128	\$165,984	\$195,387	\$226,404	\$259,107	\$293,568	\$329,864	\$368,073	\$408,278	\$450,564	\$495,020	\$541,737	\$590,812	\$642,344	:
Replacement Fund (For Funding Repla	cement of 100% of	System):																			
Deposits		\$39,717	\$40,908	\$42,136	\$43,400	\$44,702	\$46,043	\$47,424	\$48,847	\$50,312	\$51,821	\$53,376	\$54,977	\$56,627	\$58,325	\$60,075	\$61,877	\$63,734	\$65,646	\$67,615	
Interest		\$0	\$1,192	\$2,454	\$3,792	\$5,208	\$6,705	\$8,288	\$9,959	\$11,723	\$13,584	\$15,546	\$17,614	\$19,792	\$22,084	\$24,497	\$27,034	\$29,701	\$32,504	\$35,449	
End of Year Balance		\$39,717	\$81,817	\$126,407	\$173,598	\$223,508	\$276,256	\$331,968	\$390,773	\$452,808	\$518,214	\$587,137	\$659,728	\$736,147	\$816,556	\$901,128	\$990,040	\$1,083,475	\$1,181,625	\$1,284,689	\$1
Operations and Maintenance Expense	s:																				
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$5,800	\$5,974	\$6,153	\$6,338	\$6,528	\$6,724	\$6,926	\$7,133	\$7,347	\$7,568	\$7,795	\$8,029	\$8,269	\$8,517	\$8,773	\$9,036	\$9,307	\$9,587	\$9,874	
Administration, Insurance, Accountin	ng	\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
Total O&M Expenses		\$11,920	\$12,278	\$12,646	\$13,025	\$13,416	\$13,819	\$14,233	\$14,660	\$15,100	\$15,553	\$16,019	\$16,500	\$16,995	\$17,505	\$18,030	\$18,571	\$19,128	\$19,702	\$20,293	
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$8,658	\$8,918	\$9,185	\$9,461	\$9,745	\$10,037	\$10,338	\$10,648	\$10,968	\$11,297	\$11,636	\$11,985	\$12,344	\$12,715	\$13,096	\$13,489	\$13,894	\$14,311	\$14,740	
4-Week Annual Pumping Duration ⁸		\$10,292	\$10,601	\$10,919	\$11,246	\$11,584	\$11,931	\$12,289	\$12,658	\$13,037	\$13,428	\$13,831	\$14,246	\$14,674	\$15,114	\$15,567	\$16,034	\$16,515	\$17,011	\$17,521	
6-Week Annual Pumping Duration ⁸		\$11,925	\$12,283	\$12,652	\$13,031	\$13,422	\$13,825	\$14,240	\$14,667	\$15,107	\$15,560	\$16,027	\$16,508	\$17,003	\$17,513	\$18,038	\$18,579	\$19,137	\$19,711	\$20,302	
8-Week Annual Pumping Duration ⁸		\$13,559	\$13,966	\$14,385	\$14,816	\$15,261	\$15,719	\$16,190	\$16,676	\$17,176	\$17,692	\$18,222	\$18,769	\$19,332	\$19,912	\$20,509	\$21,125	\$21,758	\$22,411	\$23,083	
		,	,	. ,														. ,	. ,		
Veer			2	2		-	(-			10	11	12	12	14	15	16	17	10	10	
Year	U	1	2	5	4	5	o	'	ō	Э	10	11	12	13	14	15	10	1/	19	19	

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$4,226,085

\$8,452,171

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

8) Assumes pumping power costs, or power rates, increase at the assumed rate of inflation.

nt Balance at end of

20	21	22	23
\$17,411	\$17,933	\$18,471	\$19,025
\$9,635	\$10,447	\$11,298	\$12,191
\$348,218	\$376,598	\$406,367	\$437,584
7340,210	<i>\$370,350</i>	<i>Q</i> 400 , 30 ,	<i>4437,</i> 304
\$34,822	\$35,866	\$36,942	\$38,051
\$19,270	\$20,893	\$22,596	\$24,382
\$696,436	\$753,196	\$812,734	\$875,167
\$69,644	\$71,733	\$73,885	\$76,101
\$38,541	\$41,786	\$45,192	\$48,764
\$1,392,873	\$1,506,392	\$1,625,469	\$1,750,334
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$10,170	\$10,475	\$10,790	\$11,113
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$20,902	\$21,529	\$22,175	\$22,840
\$15,182	\$15,638	\$16,107	\$16,590
\$18,047	\$18,588	\$19,146	\$19,720
\$20,911	\$21,539	\$22,185	\$22,850
\$23,776	\$24,489	\$25,224	\$25,981
20	21	22	23

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
\$19,596	\$20,184	\$20,790	\$21,413	\$22,056	\$22,717	\$23,399	\$24,101	\$24,824	\$25,569	\$26,336	\$27,126	\$27,939	\$28,778	\$29,641	\$30,530	\$31,446	\$32,389	\$33,361	\$34,362	\$35,393	\$36,455	\$37,548	\$38,675	\$39,835	\$41,030	\$42,261
\$13,128	\$14,109	\$15,138	\$16,216	\$17,345	\$18,527	\$19,764	\$21,059	\$22,414	\$23,831	\$25,313	\$26,862	\$28,482	\$30,175	\$31,943	\$33,791	\$35,720	\$37,735	\$39,839	\$42,035	\$44,327	\$46,718	\$49,214	\$51,817	\$54,531	\$57,362	\$60,314
\$470,307	\$504,600	\$540,528	\$578,157	\$617,557	\$658,801	\$701,964	\$747,124	\$794,362	\$843,761	\$895,409	\$949,397	\$1,005,819	\$1,064,771	\$1,126,355	\$1,190,675	\$1,257,842	\$1,327,966	\$1,401,167	\$1,477,563	\$1,557,283	\$1,640,456	\$1,727,218	\$1,817,709	\$1,912,076	\$2,010,468	\$2,113,043
\$39,192	\$40,368	\$41,579	\$42,826	\$44,111	\$45,435	\$46,798	\$48,202	\$49,648	\$51,137	\$52,671	\$54,251	\$55,879	\$57,555	\$59,282	\$61,060	\$62,892	\$64,779	\$66,722	\$68,724	\$70,786	\$72,909	\$75,096	\$77,349	\$79,670	\$82,060	\$84,522
\$26,255	\$28,218	\$30,276	\$32,432	\$34,689	\$37,053	\$39,528	\$42,118	\$44,827	\$47,662	\$50,626	\$53,725	\$56,964	\$60,349	\$63,886	\$67,581	\$71,441	\$75,471	\$79,678	\$84,070	\$88,654	\$93,437	\$98,427	\$103,633	\$109,063	\$114,725	\$120,628
\$940,614	\$1,009,201	\$1,081,056	\$1,156,314	\$1,235,115	\$1,317,603	\$1,403,929	\$1,494,248	\$1,588,723	\$1,687,522	\$1,790,819	\$1,898,794	\$2,011,637	\$2,129,541	\$2,252,709	\$2,381,351	\$2,515,684	\$2,655,933	\$2,802,333	\$2,955,127	\$3,114,566	\$3,280,913	\$3,454,436	\$3,635,419	\$3,824,151	\$4,020,936	\$4,226,085
\$78,385	\$80,736	\$83,158	\$85,653	\$88,222	\$90,869	\$93,595	\$96,403	\$99,295	\$102,274	\$105,342	\$108,503	\$111,758	\$115,110	\$118,564	\$122,121	\$125,784	\$129,558	\$133,444	\$137,448	\$141,571	\$145,818	\$150,193	\$154,699	\$159,340	\$164,120	\$169,043
\$52,510	\$56,437	\$60,552	\$64,863	\$69,379	\$74,107	\$79,056	\$84,236	\$89,655	\$95,323	\$101,251	\$107,449	\$113,928	\$120,698	\$127,772	\$135,163	\$142,881	\$150,941	\$159,356	\$168,140	\$177,308	\$186,874	\$196,855	\$207,266	\$218,125	\$229,449	\$241,256
\$1,881,229	\$2,018,402	\$2,162,112	\$2,312,628	\$2,470,230	\$2,635,206	\$2,807,857	\$2,988,496	\$3,177,446	\$3,375,044	\$3,581,637	\$3,797,589	\$4,023,274	\$4,259,083	\$4,505,419	\$4,762,702	\$5,031,367	\$5,311,866	\$5,604,666	\$5,910,254	\$6,229,133	\$6,561,825	\$6,908,873	\$7,270,838	\$7,648,302	\$8,041,871	\$8,452,171
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1,948	\$2,006	\$2,066	\$2,128
\$11,447	\$11,790	\$12,144	\$12,508	\$12,883	\$13,270	\$13,668	\$14,078	\$14,500	\$14,935	\$15,384	\$15,845	\$16,320	\$16,810	\$17,314	\$17,834	\$18,369	\$18,920	\$19,487	\$20,072	\$20,674	\$21,294	\$21,933	\$22,591	\$23,269	\$23,967	\$24,686
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
\$23,525	\$24,231	\$24,958	\$25,707	\$26,478	\$27,272	\$28,090	\$28,933	\$29,801	\$30,695	\$31,616	\$32,564	\$33,541	\$34,547	\$35,584	\$36,651	\$37,751	\$38,883	\$40,050	\$41,251	\$42,489	\$43,764	\$45,077	\$46,429	\$47,822	\$49,256	\$50,734
\$17,088	\$17,600	\$18,128	\$18,672	\$19,232	\$19,809	\$20,403	\$21,016	\$21,646	\$22,295	\$22,964	\$23,653	\$24,363	\$25,094	\$25,847	\$26,622	\$27,421	\$28,243	\$29,091	\$29,963	\$30,862	\$31,788	\$32,742	\$33,724	\$34,736	\$35,778	\$36,851
\$20,312	\$20,921	\$21,549	\$22,195	\$22,861	\$23,547	\$24,253	\$24,981	\$25,730	\$26,502	\$27,297	\$28,116	\$28,960	\$29,829	\$30,723	\$31,645	\$32,594	\$33,572	\$34,579	\$35,617	\$36,685	\$37,786	\$38,919	\$40,087	\$41,290	\$42,528	\$43,804
\$23,536	\$24,242	\$24,969	\$25,718	\$26,490	\$27,285	\$28,103	\$28,946	\$29,815	\$30,709	\$31,630	\$32,579	\$33,557	\$34,563	\$35,600	\$36,668	\$37,768	\$38,901	\$40,068	\$41,270	\$42,508	\$43,784	\$45,097	\$46,450	\$47,844	\$49,279	\$50,757
\$26,760	\$27,563	\$28,390	\$29,241	\$30,119	\$31,022	\$31,953	\$32,911	\$33,899	\$34,916	\$35,963	\$37,042	\$38,153	\$39,298	\$40,477	\$41,691	\$42,942	\$44,230	\$45,557	\$46,924	\$48,332	\$49,782	\$51,275	\$52,813	\$54,398	\$56,030	\$57,711
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 2, Design Flow Rate = 10 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$2,133,000 Total Capital	Cost
Interest on Repla	acement Fu	und:	3.00%	
Rate of Inflation:			3.00%	
Project Design Li	fe:		50 Years	
		00070		3
SUMMARY REPL	ACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Project	t Replacen	nent Cost:		
To Replace	25%	After Life of Project		\$2,337,718
To Replace	50%	After Life of Project		\$4,675,436
To Replace	100%	After Life of Project	\$2,133,000	\$9,350,872
Disposal and Rer	noval Cost	:		
To Replace	25%	After Life of Project		\$61,375
To Replace	50%	After Life of Project		\$122,749
To Replace	100%	After Life of Project	\$56,000	\$245,499
Total Replaceme	ent Cost:			
To Replace	25%	After Life of Project		\$2,399,093
To Replace	50%	After Life of Project		\$4,798,185
To Replace	100%	After Life of Project	\$2,189,000	\$9,596,370

\$2,133,000

REPLACEMENT F	UND SUM	MARY										
Annual Deposit F	Required (A	Assume Equal Deposit Made	Each Year):									
To Replace	25%	After Life of Project	\$21,269									
To Replace	50%	After Life of Project	\$42,538									
To Replace	100%	After Life of Project	\$85,077									
Deposit Required	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):									
To Replace 25% After Life of Project \$11,273												
To Replace 50% After Life of Project \$22,547												
To Replace	100%	After Life of Project	\$45,093									
Deposit Required	d at Year 2	5 (Assume Deposits Increase	at the Rate of Inflation):									
To Replace	25%	After Life of Project	\$22,916									
To Replace	50%	After Life of Project	\$45,833									
To Replace	100%	After Life of Project	\$91,666									
Deposit Required at Year 50 (Assume Deposits Increase at the Rate of Inflation):												
To Replace	25%	After Life of Project	\$47,982									
To Replace	50%	After Life of Project	\$95,964									
To Replace	100%	After Life of Project	\$191,927									

input dello Trajust dollig doa		lance / lecounte b	alamee at ena o	
5oth Year Equal to Future Val	ue of Replacem	ent Cost		
TOTAL LONG-TERM COST SUMMARY:				
PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$547,250	\$641,000	\$438,341	\$1,626,591
50% Replacement	\$1,094,500	\$641,000	\$438,341	\$2,173,841
100% Replacement	\$2,189,000	\$641,000	\$438,341	\$3,268,341
Assuming the Pumping Power Costs for a 4-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$547,250	\$641,000	\$521,058	\$1,709,308
50% Replacement	\$1,094,500	\$641,000	\$521,058	\$2,256,558
LOO% Replacement	\$2,189,000	\$641,000	\$521,058	\$3,351,058
Assuming the Pumping Power Costs for a 6-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$547,250	\$641,000	\$603,774	\$1,792,024
50% Replacement	\$1,094,500	\$641,000	\$603,774	\$2,339,274
LOO% Replacement	\$2,189,000	\$641,000	\$603,774	\$3,433,774
Assuming the Pumping Power Costs for an 8-	week Annual O	perating Durat	ion:	
25% Replacement	\$547,250	\$641,000	\$686,491	\$1,874,741
50% Replacement	\$1,094,500	\$641,000	\$686,491	\$2,421,991
100% Replacement	\$2,189,000	\$641,000	\$686,491	\$3,516,491

Input Cells - Assumed or Given Values

LIFE CYCLE COSTS:

Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

Replacement Fund (For Funding Replacement	t of 25% of Sys	,		<u> </u>		640 CO0	<u> </u>	442.464	440.005	<i></i>	44.4 700	445 450	645 CO5	646.072	646 FFF	647.0F2	447 564	640.000	640.000	<i></i>
Deposits		\$11,273	\$11,612	\$11,960	\$12,319	\$12,688	\$13,069	\$13,461	\$13,865	\$14,281	\$14,709	\$15,150	\$15,605	\$16,073	\$16,555	\$17,052	\$17,564 \$7.673	\$18,090	\$18,633	\$19,192
Interest End of Year Balance		\$0 \$11.273	\$338 \$23.223	\$697 \$35.880	\$1,076 \$49.275	\$1,478 \$63,441	\$1,903 \$78,413	\$2,352 \$94.227	\$2,827 \$110,918	\$3,328 \$128,527	\$3,856 \$147,092	\$4,413 \$166,655	\$5,000 \$187,259	\$5,618 \$208,950	\$6,269 \$231,774	\$6,953 \$255,779	\$7,673 \$281,016	\$8,430 \$307.537	\$9,226 \$335,396	\$10,062 \$364,650
		311,273	323,223	\$55,660	343,273	303,441	\$78,415	<i>33</i> 4,227	\$110,918	\$120,527	\$147,0 <u>9</u> 2	\$100,055	Ş167,239	3208,930	3231,774	3233,115	3201,010	3307,337	3333,350	\$304,030
Replacement Fund (For Funding Replacement	t of 50% of Sys	tem):																		
Deposits		\$22,547	\$23,223	\$23,920	\$24,637	\$25,377	\$26,138	\$26,922	\$27,730	\$28,561	\$29,418	\$30,301	\$31,210	\$32,146	\$33,111	\$34,104	\$35,127	\$36,181	\$37,266	\$38,384
Interest		\$0	\$676	\$1,393	\$2,153	\$2,956	\$3,806	\$4,705	\$5,654	\$6,655	\$7,712	\$8,825	\$9,999	\$11,236	\$12,537	\$13,906	\$15,347	\$16,861	\$18,452	\$20,124
End of Year Balance		\$22,547	\$46,446	\$71,759	\$98,550	\$126,883	\$156,827	\$188,454	\$221,837	\$257,053	\$294,183	\$333,310	\$374,519	\$417,901	\$463,548	\$511,559	\$562,032	\$615,074	\$670,793	\$729,301
Replacement Fund (For Funding Replacement	t of 100% of Sy	stem):																		
Deposits		\$45,093	\$46,446	\$47,840	\$49,275	\$50,753	\$52,276	\$53,844	\$55,459	\$57,123	\$58,837	\$60,602	\$62,420	\$64,292	\$66,221	\$68,208	\$70,254	\$72,362	\$74,533	\$76,768
Interest		\$0	\$1,353	\$2,787	\$4,306	\$5,913	\$7,613	\$9,410	\$11,307	\$13,310	\$15,423	\$17,651	\$19,999	\$22,471	\$25,074	\$27,813	\$30,694	\$33,722	\$36,904	\$40,248
End of Year Balance		\$45,093	\$92,892	\$143,519	\$197,099	\$253,765	\$313,654	\$376,907	\$443,674	\$514,107	\$588,367	\$666,619	\$749,038	\$835,801	\$927,096	\$1,023,117	\$1,124,065	\$1,230,148	\$1,341,585	\$1,458,601
Operations and Maintenance Expenses:																				
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851
Maintenance and Small Repairs ⁷		\$6,700	\$6,901	\$7,108	\$7,321	\$7,541	\$7,767	\$8,000	\$8,240	\$8,487	\$8,742	\$9,004	\$9,274	\$9,553	\$9,839	\$10,134	\$10,438	\$10,752	\$11,074	\$11,406
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$12,820	\$13,205	\$13,601	\$14,009	\$14,429	\$14,862	\$15,308	\$15,767	\$16,240	\$16,727	\$17,229	\$17,746	\$18,278	\$18,827	\$19,391	\$19,973	\$20,572	\$21,190	\$21,825
Pumping Power Costs:																				
2-Week Annual Pumping Duration ⁸		\$8,767	\$9,030	\$9,301	\$9,580	\$9,867	\$10,163	\$10,468	\$10,782	\$11,106	\$11,439	\$11,782	\$12,135	\$12,499	\$12,874	\$13,261	\$13,658	\$14,068	\$14,490	\$14,925
4-Week Annual Pumping Duration ⁸		\$10,421	\$10,734	\$11,056	\$11,387	\$11,729	\$12,081	\$12,443	\$12,817	\$13,201	\$13,597	\$14,005	\$14,425	\$14,858	\$15,304	\$15,763	\$16,236	\$16,723	\$17,225	\$17,741
6-Week Annual Pumping Duration ⁸		\$12,075	\$12,438	\$12,811	\$13,195	\$13,591	\$13,999	\$14,419	\$14,851	\$15,297	\$15,756	\$16,228	\$16,715	\$17,217	\$17,733	\$18,265	\$18,813	\$19,378	\$19,959	\$20,558
8-Week Annual Pumping Duration ⁸		\$13,730	\$14,142	\$14,566	\$15,003	\$15,453	\$15,917	\$16,394	\$16,886	\$17,393	\$17,914	\$18,452	\$19,005	\$19,575	\$20,163	\$20,768	\$21,391	\$22,032	\$22,693	\$23,374
										<u>.</u>										
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19
	v	-	-	,	-	5	v	,	0	5	10			13	14	15	10	17	10	15

NOTES:
1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

8) Assumes pumping power costs, or power rates, increase at the assumed rate of inflation.

Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of

20	21	22	23
\$19,768	\$20,361	\$20,972	\$21,601
\$10,940	\$11,861	\$12,827	\$13,841
\$395,358	\$427,579	\$461,379	\$496,821
\$39,536	\$40,722	\$41,944	\$43,202
\$21,879	\$23,721	\$25,655	\$27,683
\$790,715	\$855,159	\$922,757	\$993,642
\$79,072	\$81,444	\$83,887	\$86,404
\$43,758	\$47,443	\$51,310	\$55,365
\$1,581,431	\$1,710,318	\$1,845,514	\$1,987,283
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$11,748	\$12,101	\$12,464	\$12,838
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$22,480	\$23,154	\$23,849	\$24,564
\$15,373	\$15,834	\$16,309	\$16,798
\$18,274	\$18,822	\$19,386	\$19,968
\$21,174	\$21,810	\$22,464	\$23,138
\$24,075	\$24,798	\$25,542	\$26,308
20	21	22	23
20	21	~~	25

314.005 \$17.187 \$18.411 \$19.993 \$22.005 \$22.000 \$23.238 \$32.035 \$32.238 \$34.239 \$33.625 \$30.656 \$42.844 \$45.225 \$47.726 \$51.860.77 \$50.288 \$53.307 \$53.397 \$57.200 \$51.351.861 \$1.478.120 \$1.57.785 \$1.67.786 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 \$1.67.756 <th< th=""><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th></th<>																							
S533.974 S572.910 S513.701 S553.974 S701.158 S776.982 S448.265 S901.977 S977.984 S10.16.624 S10.779.20 S1.327.833 S1.321.811 S1.428.120 S1.377.38 S1.370.847 S1.370.857 S1.370.857 S1.370.857 S1.370.857 S1.370.857 S1.370.857 S1.370.857.757 S1.370	. ,								. ,					. ,			. ,					. ,	\$42,631
544.486 547,208 548,674 550,083 551,383 554,727 556,369 558,000 559,001 561,345 560,347 567,307 569,326 571,006 577,555 578,007 580,386 592,208 531,383 554,727 556,369 556,000 559,001 561,345 560,347 567,337 569,326 571,006 577,555 578,007 580,008 554,113 550,986 551,108 512,235 527,355 577,200 551,112 553,513 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 533,551,73 </td <td>. ,</td> <td></td> <td></td> <td>. ,</td> <td></td> <td>. ,</td> <td></td> <td>. ,</td> <td>. ,</td> <td></td> <td>. ,</td> <td>. ,</td> <td></td> <td></td> <td></td> <td>1 /</td> <td>. ,</td> <td></td> <td></td> <td></td> <td>1 ,</td> <td></td> <td>\$55,876</td>	. ,			. ,		. ,		. ,	. ,		. ,	. ,				1 /	. ,				1 ,		\$55,876
S20,809 S32,808 S34,275 S36,822 S39,885 S42,069 S44,879 S47,819 S50,896 S51,114 S57,779 S86,075 S48,195 S72,535 S77,700 S81,112 S86,687 S90,461 S100,695 S10,67,94 S1,067,949 S1,145,820 S1,227,402 S1,312,848 S1,605,508 S1,065,508 S1,067,94 S2,285,949 S2,417,825 S2,576,66 S2,075,723 S2,886,240 S3,015,476 S3,355,173 S3,356,116 S3,355,173 S3,356,116 S3,355,173 S3,356,175 S3,356,175 S3,356,175 S3,356,175 S3,356,175 S3,356,175 S3,356,175 S3,55,575 S2,577,66 S2,073,723 S2,866,240 S3,015,775 S15,055 S10,775 S15,055 S10,775 S15,055 S10,775 S10,808 S11,272,75 S3,350,173 S3,350,173 S3,350,173 S3,350,173 S3,350,173 S3,350,173 S12,217 S11,080 S11,217,23 S11,217,23 S11,217,23 S11,217,23 S11,217,23 S11,217,23 S11,217,23 S11,217,23 S11,217,23	\$533,974	\$572,910	\$613,701	\$656,424	\$701,158	\$747,986	\$796,992	\$848,265	\$901,897	\$957,984	\$1,016,624	\$1,077,920	\$1,141,980	\$1,208,912	\$1,278,833	\$1,351,861	\$1,428,120	\$1,507,738	\$1,590,847	\$1,677,586	\$1,768,098	\$1,862,530	\$1,961,038
\$29,009 \$32,028 \$34,279 \$36,822 \$39,285 \$42,009 \$44,879 \$47,819 \$50,007,949 \$1,145,820 \$1,27,402 \$1,132,848 \$1,402,317 \$1,593,988 \$1,605,589 \$2,033,248 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,283,959 \$2,417,825 \$2,185,640 \$3,015,476 \$3,015,476 \$3,015,476 \$3,015,476 \$3,015,476 \$3,015,476 \$3,015,476 \$3,015,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476 \$3,013,476<	\$44.498	\$45,833	\$47.208	\$48.624	\$50.083	\$51,585	\$53,133	\$54,727	\$56,369	\$58.060	\$59.801	\$61,595	\$63,443	\$65.347	\$67.307	\$69.326	\$71,406	\$73,548	\$75.755	\$78.027	\$80.368	\$82,779	\$85,263
S88,996 S91,666 S94,416 S97,248 S100,165 S103,170 S106,266 S109,454 S112,737 S116,119 S119,603 S123,191 S126,887 S130,693 S134,614 S138,652 S142,812 S147,096 S151,509 S156,055 S160,736 S165,558 S S2,135,897 S2,291,640 S2,464,805 S2,627,697 S2,804,633 S2,991,943 S3,87,957 S3,839,095 S3,607,588 S3,81,935 S4,066,496 S4,311,681 S4,856,499 S5,115,333 S5,407,445 S5,712,481 S6,030,951 S6,630,951 S6,710,345 S7,077,30 S7,757 S8,010 S8,250 S8,498 S8,753 S9,015 S9,286 S9,561	. ,		, ,	. ,		. ,		1 - 7	. ,		. ,										1 /	1 - 7 -	\$111,752
\$59,618 \$68,077 \$68,077 \$58,749 \$77,771 \$58,139 \$59,539 \$101,792 \$114,958 \$121,995 \$137,038 \$145,069 \$153,460 \$153,460 \$152,223 \$171,374 \$180,929 \$190,902 \$201,310 \$21,172 \$ \$2,135,497 \$2,291,640 \$2,655,697 \$2,804,633 \$2,991,943 \$3,387,967 \$3,393,059 \$3,607,588 \$3,381,335 \$4,666,496 \$4,311,681 \$4,567,919 \$4,835,649 \$5,113,333 \$5,407,445 \$5,712,481 \$6,630,951 \$6,633,389 \$6,710,345 \$7,777,320 \$7,777,380,010 \$8,250 \$8,498 \$8,753 \$9,015 \$9,826 \$9,851 \$10,147 \$10,475 \$11,088 \$11,420 \$11,763 \$12,116 \$2,605 \$2,683 \$2,764 \$2,847 \$2,932 \$3,000 \$3,300 \$3,399 \$3,501 \$3,606 \$3,714 \$3,826 \$3,940 \$4,305 \$4,435 \$4,455 \$4,456 \$4,755 \$4,846 \$4,757 \$4,846 \$1,420 \$1,763 \$12,146 \$1,848 \$1,813 \$1,414 \$1,1763 \$12,1475 \$12,145 \$1,	\$1,067,949	\$1,145,820	\$1,227,402	\$1,312,848	\$1,402,317	\$1,495,971	\$1,593,983	\$1,696,530	\$1,803,794	\$1,915,967	\$2,033,248	\$2,155,841	\$2,283,959	\$2,417,825	\$2,557,666	\$2,703,723	\$2,856,240	\$3,015,476	\$3,181,695	\$3,355,173	\$3,536,196	\$3,725,061	\$3,922,075
SSS,618 Sc4,077 Sc48,749 S78,718 S84,133 S89,758 S95,639 S101,792 S112,995 S122,9350 S137,038 S145,069 S153,460 S152,223 S17,1374 S180,929 S190,902 S20,1310 S21,172 S S2,135,897 S2,231,640 S2,645,805 S2,625,697 S2,804,633 S2,991,943 S3,187,967 S3,393,059 S3,607,588 S3,81,935 S4,066,496 S4,311,681 S4,567,919 S4,835,649 S5,115,333 S5,007,445 S5,712,481 S6,030,951 S6,633,389 S6,710,345 S7,777,320 S7,777,380,010 S8,250 S8,498 S8,753 S9,015 S9,286 S9,564 S9,851 S10,147 S10,475 S11,048 S11,763 S12,116 S1,460 S4,305 S4,335 S4,465 S4,945 S4,846 S4,753 S9,015 S9,286 S9,851 S10,147 S10,475 S11,048 S11,763 S12,116 S1,460 S1,376 S12,146 S12,848 S12,149 S12,848 S1,371 S18,846 S1,371 S18,846 S1,371 S18,362 S1,449 S1,4305 S1,417 S12,187																							
\$2,135,897 \$2,291,640 \$2,2454,805 \$2,265,697 \$2,804,633 \$2,991,943 \$3,187,967 \$3,393,059 \$3,007,588 \$3,831,935 \$4,066,649 \$4,311,681 \$4,567,919 \$4,835,649 \$5,115,333 \$5,407,445 \$5,712,481 \$6,030,951 \$6,363,389 \$6,710,345 \$7,702,392 \$7,450,122 \$7,450,122 \$7,50 \$5,513 \$5,670 \$2,605 \$2,605 \$2,605 \$2,604 \$2,821 \$3,020 \$3,311 \$3,204 \$3,300 \$3,399 \$3,501 \$3,666 \$5,712 \$5,126 \$1,045 \$4,059 \$4,035 \$4,435 \$4,435 \$4,435 \$4,435 \$4,568 \$4,705 \$4,836 \$4,568 \$4,715 \$4,836 \$1,705 \$1,188 \$1,1178 \$1,128 \$1,260 \$1,710 \$1,804 \$1,853 \$19,418 \$20,001 \$21,219 \$21,856 \$22,511 \$23,187 \$23,882 \$24,599 \$3,167 \$3,260 \$3,401 \$1,863 \$19,418 \$20,001 \$21,219 \$21,816 \$22,511 \$23,187 \$23,882 \$24,599 \$3,167 \$3,265 \$3,715 \$3,161 \$3,600	\$88,996	\$91,666	\$94,416	\$97,248	\$100,165	\$103,170	\$106,266	\$109,454	\$112,737	\$116,119	\$119,603	\$123,191	\$126,887	\$130,693	\$134,614	\$138,652	\$142,812	\$147,096	\$151,509	\$156,055	\$160,736	\$165,558	\$170,525
\$6,513 \$6,708 \$6,909 \$7,117 \$7,330 \$7,550 \$7,777 \$8,010 \$8,250 \$8,498 \$8,753 \$9,015 \$9,286 \$9,851 \$10,147 \$10,451 \$10,765 \$11,088 \$11,420 \$11,763 \$12,116 \$2,605 \$2,683 \$2,764 \$2,847 \$2,932 \$3,020 \$3,111 \$3,204 \$3,300 \$3,399 \$3,501 \$3,666 \$3,714 \$3,826 \$3,940 \$4,059 \$4,180 \$4,435 \$4,568 \$4,705 \$4,846 \$987 \$1,016 \$1,047 \$1,078 \$1,111 \$1,144 \$1,178 \$1,214 \$1,250 \$1,288 \$1,326 \$1,497 \$1,493 \$1,537 \$1,584 \$1,631 \$1,680 \$1,730 \$1,782 \$1,836 \$1,3223 \$13,620 \$14,449 \$1,483 \$15,229 \$15,789 \$15,727 \$2,050 \$2,775 \$2,625 \$2,732 \$2,814 \$2,898 \$3,075 \$3,167 \$3,365 \$3,661 \$1,9418 \$20,060 \$26,660 \$20,660 \$20,660 \$22,877 \$2,205 \$2,675 \$22,875<	\$59,618	\$64,077	\$68,749	\$73,644	\$78,771	\$84,139	\$89,758	\$95,639	\$101,792	\$108,228	\$114,958	\$121,995	\$129,350	\$137,038	\$145,069	\$153,460	\$162,223	\$171,374	\$180,929	\$190,902	\$201,310	\$212,172	\$223,504
\$2,605 \$2,683 \$2,764 \$2,847 \$2,932 \$3,000 \$3,111 \$3,204 \$3,300 \$3,399 \$3,511 \$3,606 \$3,714 \$3,826 \$3,940 \$4,059 \$4,405 \$4,435 \$4,458 \$4,456 \$4,705 \$4,846 \$987 \$1,016 \$1,047 \$1,078 \$1,111 \$1,144 \$1,178 \$1,214 \$1,250 \$1,288 \$1,326 \$1,366 \$1,493 \$1,537 \$1,584 \$1,611 \$1,680 \$1,720 \$1,782 \$1,836 \$13,223 \$13,620 \$14,042 \$14,883 \$15,329 \$15,789 \$1,623 \$16,751 \$17,233 \$17,711 \$18,904 \$18,853 \$19,418 \$20,005 \$21,219 \$21,816 \$22,814 \$2,884 \$2,884 \$2,885 \$3,075 \$3,167 \$3,2262 \$3,306 \$3,441 \$3,555 \$3,671 \$0 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50	\$2,135,897	\$2,291,640	\$2,454,805	\$2,625,697	\$2,804,633	\$2,991,943	\$3,187,967	\$3,393,059	\$3,607,588	\$3,831,935	\$4,066,496	\$4,311,681	\$4,567,919	\$4,835,649	\$5,115,333	\$5,407,445	\$5,712,481	\$6,030,951	\$6,363,389	\$6,710,345	\$7,072,392	\$7,450,122	\$7,844,151
\$22,605 \$22,683 \$22,764 \$22,847 \$22,932 \$3,000 \$3,111 \$3,204 \$3,300 \$3,399 \$3,511 \$3,606 \$3,714 \$3,826 \$3,940 \$4,059 \$4,435 \$4,435 \$4,568 \$4,705 \$4,846 \$987 \$1,016 \$1,047 \$1,078 \$1,111 \$1,144 \$1,178 \$1,214 \$1,250 \$1,288 \$1,326 \$1,366 \$1,407 \$1,449 \$1,483 \$1,537 \$1,5184 \$1,611 \$1,680 \$1,720 \$1,782 \$1,836 \$13,223 \$13,620 \$14,028 \$14,449 \$14,883 \$15,329 \$15,789 \$16,633 \$16,751 \$17,253 \$17,717 \$18,304 \$18,853 \$19,418 \$20,007 \$21,119 \$21,856 \$22,511 \$22,814 \$24,569 \$20,610 \$21,219 \$21,219 \$21,219 \$21,218 \$22,814 \$21,816 \$23,007 \$3,167 \$33,202 \$3,301 \$34,003 \$35,023 \$20,610 \$21,219 \$21,218 \$22,814 \$23,804 \$3,6071 \$20,601 \$41,819 \$43,074 \$44,366 \$46,597 \$47,068																							
\$987 \$1,016 \$1,047 \$1,078 \$1,111 \$1,178 \$1,214 \$1,250 \$1,288 \$1,326 \$1,407 \$1,493 \$1,537 \$1,584 \$1,631 \$1,680 \$1,730 \$1,782 \$1,836 \$13,223 \$13,620 \$14,449 \$14,883 \$15,329 \$15,789 \$16,623 \$16,751 \$17,253 \$17,771 \$18,804 \$18,853 \$19,418 \$20,001 \$20,601 \$21,219 \$21,856 \$22,511 \$23,187 \$23,882 \$24,599 \$3,671 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50	\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479
\$13,223 \$13,620 \$14,028 \$14,449 \$14,883 \$15,329 \$15,789 \$16,263 \$16,751 \$17,725 \$17,771 \$18,304 \$18,853 \$19,418 \$20,001 \$20,601 \$21,219 \$21,856 \$22,511 \$23,187 \$23,882 \$24,599 \$3,971 \$51,974 \$20,051 \$2,127 \$2,288 \$2,357 \$2,427 \$2,500 \$2,575 \$2,652 \$2,732 \$2,814 \$2,898 \$2,985 \$3,075 \$3,167 \$3,262 \$3,360 \$3,461 \$3,565 \$3,671 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50 \$50	\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992
\$1,974 \$2,033 \$2,094 \$2,157 \$2,221 \$2,288 \$2,357 \$2,427 \$2,500 \$2,575 \$2,652 \$2,732 \$2,814 \$2,898 \$2,985 \$3,167 \$3,262 \$3,360 \$3,461 \$3,565 \$3,671 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 \$0 </td <td>\$987</td> <td>\$1,016</td> <td>\$1,047</td> <td>\$1,078</td> <td>\$1,111</td> <td>\$1,144</td> <td>\$1,178</td> <td>\$1,214</td> <td>\$1,250</td> <td>\$1,288</td> <td>\$1,326</td> <td>\$1,366</td> <td>\$1,407</td> <td>\$1,449</td> <td>\$1,493</td> <td>\$1,537</td> <td>\$1,584</td> <td>\$1,631</td> <td>\$1,680</td> <td>\$1,730</td> <td>\$1,782</td> <td>\$1,836</td> <td>\$1,891</td>	\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891
S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 S0 <th< td=""><td>\$13,223</td><td>\$13,620</td><td>\$14,028</td><td>\$14,449</td><td>\$14,883</td><td>\$15,329</td><td>\$15,789</td><td>\$16,263</td><td>\$16,751</td><td>\$17,253</td><td>\$17,771</td><td>\$18,304</td><td>\$18,853</td><td>\$19,418</td><td>\$20,001</td><td>\$20,601</td><td>\$21,219</td><td>\$21,856</td><td>\$22,511</td><td>\$23,187</td><td>\$23,882</td><td>\$24,599</td><td>\$25,337</td></th<>	\$13,223	\$13,620	\$14,028	\$14,449	\$14,883	\$15,329	\$15,789	\$16,263	\$16,751	\$17,253	\$17,771	\$18,304	\$18,853	\$19,418	\$20,001	\$20,601	\$21,219	\$21,856	\$22,511	\$23,187	\$23,882	\$24,599	\$25,337
\$25,301 \$26,660 \$27,648 \$28,477 \$29,331 \$30,211 \$31,118 \$32,051 \$33,013 \$34,003 \$35,023 \$36,074 \$37,156 \$38,271 \$39,419 \$40,601 \$41,819 \$44,366 \$46,697 \$47,068 \$17,302 \$17,821 \$18,356 \$18,906 \$19,474 \$20,058 \$20,660 \$21,279 \$21,918 \$22,575 \$23,253 \$23,950 \$24,669 \$25,409 \$26,171 \$26,956 \$27,765 \$28,598 \$29,456 \$30,339 \$31,250 \$32,187 \$20,567 \$21,184 \$21,820 \$22,474 \$23,148 \$23,843 \$24,558 \$25,295 \$26,054 \$26,835 \$27,640 \$28,470 \$29,324 \$30,003 \$31,110 \$32,043 \$33,994 \$35,014 \$36,064 \$37,146 \$38,261 \$23,832 \$24,547 \$25,283 \$26,602 \$22,627 \$29,324 \$30,003 \$31,110 \$32,043 \$33,994 \$35,014 \$36,044 \$37,146 \$38,261 \$23,832 \$24,547 \$25,283 \$26,6042 \$26,823 \$27,628 \$28,457 \$29,31	\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782
\$17,302 \$17,821 \$18,356 \$18,906 \$19,474 \$20,058 \$20,660 \$21,279 \$21,918 \$22,575 \$23,253 \$23,950 \$24,669 \$25,409 \$26,171 \$26,956 \$27,765 \$28,598 \$29,456 \$30,339 \$31,250 \$32,187 \$20,567 \$21,184 \$21,820 \$22,474 \$23,148 \$23,843 \$24,558 \$25,295 \$26,054 \$26,835 \$27,640 \$28,470 \$29,324 \$30,203 \$31,110 \$32,043 \$33,004 \$33,994 \$35,014 \$36,064 \$37,146 \$38,261 \$23,832 \$24,547 \$25,283 \$26,042 \$26,823 \$27,628 \$28,457 \$29,310 \$30,190 \$31,095 \$32,028 \$32,989 \$33,979 \$34,998 \$36,048 \$37,130 \$38,243 \$39,391 \$40,572 \$41,790 \$43,043 \$44,335 \$27,097 \$27,910 \$28,747 \$29,610 \$30,498 \$31,413 \$32,355 \$33,326 \$34,326 \$35,355 \$36,416 \$37,509 \$38,634 \$39,793 \$40,987 \$42,216 \$43,483 \$44,787 \$46,131 \$47,515 \$48,940 \$50,408			1.5														1.	1 -					\$0
\$20,567 \$21,184 \$21,820 \$22,474 \$23,148 \$23,843 \$24,558 \$25,295 \$26,054 \$26,835 \$27,640 \$28,470 \$29,324 \$30,203 \$31,110 \$32,043 \$33,094 \$35,014 \$35,014 \$36,064 \$37,146 \$38,261 \$23,832 \$24,547 \$25,283 \$26,042 \$26,823 \$27,628 \$28,457 \$29,310 \$30,190 \$31,095 \$32,028 \$32,989 \$33,979 \$34,998 \$36,048 \$37,130 \$38,243 \$39,391 \$40,572 \$41,790 \$43,043 \$44,335 \$27,970 \$27,910 \$28,747 \$29,610 \$30,498 \$31,413 \$32,355 \$33,326 \$34,326 \$35,355 \$36,416 \$37,509 \$38,634 \$39,793 \$40,987 \$42,216 \$43,483 \$44,787 \$46,131 \$47,515 \$48,940 \$50,048	\$25,301	\$26,060	\$26,842	\$27,648	\$28,477	\$29,331	\$30,211	\$31,118	\$32,051	\$33,013	\$34,003	\$35,023	\$36,074	\$37,156	\$38,271	\$39,419	\$40,601	\$41,819	\$43,074	\$44,366	\$45,697	\$47,068	\$48,480
\$20,567 \$21,184 \$21,820 \$22,474 \$23,148 \$23,843 \$24,558 \$25,295 \$26,054 \$26,835 \$27,640 \$28,470 \$29,324 \$30,203 \$31,110 \$32,043 \$33,094 \$35,014 \$35,014 \$36,064 \$37,146 \$38,261 \$23,832 \$24,547 \$25,283 \$26,042 \$26,823 \$27,628 \$28,457 \$29,310 \$30,190 \$31,095 \$32,028 \$32,989 \$33,979 \$34,998 \$36,048 \$37,130 \$38,243 \$39,391 \$40,572 \$41,790 \$43,043 \$44,335 \$27,970 \$27,910 \$28,747 \$29,610 \$30,498 \$31,413 \$32,355 \$33,326 \$34,326 \$35,355 \$36,416 \$37,509 \$38,634 \$39,793 \$40,987 \$42,216 \$43,483 \$44,787 \$46,131 \$47,515 \$48,940 \$50,048																							
\$23,832 \$24,547 \$25,283 \$26,042 \$26,823 \$27,628 \$28,457 \$29,310 \$30,190 \$31,095 \$32,028 \$32,989 \$33,979 \$34,998 \$36,048 \$37,130 \$38,243 \$39,391 \$40,572 \$41,790 \$43,043 \$44,335 \$27,097 \$27,910 \$28,747 \$29,610 \$30,498 \$31,413 \$32,355 \$33,326 \$35,355 \$36,416 \$37,509 \$38,634 \$39,793 \$40,987 \$42,216 \$43,483 \$44,787 \$46,131 \$47,515 \$48,940 \$50,408	\$17,302	\$17,821	\$18,356	\$18,906	\$19,474	\$20,058	\$20,660	\$21,279	\$21,918	\$22,575	\$23,253	\$23,950	\$24,669	\$25,409	\$26,171	\$26,956	\$27,765	\$28,598	\$29,456	\$30,339	\$31,250	\$32,187	\$33,153
\$27,097 \$27,910 \$28,747 \$29,610 \$30,498 \$31,413 \$32,355 \$33,326 \$34,326 \$35,355 \$36,416 \$37,509 \$38,634 \$39,793 \$40,987 \$42,216 \$43,483 \$44,787 \$46,131 \$47,515 \$48,940 \$50,408	\$20,567	\$21,184	\$21,820	\$22,474	\$23,148	\$23,843	\$24,558	\$25,295	\$26,054	\$26,835	\$27,640	\$28,470	\$29,324	\$30,203	\$31,110	\$32,043	\$33,004	\$33,994	\$35,014	\$36,064	\$37,146	\$38,261	\$39,409
	\$23,832	\$24,547	\$25,283	\$26,042	\$26,823	\$27,628	\$28,457	\$29,310	\$30,190	\$31,095	\$32,028	\$32,989	\$33,979	\$34,998	\$36,048	\$37,130	\$38,243	\$39,391	\$40,572	\$41,790	\$43,043	\$44,335	\$45,665
	\$27,097	\$27,910	\$28,747	\$29,610	\$30,498	\$31,413	\$32,355	\$33,326	\$34,326	\$35,355	\$36,416	\$37,509	\$38,634	\$39,793	\$40,987	\$42,216	\$43,483	\$44,787	\$46,131	\$47,515	\$48,940	\$50,408	\$51,921
			T	T	<u> </u>		T	T	T	<u> </u>	T	<u> </u>	П	Т	I	<u> </u>	<u> </u>	I	I		I		—T
24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

46	47	48	49	50
\$42,631	\$43,910	\$45,227	\$46,584	\$47,982
\$55,876	\$58,831	\$61,913	\$65,128	\$68,479
961,038	\$2,063,779		\$2,282,632	\$2,399,093
			.,.,.	
	407 000	400.455	400 A 60	405.004
\$85,263	\$87,820		\$93,169	\$95,964
111,752 9 22,075	\$117,662 \$4,127,558	\$123,827 \$4,341,840	\$130,255 \$4,565,264	\$136,958 \$4,798,185
522,075	J 4 ,127, 3 30	J4,J41,040	J4,303,204	<u>,,,,,,</u> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
170,525	\$175,641	\$180,910	\$186,337	\$191,927
223,504	\$235,325	\$247,653	\$260,510	\$273,916
844,151	\$8,255,116	\$8,683,679	\$9,130,527	\$9,596,370
\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$1,891	\$1,948	\$2,006	\$2,066	\$2,128
\$25,337	\$26,097	\$26,880	\$27,686	\$28,517
\$3,782	\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0	\$0
\$48,480	\$49,934	\$51,432	\$52,975	\$54,565
\$33,153	\$34,147	\$35,172	\$36,227	\$37,314
\$39,409	\$40,591	\$41,809	\$43,063	\$44,355
\$45,665	\$47,035	\$48,446	\$49,899	\$51,396
\$51,921	\$53,478	\$55,083	\$56,735	\$58,437
				<u> </u>
16	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 3, Design Flow Rate = 10 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$1,861,000 Total Ca	pital Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replace	ment Cost:		
To Replace	25%	After Life of Project		\$2,039,612
To Replace	50%	After Life of Project		\$4,079,225
To Replace	100%	After Life of Project	\$1,861,000	\$8,158,449
Disposal and Re	moval Cost			
To Replace	25%	After Life of Project		\$52,607
To Replace	50%	After Life of Project		\$105,214
To Replace	100%	After Life of Project	\$48,000	\$210,427
Total Replacem	ent Cost:			
To Replace	25%	After Life of Project		\$2,092,219
To Replace	50%	After Life of Project		\$4,184,438

\$1,909,000

\$1,861,000

\$8,368,877

REPLACEMENT	FUND SUIV	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$18,549	
To Replace	50%	After Life of Project	\$37,097	
To Replace	100%	After Life of Project	\$74,194	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Bate of Inflation):	
To Replace	25%	After Life of Project	\$9,831	
To Replace	50%	After Life of Project	\$19,663	
To Replace	100%	After Life of Project	\$39,325	
Deposit Require	d at Year 2	5 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$19,985	
To Replace	50%	After Life of Project	\$39,970	
To Replace	100%	After Life of Project	\$79,940	
Deposit Require	d at Year 5	0 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$41,844	
To Replace	50%	After Life of Project	\$83,689	
To Replace	100%	After Life of Project	\$167,378	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual C	perating Durat	ion:	
25% Replacement	\$477,250	\$596,000	\$424,213	\$1,497,463
50% Replacement	\$954,500	\$596,000	\$424,213	\$1,974,713
100% Replacement	\$1,909,000	\$596,000	\$424,213	\$2,929,213
Assuming the Pumping Power Costs for a 4-	week Annual C	perating Durat	ion:	
25% Replacement	\$477,250	\$596,000	\$504,241	\$1,577,491
50% Replacement	\$954,500	\$596,000	\$504,241	\$2,054,741
100% Replacement	\$1,909,000	\$596,000	\$504,241	\$3,009,241
Assuming the Pumping Power Costs for a 6-	week Annual C	perating Durat	ion:	
25% Replacement	\$477,250	\$596,000	\$584,270	\$1,657,520
50% Replacement	\$954,500	\$596,000	\$584,270	\$2,134,770
100% Replacement	\$1,909,000	\$596,000	\$584,270	\$3,089,270
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$477,250	\$596,000	\$664,298	\$1,737,548
50% Replacement	\$954,500	\$596,000	\$664,298	\$2,214,798
100% Replacement	\$1,909,000	\$596,000	\$664,298	\$3,169,298

LIFE	CYCLE	COSTS:
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Year 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19	LIFE CYCLE COSTS:																				
	Year	0	1	2	3	4	5	6	7	8	9	10	12	13	14	15	16	17	18	19	

Capital Expenses:

To Replace 100% After Life of Project

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Deposits		\$9,831	\$10,126	\$10,430	\$10,743	\$11,065	\$11,397	\$11,739	\$12,091	\$12,454	\$12,828	\$13,213	\$13,609	\$14,017	\$14,438	\$14,871	\$15,317	\$15,776	\$16,250	\$16,737	(
Interest		\$0	\$295	\$608	\$939	\$1,289	\$1,660	\$2,052	\$2,465	\$2,902	\$3,363	\$3,848	\$4,360	\$4,899	\$5,467	\$6,064	\$6,692	\$7,352	\$8,046	\$8,775	,
End of Year Balance		\$9,831	\$20,253	\$31,290	\$42,972	\$55,326	\$68,383	\$82,174	\$96,731	\$112,087	\$128,277	\$145,338	\$163,307	\$182,223	\$202,127	\$223,062	\$245,071	\$268,199	\$292,495	\$318,007	/
Replacement Fund (For Funding Replacement	of 50% of Sy	,					444 - 444	*** ***				*** ***			*** ***			*** ***		400	_
Deposits		\$19,663	\$20,253	\$20,860	\$21,486	\$22,131	\$22,794	\$23,478	\$24,183	\$24,908	\$25,655	\$26,425	\$27,218	\$28,034	\$28,875	\$29,742	\$30,634	\$31,553	\$32,499	\$33,474	
Interest		\$0		\$1,215	\$1,877	\$2,578	\$3,320	\$4,103	\$4,930	\$5,804	\$6,725	\$7,697	\$8,720	\$9,798	\$10,933	\$12,128	\$13,384	\$14,704	\$16,092	\$17,550	
End of Year Balance		\$19,663	\$40,505	\$62,580	\$85,944	\$110,653	\$136,767	\$164,348	\$193,461	\$224,173	\$256,554	\$290,675	\$326,613	\$364,446	\$404,255	\$446,124	\$490,142	\$536,399	\$584,990	\$636,014	
Replacement Fund (For Funding Replacement	of 100% of S	ystem):																			
Deposits		\$39,325	\$40,505	\$41,720	\$42,972	\$44,261	\$45,589	\$46,957	\$48,365	\$49,816	\$51,311	\$52,850	\$54,436	\$56,069	\$57,751	\$59,483	\$61,268	\$63,106	\$64,999	\$66,949	,
Interest		\$0	\$1,180	\$2,430	\$3,755	\$5,157	\$6,639	\$8,206	\$9,861	\$11,608	\$13,450	\$15,393	\$17,441	\$19,597	\$21,867	\$24,255	\$26,767	\$29,408	\$32,184	\$35,099	j.
End of Year Balance		\$39,325	\$81,010	\$125,161	\$171,888	\$221,305	\$273,534	\$328,696	\$386,922	\$448,346	\$513,107	\$581,351	\$653,227	\$728,892	\$808,509	\$892,248	\$980,283	\$1,072,797	\$1,169,980	\$1,272,028	1
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	/
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$5,800	\$5,974	\$6,153	\$6,338	\$6,528	\$6,724	\$6,926	\$7,133	\$7,347	\$7,568	\$7,795	\$8,029	\$8,269	\$8,517	\$8,773	\$9,036	\$9,307	\$9,587	\$9,874	ŧ
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	1
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	J
Total O&M Expenses		\$11,920	\$12,278	\$12,646	\$13,025	\$13,416	\$13,819	\$14,233	\$14,660	\$15,100	\$15,553	\$16,019	\$16,500	\$16,995	\$17,505	\$18,030	\$18,571	\$19,128	\$19,702	\$20,293	÷
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$8,484	\$8,739	\$9,001	\$9,271	\$9,549	\$9,836	\$10,131	\$10,435	\$10,748	\$11,070	\$11,402	\$11,744	\$12,097	\$12,459	\$12,833	\$13,218	\$13,615	\$14,023	\$14,444	4
4-Week Annual Pumping Duration ⁸		\$10,085	\$10,387	\$10,699	\$11,020	\$11,351	\$11,691	\$12,042	\$12,403	\$12,775	\$13,158	\$13,553	\$13,960	\$14,379	\$14,810	\$15,254	\$15,712	\$16,183	\$16,669	\$17,169	,
6-Week Annual Pumping Duration ⁸		\$11,685	\$12,036	\$12,397	\$12,769	\$13,152	\$13,547	\$13,953	\$14,372	\$14,803	\$15,247	\$15,704	\$16,175	\$16,661	\$17,160	\$17,675	\$18,205	\$18,752	\$19,314	\$19,894	1
8-Week Annual Pumping Duration ⁸		\$13,286	\$13,685	\$14,095	\$14,518	\$14,953	\$15,402	\$15,864	\$16,340	\$16,830	\$17,335	\$17,855	\$18,391	\$18,943	\$19,511	\$20,096	\$20,699	\$21,320	\$21,960	\$22,618	
		÷10)100	÷10,000	÷1,000	<i>11,010</i>	÷ 1 1,500	÷10) 101	÷10)001	<i>+_0)010</i>	÷=0)000	<i>+_/)000</i>	<i>+_1,000</i>	÷-0,001	÷ 20)5 10	<i>, 10)011</i>	÷10,000	÷20,000	÷==)0=0	÷11)500	÷==)010	
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Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23					
\$17,239	\$17,757	\$18,289	\$18,838					
\$9,540	\$10,344	\$11,187	\$12,071					
\$344,787	\$372,887	\$402,363	\$433,271					
624 470	Ć25 542	¢26 570	627.676					
\$34,479	\$35,513	\$36,578	\$37,676					
\$19,080 \$689,573	\$20,687 \$745,773	\$22,373 \$804,725	\$24,142 \$866,543					
\$089,57 5	\$745,775	\$604,725	3000,343					
\$68,957	\$71,026	\$73,157	\$75,352					
\$38,161	\$41,374	\$44,746	\$48,284					
\$1,379,147	\$1,491,547	\$1,609,450	\$1,733,085					
\$5,787	\$5,960	\$6,139	\$6,323					
\$2,315	\$2,384	\$2,456	\$2,529					
\$877	\$903	\$930	\$958					
\$10,170	\$10,475	\$10,790	\$11,113					
\$1,754	\$1,806	\$1,860	\$1,916					
\$0	\$0	\$0	\$0					
\$20,902	\$21,529	\$22,175	\$22,840					
\$14,877	\$15,324	\$15,783	\$16,257					
\$17,684	\$18,214	\$18,761	\$19,324					
\$20,490	\$21,105	\$21,738	\$22,390					
\$23,297	\$23,996	\$24,716	\$25,457					
			I					
20	21	22	23					

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
•																										
\$19,403 \$12,998 \$465,672	\$19,985 \$13,970 \$499,628	\$20,585 \$14,989 \$535,201	\$21,202 \$16,056 \$572,459	\$21,838 \$17,174 \$611,472	\$22,493 \$18,344 \$652,309	\$23,168 \$19,569 \$695,047	\$23,863 \$20,851 \$739,761	\$24,579 \$22,193 \$786,533	\$25,317 \$23,596 \$835,446	\$26,076 \$25,063 \$886,585	\$26,858 \$26,598 \$940,041	\$27,664 \$28,201 \$995,906	\$28,494 \$29,877 \$1,054,278	\$29,349 \$31,628 \$1,115,255	\$30,229 \$33,458 \$1,178,942	\$31,136 \$35,368 \$1,245,446	\$32,070 \$37,363 \$1,314,880	\$33,032 \$39,446 \$1,387,358	\$34,023 \$41,621 \$1,463,002	\$35,044 \$43,890 \$1,541,937	\$36,095 \$46,258 \$1,624,290	\$37,178 \$48,729 \$1,710,197	\$38,294 \$51,306 \$1,799,796	\$39,442 \$53,994 \$1,893,233	\$40,626 \$56,797 \$1,990,655	\$41,844 \$59,720 \$2,092,219
3 4 03,072	Ş 4 55,028	<i>\$</i> 5555 ,201	<i>4372,</i> 4 33	<i>J</i> 011,472	<i>3032,303</i>	<i>3033,047</i>	<i>\$735,70</i> 1	<i>9780,33</i> 3	Ş855,440	3000,303	Ş340,041	<i>Ş</i> 353,500	Ş1,0 5 4,278	<i>Ş1,113,233</i>	Ş1,178,542	<i>31,243,440</i>	<i>91,914,000</i>	Ş1,367,336	Ş1,403,002	Ş1, 3 41,337	Ş1,02 4 ,230	<i>Ş1,1</i> 10,1 <i>51</i>	<i>Ş</i> 1,7 <i>5</i> 5,750	<i>Ş1,033,233</i>	<i>J</i> 1 , <i>J</i> J0 , UJJ	<i>\$2,032,215</i>
\$38,806 \$25,996 \$931,345	\$39,970 \$27,940 \$999,256	\$41,169 \$29,978 \$1,070,403	\$42,404 \$32,112 \$1,144,919	\$43,677 \$34,348 \$1,222,943	\$44,987 \$36,688 \$1,304,618	\$46,336 \$39,139 \$1,390,093	\$47,727 \$41,703 \$1,479,523	\$49,158 \$44,386 \$1,573,067	\$50,633 \$47,192 \$1,670,892	\$52,152 \$50,127 \$1,773,170	\$53,717 \$53,195 \$1,880,082	\$55,328 \$56,402 \$1,991,813	\$56,988 \$59,754 \$2,108,555	\$58,698 \$63,257 \$2,230,509	\$60,459 \$66,915 \$2,357,883	\$62,272 \$70,736 \$2,490,892	\$64,140 \$74,727 \$2,629,759	\$66,065 \$78,893 \$2,774,717	\$68,047 \$83,242 \$2,926,005	\$70,088 \$87,780 \$3,083,873	\$72,191 \$92,516 \$3,248,580	\$74,356 \$97,457 \$3,420,394	\$76,587 \$102,612 \$3,599,593	\$78,885 \$107,988 \$3,786,465	\$81,251 \$113,594 \$3,981,310	\$83,689 \$119,439 \$4,184,438
\$77,612 \$51,993	\$79,940 \$55,881	\$82,339 \$59,955	\$84,809 \$64,224	\$87,353 \$68,695	\$89,974 \$73,377	\$92,673 \$78,277	\$95,453 \$83,406	\$98,317 \$88,771	\$101,266 \$94,384	\$104,304 \$100,253	\$107,433 \$106,390	\$110,656 \$112,805	\$113,976 \$119,509	\$117,395 \$126,513	\$120,917 \$133,831	\$124,545 \$141,473	\$128,281 \$149,454	\$132,129 \$157,786	\$136,093 \$166,483	\$140,176 \$175,560	\$144,381 \$185,032	\$148,713 \$194,915	\$153,174 \$205,224	\$157,769 \$215,976	\$162,502 \$227,188	\$167,378 \$238,879
\$1,862,690	\$1,998,511	\$2,140,805	\$2,289,838	\$2,445,886	\$2,609,236	\$2,780,186	\$2,959,045	\$3,146,133	\$3,341,783	\$3,546,341	\$3,760,164	\$3,983,626	\$4,217,110	\$4,461,019	\$4,715,767	\$4,981,784	\$5,259,519	\$5,549,434	\$5,852,010	\$6,167,746	\$6,497,160	\$6,840,787	\$7,199,185	\$7,572,930	\$7,962,620	\$8,368,877
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606		\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296		
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1,948	\$2,006	\$2,066	
\$11,447	\$11,790	\$12,144	\$12,508	\$12,883	\$13,270	\$13,668	\$14,078	\$14,500	\$14,935	\$15,384	\$15,845		\$16,810	\$17,314	\$17,834	\$18,369	\$18,920	\$19,487	\$20,072	\$20,674	\$21,294	\$21,933	\$22,591	\$23,269	\$23,967	\$24,686
\$1,974 \$0	\$2,033 \$0	\$2,094 \$0	\$2,157 \$0	\$2,221 \$0	\$2,288 \$0	\$2,357 \$0	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,652 \$0	\$2,732 \$0	\$2,814 \$0	\$2,898 \$0	\$2,985 \$0	\$3,075 \$0	\$3,167 \$0	\$3,262 \$0	\$3,360 \$0	\$3,461 \$0	\$3,565 \$0	\$3,671 \$0	\$3,782 \$0	\$3,895 \$0	\$4,012 \$0	\$4,132 \$0	\$4,256 \$0
\$23,525	\$24,231	\$24,958	\$25,707	\$26,478	\$27,272	\$28,090	\$28,933	\$29,801	\$30,695	\$31,616	\$32,564	\$33,541	\$34,547	\$35,584	\$36,651	\$37,751		\$40,050	\$41,251	\$42,489	\$43,764	\$45,077	\$46,429	\$47,822	\$49,256	\$50,734
\$16,744	\$17,247	\$17,764	\$18,297	\$18,846	\$19,411	\$19,994	\$20,594	\$21,211	\$21,848	\$22,503	\$23,178	\$23,874	\$24,590	\$25,327	\$26,087	\$26,870	\$27,676	\$28,506	\$29,361	\$30,242	\$31,150	\$32,084	\$33,047	\$34,038	\$35,059	\$36,111
\$19,903	\$20,500	\$21,115	\$21,749	\$22,401	\$23,073	\$23,766	\$24,479	\$25,213	\$25,969	\$26,748	\$27,551		\$29,229	\$30,105	\$31,009	\$31,939	\$32,897	\$33,884	\$34,901	\$35,948	\$37,026	\$38,137	\$39,281	\$40,459		\$42,923
\$23,062	\$23,754	\$24,467	\$25,201	\$25,957	\$26,735	\$27,537	\$28,364	\$29,214	\$30,091	\$30,994	\$31,923		\$33,868	\$34,884	\$35,930	\$37,008	\$38,118	\$39,262	\$40,440	\$41,653	\$42,902	\$44,189	\$45,515	\$46,881	\$48,287	\$49,736
\$26,221	\$27,008	\$27,818	\$28,652	\$29,512	\$30,397	\$31,309	\$32,249	\$33,216	\$34,212	\$35,239	\$36,296	\$37,385	\$38,506	\$39,662	\$40,851	\$42,077	\$43,339	\$44,639	\$45,979	\$47,358	\$48,779	\$50,242	\$51,749	\$53,302	\$54,901	\$56,548
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 4, Design Flow Rate = 10 CFS

ASSUMPTIONS:				
Estimated Capit	al Cost:		\$2,755,000 Total Capit	al Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REP	ΙΔΟΈΜΕΝΤ	COSTS	CURRENT COST ²	FUTURE COST ³
	-		CURRENT COST	FUTURE COST
Estimated Proje				¢2.010.415
To Replace	25%	After Life of Project		\$3,019,415
To Replace	50%	After Life of Project		\$6,038,831
To Replace	100%	After Life of Project	\$2,755,000	\$12,077,661
Disposal and Re	moval Cost	:		
To Replace	25%	After Life of Project		\$78,910
To Replace	50%	After Life of Project		\$157,821
To Replace	100%	After Life of Project	\$72,000	\$315,641
Total Replacem	ent Cost:			
To Replace	25%	After Life of Project		\$3,098,326
To Replace	50%	After Life of Project		\$6,196,651
To Replace	100%	After Life of Project	\$2,827,000	\$12,393,302

\$2,755,000

Annual Deposit Required (Assume Equal Deposit Made Each Year): To Replace 25% After Life of Project \$27,468 To Replace 50% After Life of Project \$54,936 To Replace 100% After Life of Project \$109,873 Deposit Required at Year 1 (Assume Deposits Increase at the Rate of Inflation): To Replace 25% To Replace 25% After Life of Project \$14,559 To Replace 50% After Life of Project \$29,118 To Replace 100% After Life of Project \$58,236	
To Replace 50% After Life of Project \$54,936 To Replace 100% After Life of Project \$109,873 Deposit Required at Year 1 (Assume Deposits Increase at the Rate of Inflation): To Replace 25% After Life of Project \$14,559 To Replace 50% After Life of Project \$29,118	
To Replace 100% After Life of Project \$109,873 Deposit Required at Year 1 (Assume Deposits Increase at the Rate of Inflation): To Replace 25% After Life of Project \$14,559 To Replace 50% After Life of Project \$29,118	
Deposit Required at Year 1 (Assume Deposits Increase at the Rate of Inflation): To Replace 25% After Life of Project \$14,559 To Replace 50% After Life of Project \$29,118	
To Replace 25% After Life of Project \$14,559 To Replace 50% After Life of Project \$29,118	
To Replace 25% After Life of Project \$14,559 To Replace 50% After Life of Project \$29,118	
To Replace 50% After Life of Project \$29,118	
To Replace 100% After Life of Project \$58,236	
Deposit Required at Year 25 (Assume Deposits Increase at the Rate of Inflation):	
To Replace 25% After Life of Project \$29,596	
To Replace 50% After Life of Project \$59,191	
To Replace 100% After Life of Project \$118,382	
Deposit Required at Year 50 (Assume Deposits Increase at the Rate of Inflation):	
To Replace 25% After Life of Project \$61,967	
To Replace 50% After Life of Project \$123,933	
To Replace 100% After Life of Project \$247,866	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$706,750	\$736,000	\$404,651	\$1,847,401
50% Replacement	\$1,413,500	\$736,000	\$404,651	\$2,554,151
100% Replacement	\$2,827,000	\$736,000	\$404,651	\$3,967,651
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$706,750	\$736,000	\$480,957	\$1,923,707
50% Replacement	\$1,413,500	\$736,000	\$480,957	\$2,630,457
100% Replacement	\$2,827,000	\$736,000	\$480,957	\$4,043,957
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$706,750	\$736,000	\$557,263	\$2,000,013
50% Replacement	\$1,413,500	\$736,000	\$557,263	\$2,706,763
100% Replacement	\$2,827,000	\$736,000	\$557,263	\$4,120,263
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$706,750	\$736,000	\$633,569	\$2,076,319
50% Replacement	\$1,413,500	\$736,000	\$633,569	\$2,783,069
100% Replacement	\$2,827,000	\$736,000	\$633,569	\$4,196,569

LIFE CYCLE COSTS:

	LIFE CICLE COSTS.																					
ľ	Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	
_																						

Capital Expenses:

Replacement Fund (For Funding Replacement of 25% of System):

Replacement i unu (i or i unung Replacemen	11 OI 23/0 OI 39	stem).																			
Deposits		\$14,559	\$14,996	\$15,446	\$15,909	\$16,386	\$16,878	\$17,384	\$17,906	\$18,443	\$18,996	\$19,566	\$20,153	\$20,758	\$21,380	\$22,022	\$22,683	\$23,363	\$24,064	\$24,786	,
Interest		\$0	\$437	\$900	\$1,390	\$1,909	\$2,458	\$3,038	\$3,651	\$4,297	\$4,980	\$5,699	\$6,457	\$7,255	\$8,096	\$8,980	\$9,910	\$10,888	\$11,915	\$12,995	
End of Year Balance		\$14,559	\$29,992	\$46,337	\$63,636	\$81,932	\$101,268	\$121,690	\$143,246	\$165,987	\$189,963	\$215,228	\$241,838	\$269,850	\$299,326	\$330,328	\$362,920	\$397,171	\$433,150	\$470,930	
Replacement Fund (For Funding Replacemer	nt of 50% of Sy	/stem):																			
Deposits		\$29,118	\$29,992	\$30,891	\$31,818	\$32,773	\$33,756	\$34,769	\$35,812	\$36,886	\$37,993	\$39,132	\$40,306	\$41,515	\$42,761	\$44,044	\$45,365	\$46,726	\$48,128	\$49,572	
Interest		\$0	\$874	\$1,799	\$2,780	\$3,818	\$4,916	\$6,076	\$7,301	\$8,595	\$9,959	\$11,398	\$12,914	\$14,510	\$16,191	\$17,960	\$19,820	\$21,775	\$23,830	\$25,989	,
End of Year Balance		\$29,118	\$59,983	\$92,674	\$127,273	\$163,863	\$202,535	\$243,380	\$286,493	\$331,973	\$379,925	\$430,455	\$483,675	\$539,701	\$598,653	\$660,656	\$725,841	\$794,342	\$866,300	\$941,861	
Replacement Fund (For Funding Replacemer	nt of 100% of S	System):																			
Deposits		\$58,236	\$59,983	\$61,783	\$63,636	\$65,545	\$67,512	\$69,537	\$71,623	\$73,772	\$75,985	\$78,265	\$80,613	\$83,031	\$85,522	\$88,087	\$90,730	\$93,452	\$96,256	\$99,143	
Interest		\$0	\$1,747	\$3,599	\$5,560	\$7,636	\$9,832	\$12,152	\$14,603	\$17,190	\$19,918	\$22,796	\$25,827	\$29,021	\$32,382	\$35,919	\$39,639	\$43,550	\$47,661	\$51,978	,
End of Year Balance		\$58,236	\$119,967	\$185,348	\$254,545	\$327,727	\$405,070	\$486,759	\$572,985	\$663,947	\$759,850	\$860,910	\$967,350	\$1,079,402	\$1,197,306	\$1,321,312	\$1,451,682	\$1,588,684	\$1,732,600	\$1,883,721	
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	j.
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	2
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$8,600	\$8,858	\$9,124	\$9,397	\$9,679	\$9,970	\$10,269	\$10,577	\$10,894	\$11,221	\$11,558	\$11,904	\$12,262	\$12,629	\$13,008	\$13,399	\$13,800	\$14,214	\$14,641	
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	:
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	,
Total O&M Expenses		\$14,720	\$15,162	\$15,616	\$16,085	\$16,567	\$17,065	\$17,576	\$18,104	\$18,647	\$19,206	\$19,782	\$20,376	\$20,987	\$21,617	\$22,265	\$22,933	\$23,621	\$24,330	\$25,060	1
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$8,093	\$8,336	\$8,586	\$8,843	\$9,109	\$9,382	\$9,663	\$9,953	\$10,252	\$10,560	\$10,876	\$11,203	\$11,539	\$11,885	\$12,241	\$12,609	\$12,987	\$13,377	\$13,778	;
4-Week Annual Pumping Duration ⁸		\$9,619	\$9,908	\$10,205	\$10,511	\$10,826	\$11,151	\$11,486	\$11,830	\$12,185	\$12,551	\$12,927	\$13,315	\$13,715	\$14,126	\$14,550	\$14,986	\$15,436	\$15,899	\$16,376	,
6-Week Annual Pumping Duration ⁸		\$11,145	\$11,480	\$11,824	\$12,179	\$12,544	\$12,920	\$13,308	\$13,707	\$14,118	\$14,542	\$14,978	\$15,428	\$15,890	\$16,367	\$16,858	\$17,364	\$17,885	\$18,421	\$18,974	į.
8-Week Annual Pumping Duration ⁸		\$12,671	\$13,052	\$13,443	\$13,846	\$14,262	\$14,690	\$15,130	\$15,584	\$16,052	\$16,533	\$17,029	\$17,540	\$18,066	\$18,608	\$19,167	\$19,742	\$20,334	\$20,944	\$21,572	
				1					1	1	1	1									Г
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
625 F20	ćac 205	627.004	627.007
\$25,529 \$14,128	\$26,295 \$15,318	\$27,084 \$16,566	\$27,897 \$17,876
\$510,588	\$15,518 \$552,201	\$595,851	\$641,623
JJ10,300	<i>ŞJJZ</i> ,201	<i>,,,,,,,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	JU11, 02 J
\$51,059	\$52,591	\$54,168	\$55,793
\$28,256	\$30,635	\$33,132	\$35,751
\$1,021,175	\$1,104,401	\$1,191,701	\$1,283,246
\$102,118	\$105,181	\$108,336	\$111,587
\$56,512	\$61,271	\$66,264	\$71,502
\$2,042,351	\$2,208,802	\$2,383,403	\$2,566,491
Á5 707	65 oco	<i>66 400</i>	<i>.</i>
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$15,080	\$15,533	\$15,999	\$16,478
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$25,812	\$26,586	\$27,384	\$28,205
\$14,191	\$14,617	\$15,055	\$15,507
\$16,867	\$17,373	\$17,894	\$18,431
\$19,543	\$20,130	\$20,733	\$21,355
\$22,219	\$22,886	\$23,572	\$24,280
÷-=)==0	÷==)000	÷=0,07 =	÷= 1)=00
20	21	22	23

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
\$28,734	\$29,596	\$30,483	\$31,398	\$32,340	\$33,310	\$34,309	\$35,339	\$36,399	\$37,491	\$38,615	\$39,774	\$40,967	\$42,196	\$43,462	\$44,766	\$46,109	\$47,492	\$48,917	\$50,384	\$51,896	\$53,453	\$55,056	\$56,708	\$58,409	\$60,162	\$61,967
\$19,249	\$20,688	\$22,197	\$23,777	\$25,432	\$27,165	\$28,980	\$30,878	\$32,865	\$34,943	\$37,116	\$39,388	\$41,763	\$44,245	\$46,838	\$49,547	\$52,376	\$55,331	\$58,415	\$61,635	\$64,996	\$68,503	\$72,161	\$75,978	\$79,958	\$84,110	\$88,438
\$689,605	\$739,889	\$792,569	\$847,744	\$905,516	\$965,992	\$1,029,281	\$1,095,498	\$1,164,761	\$1,237,195	\$1,312,926	\$1,392,088	\$1,474,818	\$1,561,259	\$1,651,558	\$1,745,871	\$1,844,356	\$1,947,179	\$2,054,511	\$2,166,531	\$2,283,423	\$2,405,379	\$2,532,596	\$2,665,282	\$2,803,650	\$2,947,921	\$3,098,326
\$57,467	\$59,191	\$60,967	\$62,796	\$64,680	\$66,620	\$68,619	\$70,677	\$72,798	\$74,982	\$77,231	\$79,548	\$81,934	\$84,392	\$86,924	\$89,532	\$92,218	\$94,984	\$97,834	\$100,769	\$103,792	\$106,906	\$110,113	\$113,416	\$116,819	\$120,323	\$123,933
\$38,497	\$41,376	\$44,393	\$47,554	\$50,865	\$54,331	\$57,959	\$61,757	\$65,730	\$69,886	\$74,232	\$78,776	\$83,525	\$88,489	\$93,676	\$99,094	\$104,752	\$110,661	\$116,831	\$123,271	\$129,992	\$137,005	\$144,323	\$151,956	\$159,917	\$168,219	\$176,875
\$1,379,210	\$1,479,778	\$1,585,138	\$1,695,488	\$1,811,032	\$1,931,983	\$2,058,561	\$2,190,995	\$2,329,523	\$2,474,390	\$2,625,853	\$2,784,176	\$2,949,636	\$3,122,517	\$3,303,117	\$3,491,742	\$3,688,712	\$3,894,358	\$4,109,023	\$4,333,062	\$4,566,846	\$4,810,757	\$5,065,193	\$5,330,565	\$5,607,301	\$5,895,843	\$6,196,651
\$114,934	\$118,382	\$121,934	\$125,592	\$129,359	\$133,240	\$137,237	\$141,355	\$145,595	\$149,963	\$154,462	\$159,096	\$163,869	\$168,785	\$173,848	\$179,064	\$184,436	\$189,969	\$195,668	\$201,538	\$207,584	\$213,811	\$220,226	\$226,833	\$233,638	\$240,647	\$247,866
\$76,995	\$82,753	\$88,787	\$95,108	\$101,729	\$108,662	\$115,919	\$123,514	\$131,460	\$139,771	\$148,463	\$157,551	\$167,051	\$176,978	\$187,351	\$198,187	\$209,505	\$221,323	\$233,661	\$246,541	\$259,984	\$274,011	\$288,645	\$303,912	\$319,834	\$336,438	\$353,751
\$2,758,420	\$2,959,555	\$3,170,275	\$3,390,975	\$3,622,064	\$3,863,966	\$4,117,123	\$4,381,991	\$4,659,046	\$4,948,780	\$5,251,705	\$5,568,352	\$5,899,272	\$6,245,035	\$6,606,234	\$6,983,485	\$7,377,425	\$7,788,716	\$8,218,045	\$8,666,124	\$9,133,692	\$9,621,514	\$10,130,386	\$10,661,130	\$11,214,601	\$11,791,686	\$12,393,302
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1,948	\$2,006	\$2,066	\$2,128
\$16,973	\$17,482	\$18,006	\$18,547	\$19,103	\$19,676	\$20,266	\$20,874	\$21,501	\$22,146	\$22,810	\$23,494	\$24,199	\$24,925	\$25,673	\$26,443	\$27,236	\$28,054	\$28,895	\$29,762	\$30,655	\$31,574	\$32,522	\$33,497	\$34,502	\$35,537	\$36,603
\$1,974 \$0 \$29,051	\$2,033 \$0 \$29,923	\$2,094 \$0 \$30,820	\$2,157 \$0 \$31,745	\$2,221 \$0 \$32,697	\$2,288 \$0 \$33,678	\$2,357 \$0 \$34,689	\$2,427 \$0 \$35,729	\$2,500 \$0 \$36,801	\$2,575 \$0 \$37,905	\$2,652 \$0 \$39,042	\$2,732 \$0 \$40,214	\$2,814 \$0	\$2,898 \$0 \$42,663	\$2,985 \$0 \$43,943	\$3,075 \$0 \$45,261	\$3,167 \$0 \$46,619	\$3,262 \$0 \$48,017	\$3,360 \$0 \$49,458	\$3,461 \$0 \$50,941	\$3,565 \$0 \$52,470	\$3,671 \$0 \$54,044	\$3,782 \$0 \$55,665	\$3,895 \$0 \$57,335	\$4,012 \$0 \$59,055	\$4,132 \$0 \$60,827	\$4,256 \$0 \$62,652
\$15,972	\$16,451	\$16,945	\$17,453	\$17,977	\$18,516	\$19,072	\$19,644	\$20,233	\$20,840	\$21,465	\$22,109	\$22,773	\$23,456	\$24,159	\$24,884	\$25,631	\$26,400	\$27,192	\$28,007	\$28,848	\$29,713	\$30,605	\$31,523	\$32,468	\$33,442	\$34,446
\$18,984	\$19,554	\$20,140	\$20,745	\$21,367	\$22,008	\$22,668	\$23,348	\$24,049	\$24,770	\$25,513	\$26,279	\$27,067	\$27,879	\$28,715	\$29,577	\$30,464	\$31,378	\$32,319	\$33,289	\$34,288	\$35,316	\$36,376	\$37,467	\$38,591	\$39,749	\$40,941
\$21,996	\$22,656	\$23,336	\$24,036	\$24,757	\$25,500	\$26,265	\$27,052	\$27,864	\$28,700	\$29,561	\$30,448	\$31,361	\$32,302	\$33,271	\$34,269	\$35,297	\$36,356	\$37,447	\$38,570	\$39,727	\$40,919	\$42,147	\$43,411	\$44,714	\$46,055	\$47,437
\$25,008	\$25,758	\$26,531	\$27,327	\$28,147	\$28,991	\$29,861	\$30,757	\$31,679	\$32,630	\$33,609	\$34,617	\$35,656	\$36,725	\$37,827	\$38,962	\$40,131	\$41,335	\$42,575	\$43,852	\$45,167	\$46,522	\$47,918	\$49,356	\$50,836	\$52,361	\$53,932
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 5, Design Flow Rate = 10 CFS

ASSUMPTIONS:				
Estimated Capit	al Cost:		\$3,165,000 Total Capita	al Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	1:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REP		COCTO		
SUMMARY REP	LACEIVIEN	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replace	ment Cost:		
To Replace	25%	After Life of Project		\$3,468,766
To Replace	50%	After Life of Project		\$6,937,531
To Replace	100%	After Life of Project	\$3,165,000	\$13,875,063
Disposal and Re	moval Cost	:		
To Replace	25%	After Life of Project		\$90,966
To Replace	50%	After Life of Project		\$181,932
To Replace	100%	After Life of Project	\$83,000	\$363,864
Total Replacem	ent Cost:			
To Replace	25%	After Life of Project		\$3,559,732

\$3,248,000

\$3,165,000

REPLACEMENT	FUND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$31,559	
To Replace	50%	After Life of Project	\$63,118	
To Replace	100%	After Life of Project	\$126,235	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$16,727	
To Replace	50%	After Life of Project	\$33,454	
To Replace	100%	After Life of Project	\$66,909	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$34,003	
To Replace	50%	After Life of Project	\$68,006	
To Replace	100%	After Life of Project	\$136,012	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$71,195	
To Replace	50%	After Life of Project	\$142,389	
To Replace	100%	After Life of Project	\$284,779	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$812,000	\$806,000	\$389,436	\$2,007,436
50% Replacement	\$1,624,000	\$806,000	\$389,436	\$2,819,436
100% Replacement	\$3,248,000	\$806,000	\$389,436	\$4,443,436
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$812,000	\$806,000	\$462,847	\$2,080,847
50% Replacement	\$1,624,000	\$806,000	\$462,847	\$2,892,847
100% Replacement	\$3,248,000	\$806,000	\$462,847	\$4,516,847
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$812,000	\$806,000	\$536,258	\$2,154,258
50% Replacement	\$1,624,000	\$806,000	\$536,258	\$2,966,258
100% Replacement	\$3,248,000	\$806,000	\$536,258	\$4,590,258
Assuming the Pumping Power Costs for an 8	-week Annual	Operating Dura	ation:	
25% Replacement	\$812,000	\$806,000	\$609,669	\$2,227,669
50% Replacement	\$1,624,000	\$806,000	\$609,669	\$3,039,669
100% Replacement	\$3,248,000	\$806,000	\$609,669	\$4,663,669

LIFE CYCLE COSTS:

LIFE CYCLE COSTS:																					
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	
																					-

Capital Expenses:

Replacement Fund (For Funding Replacement of 25% of System):

To Replace 50% After Life of Project

To Replace 100% After Life of Project

Replacement runu (roi runuing Replacemen	11 01 23/6 01 39	stem).																			
Deposits		\$16,727	\$17,229	\$17,746	\$18,278	\$18,827	\$19,391	\$19,973	\$20,572	\$21,190	\$21,825	\$22,480	\$23,154	\$23,849	\$24,564	\$25,301	\$26,060	\$26,842	\$27,648	\$28,477	
Interest		\$0	\$502	\$1,034	\$1,597	\$2,193	\$2,824	\$3,490	\$4,194	\$4,937	\$5,721	\$6,548	\$7,418	\$8,336	\$9,301	\$10,317	\$11,386	\$12,509	\$13,690	\$14,930	
End of Year Balance		\$16,727	\$34,458	\$53,238	\$73,113	\$94,133	\$116,348	\$139,812	\$164,579	\$190,706	\$218,252	\$247,280	\$277,852	\$310,037	\$343,902	\$379,521	\$416,967	\$456,318	\$497,655	\$541,062	ę
Replacement Fund (For Funding Replacemen	nt of 50% of Sy	/stem):																			
Deposits		\$33,454	\$34,458	\$35,492	\$36,557	\$37,653	\$38,783	\$39,946	\$41,145	\$42,379	\$43,650	\$44,960	\$46,309	\$47,698	\$49,129	\$50,603	\$52,121	\$53,684	\$55,295	\$56,954	
Interest		\$0	\$1,004	\$2,067	\$3,194	\$4,387	\$5,648	\$6,981	\$8,389	\$9,875	\$11,442	\$13,095	\$14,837	\$16,671	\$18,602	\$20,634	\$22,771	\$25,018	\$27,379	\$29,859	
End of Year Balance		\$33,454	\$68,916	\$106,475	\$146,226	\$188,266	\$232,697	\$279,624	\$329,158	\$381,411	\$436,504	\$494,559	\$555,705	\$620,074	\$687,805	\$759,042	\$833,934	\$912,636	\$995,310	\$1,082,124	\$1
Replacement Fund (For Funding Replacemen	nt of 100% of S	System):																			
Deposits		\$66,909	\$68,916	\$70,984	\$73,113	\$75,306	\$77,566	\$79,893	\$82,289	\$84,758	\$87,301	\$89,920	\$92,617	\$95,396	\$98,258	\$101,206	\$104,242	\$107,369	\$110,590	\$113,908	:
Interest		\$0	\$2,007	\$4,135	\$6,389	\$8,774	\$11,296	\$13,962	\$16,777	\$19,749	\$22,885	\$26,190	\$29,674	\$33,342	\$37,204	\$41,268	\$45,543	\$50,036	\$54,758	\$59,719	
End of Year Balance		\$66,909	\$137,832	\$212,951	\$292,452	\$376,532	\$465,394	\$559,248	\$658,315	\$762,823	\$873,008	\$989,118	\$1,111,409	\$1,240,147	\$1,375,610	\$1,518,083	\$1,667,868	\$1,825,273	\$1,990,621	\$2,164,247	\$2
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$10,000	\$10,300	\$10,609	\$10,927	\$11,255	\$11,593	\$11,941	\$12,299	\$12,668	\$13,048	\$13,439	\$13,842	\$14,258	\$14,685	\$15,126	\$15,580	\$16,047	\$16,528	\$17,024	
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
Total O&M Expenses		\$16,120	\$16,604	\$17,102	\$17,615	\$18,143	\$18,687	\$19,248	\$19,826	\$20,420	\$21,033	\$21,664	\$22,314	\$22,983	\$23,673	\$24,383	\$25,114	\$25,868	\$26,644	\$27,443	
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$7,789	\$8,022	\$8,263	\$8,511	\$8,766	\$9,029	\$9,300	\$9,579	\$9,867	\$10,163	\$10,467	\$10,781	\$11,105	\$11,438	\$11,781	\$12,135	\$12,499	\$12,874	\$13,260	
4-Week Annual Pumping Duration ⁸		\$9,257	\$9,535	\$9,821	\$10,115	\$10,419	\$10,731	\$11,053	\$11,385	\$11,726	\$12,078	\$12,441	\$12,814	\$13,198	\$13,594	\$14,002	\$14,422	\$14,855	\$15,300	\$15,759	
6-Week Annual Pumping Duration ⁸		\$10,725	\$11,047	\$11,378	\$11,720	\$12,071	\$12,433	\$12,806	\$13,191	\$13,586	\$13,994	\$14,414	\$14,846	\$15,292	\$15,750	\$16,223	\$16,709	\$17,211	\$17,727	\$18,259	
8-Week Annual Pumping Duration ⁸		\$12,193	\$12,559	\$12,936	\$13,324	\$13,724	\$14,135	\$14,560	\$14,996	\$15,446	\$15,910	\$16,387	\$16,878	\$17,385	\$17,906	\$18,444	\$18,997	\$19,567	\$20,154	\$20,758	
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$7,119,463

\$14,238,927

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
\$29,331	\$30,211	\$31,118	\$32,051
\$16,232	\$17,599	\$19,033	\$20,538
\$586,625	\$634,435	\$684,585	\$737,174
ć50.000	¢60,422	ćca 225	664 402
\$58,662 \$32,464	\$60,422 \$35,197	\$62,235 \$38,066	\$64,102 \$41,075
\$1,173,250	\$1,268,870	\$1,369,171	\$1,474,348
<i>J</i> JJJJJJJJJJJJJ	<i>Ş1,200,070</i>	<i>Ş1,303,171</i>	<i>Ş</i> 1,474,340
\$117,325	\$120,845	\$124,470	\$128,204
\$64,927	\$70,395	\$76,132	\$82,150
\$2,346,500	\$2,537,739	\$2,738,342	\$2,948,696
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$17,535	\$18,061	\$18,603	\$19,161
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$28,267	\$29,115	\$29,988	\$30,888
\$13,658	\$14,067	\$14,489	\$14,924
\$16,232	\$16,719	\$17,221	\$17,737
\$18,807	\$19,371	\$19,952	\$20,551
\$21,381	\$22,023	\$22,683	\$23,364
			<u> </u>
20	21	22	23

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
\$33,013 \$22,115	\$34,003 \$23,769	\$35,023 \$25,502	\$36,074 \$27,318	\$37,156 \$29,220	\$38,271 \$31,211	\$39,419 \$33,295	\$40,601 \$35,477	\$41,819 \$37,759	\$43,074 \$40,147	\$44,366 \$42,643	\$45,697 \$45,253	\$47,068 \$47,982	\$48,480 \$50,833	\$49,934 \$53,813	\$51,433 \$56,925	\$52,975 \$60,176	\$54,565 \$63,571	\$56,202 \$67,115	\$57,888 \$70,814	\$59,624 \$74,675	\$61,413 \$78,704	\$63,256 \$82,908	\$65,153 \$87,293	\$67,108 \$91,866	\$69,121 \$96,635	\$101,608
\$792,302	\$850,074	\$910,599	\$973,991	\$1,040,366	\$1,109,848	\$1,182,562	\$1,258,640	\$1,338,219	\$1,421,440	\$1,508,449	\$1,599,399	\$1,694,449	\$1,793,763	\$1,897,510	\$2,005,868	\$2,119,020	\$2,237,155	\$2,360,471	\$2,489,173	\$2,623,473	\$2,763,590	\$2,909,753	\$3,062,199	\$3,221,173	\$3,386,929	\$3,559,732
\$66,025 \$44,230 \$1,584,604	\$68,006 \$47,538 \$1,700,148	\$70,046 \$51,004 \$1,821,198	\$72,147 \$54,636 \$1,947,982	\$74,312 \$58,439 \$2,080,733	\$76,541 \$62,422 \$2,219,696	\$78,837 \$66,591 \$2,365,125	\$81,203 \$70,954 \$2,517,281	\$83,639 \$75,518 \$2,676,438	\$86,148 \$80,293 \$2,842,879	\$88,732 \$85,286 \$3,016,898	\$91,394 \$90,507 \$3,198,799	\$94,136 \$95,964 \$3,388,899	\$96,960 \$101,667 \$3,587,526	\$99,869 \$107,626 \$3,795,021	\$102,865 \$113,851 \$4,011,736	\$105,951 \$120,352 \$4,238,039	\$109,130 \$127,141 \$4,474,310	\$112,403 \$134,229 \$4,720,943	\$115,776 \$141,628 \$4,978,347	\$119,249 \$149,350 \$5,246,946	\$122,826 \$157,408 \$5,527,180	\$126,511 \$165,815 \$5,819,507	\$130,306 \$174,585 \$6,124,398	\$134,216 \$183,732 \$6,442,346	\$193,270	\$203,216
\$132,050 \$88,461 \$3,169,207	\$136,012 \$95,076 \$3,400,295	\$140,092 \$102,009 \$3,642,396	\$144,295 \$109,272 \$3,895,963	\$148,624 \$116,879 \$4,161,466	\$153,082 \$124,844 \$4,439,392	\$157,675 \$133,182 \$4,730,249	\$162,405 \$141,907 \$5,034,562	\$167,277 \$151,037 \$5,352,876	\$172,296 \$160,586 \$5,685,758	\$177,465 \$170,573 \$6,033,795	\$182,789 \$181,014 \$6,397,598	\$191,928	\$193,920 \$203,334 \$7,175,052	\$199,738 \$215,252 \$7,590,042	\$205,730 \$227,701 \$8,023,473	\$211,902 \$240,704 \$8,476,079	\$218,259 \$254,282 \$8,948,620	\$224,807 \$268,459 \$9,441,886	\$231,551 \$283,257 \$9,956,693	\$238,498 \$298,701 \$10,493,892	\$245,652 \$314,817 \$11,054,361	\$253,022 \$331,631 \$11,639,014	\$260,613 \$349,170 \$12,248,797		\$276,484 \$386,541 \$13,547,717	\$406,432
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239		
\$2,605 \$987	\$2,683 \$1,016	\$2,764 \$1,047	\$2,847 \$1,078	\$2,932 \$1,111	\$3,020 \$1,144	\$3,111 \$1,178	\$3,204 \$1,214	\$3,300 \$1,250	\$3,399 \$1,288	\$3,501 \$1,326	\$3,606 \$1,366	\$3,714 \$1,407	\$3,826 \$1,449	\$3,940 \$1,493	\$4,059 \$1,537	\$4,180 \$1,584	\$4,306 \$1,631	\$4,435 \$1,680	\$4,568 \$1,730	\$4,705 \$1,782	\$4,846 \$1,836	\$4,992 \$1,891	\$5,141 \$1,948	\$5,296 \$2,006		
\$19,736	\$20,328	\$20,938	\$21,566	\$22,213	\$22,879	\$23,566	\$24,273	\$25,001	\$25,751	\$26,523	\$27,319	. ,	\$28,983	\$29,852	\$30,748	\$31,670	\$32,620	\$33,599	\$34,607	\$35,645	\$36,715	\$37,816	\$38,950	\$40,119		. ,
\$1,974 \$0	\$2,033 \$0	\$2,094 \$0	\$2,157 \$0	\$2,221 \$0	\$2,288 \$0	\$2,357 \$0	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,652 \$0	\$2,732 \$0	\$2,814 \$0	\$2,898 \$0	\$2,985 \$0	\$3,075 \$0	\$3,167 \$0	\$3,262 \$0	\$3,360 \$0	\$3,461 \$0	\$3,565 \$0	\$3,671 \$0	\$3,782 \$0	\$3,895 \$0	\$4,012 \$0	\$4,132 \$0	
\$31,814	\$32,769	\$33,752	\$34,764	\$35,807	\$36,881	\$37,988	\$39,127	\$40,301	\$41,510	\$42,756	\$44,038	-	\$46,720	\$48,122	\$49,566	\$51,052	\$52,584	\$54,162	\$55,786	\$57,460	\$59,184	\$60,959	\$62,788	\$64,672		
\$15,372	\$15,833	\$16,308	\$16,797	\$17,301	\$17,820	\$18,355	\$18,905	\$19,472	\$20,057	\$20,658	\$21,278	\$21,916	\$22,574	\$23,251	\$23,949	\$24,667	\$25,407	\$26,169	\$26,954	\$27,763	\$28,596	\$29,454	\$30,337	\$31,248	\$32,185	\$33,150
\$18,269	\$18,817	\$19,382	\$19,963	\$20,562	\$21,179	\$21,815	\$22,469	\$23,143	\$23,837	\$24,552	\$25,289	\$26,048	\$26,829	\$27,634	\$28,463	\$29,317	\$30,196	\$31,102	\$32,035	\$32,997	\$33,986	\$35,006	\$36,056	\$37,138		
\$21,167	\$21,802	\$22,456	\$23,130	\$23,824	\$24,538	\$25,275	\$26,033	\$26,814	\$27,618	\$28,447	\$29,300		\$31,084	\$32,017	\$32,978	\$33,967	\$34,986	\$36,035	\$37,116	\$38,230	\$39,377	\$40,558	\$41,775			
\$24,065	\$24,787	\$25,530	\$26,296	\$27,085	\$27,898	\$28,734	\$29,597	\$30,484	\$31,399	\$32,341	\$33,311	\$34,310	\$35,340	\$36,400	\$37,492	\$38,617	\$39,775	\$40,968	\$42,198	\$43,463	\$44,767	\$46,110	\$47,494	\$48,919	\$50,386	\$51,898
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 1, Design Flow Rate = 20 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$2,687,000 Total Capita	l Cost
Interest on Repla	acement Fu	und:	3.00%	
Rate of Inflation			3.00%	
Project Design Li	fe:		50 Years	
			2	
SUMMARY REPL	ACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Project	t Replacer	nent Cost:		
To Replace	25%	After Life of Project		\$2,944,889
To Replace	50%	After Life of Project		\$5,889,778
To Replace	100%	After Life of Project	\$2,687,000	\$11,779,555
Disposal and Rer	noval Cost	:		
To Replace	25%	After Life of Project		\$73,430
To Replace	50%	After Life of Project		\$146,861
To Replace	100%	After Life of Project	\$67,000	\$293,722
Total Replaceme	ent Cost:			
To Replace	25%	After Life of Project		\$3,018,319
To Replace	50%	After Life of Project		\$6,036,639
To Replace	100%	After Life of Project	\$2,754,000	\$12,073,277

\$2,687,000

sume Equal Deposit Made E After Life of Project After Life of Project After Life of Project	ach Year): \$26,759 \$53,518 \$107,036	
After Life of Project	\$53,518	
After Life of Project	\$107,036	
ssume Deposits Increase a	t the Rate of Inflation):	
After Life of Project	\$14,183	
After Life of Project	\$28,366	
After Life of Project	\$56,732	
Assume Deposits Increase	at the Rate of Inflation):	
After Life of Project	\$28,831	
After Life of Project	\$57,663	
After Life of Project	\$115,325	
Assume Deposits Increase	at the Rate of Inflation):	
After Life of Project	\$60,366	
After Life of Project	\$120,733	
After Life of Project	\$241,466	
	After Life of Project After Life of Project After Life of Project Assume Deposits Increase After Life of Project After Life of Project After Life of Project Assume Deposits Increase After Life of Project After Life of Project	After Life of Project \$28,366 After Life of Project \$56,732 Assume Deposits Increase at the Rate of Inflation): After Life of Project After Life of Project \$28,831 After Life of Project \$57,663 After Life of Project \$115,325 Assume Deposits Increase at the Rate of Inflation): Assume Deposits Increase at the Rate of Inflation): After Life of Project \$60,366 After Life of Project \$120,733

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5oth Year Equal to Future Val	ue of Replacem	ent Cost		
FOTAL LONG-TERM COST SUMMARY:				
PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$688,500	\$711,000	\$851,322	\$2,250,822
50% Replacement	\$1,377,000	\$711,000	\$851,322	\$2,939,322
100% Replacement	\$2,754,000	\$711,000	\$851,322	\$4,316,322
Assuming the Pumping Power Costs for a 4-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$688,500	\$711,000	\$1,012,619	\$2,412,119
50% Replacement	\$1,377,000	\$711,000	\$1,012,619	\$3,100,619
100% Replacement	\$2,754,000	\$711,000	\$1,012,619	\$4,477,619
Assuming the Pumping Power Costs for a 6-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$688,500	\$711,000	\$1,173,916	\$2,573,416
50% Replacement	\$1,377,000	\$711,000	\$1,173,916	\$3,261,916
100% Replacement	\$2,754,000	\$711,000	\$1,173,916	\$4,638,916
Assuming the Pumping Power Costs for an 8-	week Annual O	perating Durat	ion:	
25% Replacement	\$688,500	\$711,000	\$1,335,214	\$2,734,714
50% Replacement	\$1,377,000	\$711,000	\$1,335,214	\$3,423,214
100% Replacement	\$2,754,000	\$711,000	\$1,335,214	\$4,800,214

Input Cells - Assumed or Given Values

LIFE CYCLE COSTS:

Year 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19

Capital Expenses:

Replacement Fund (For Funding Replacement	of 25% of Sys	tem):																			
Deposits		\$14,183		\$15,047	\$15,498	\$15,963	\$16,442	\$16,935	\$17,443	\$17,967	\$18,506	\$19,061	\$19,633	\$20,222	\$20,828	\$21,453	\$22,097	\$22,760	\$23,443	\$24,146	
Interest		\$0		\$877	\$1,354	\$1,860	\$2,394	\$2,960	\$3,556	\$4,186	\$4,851	\$5,552	\$6,290	\$7,068	\$7,886	\$8,748	\$9,654	\$10,606	\$11,607	\$12,659	
End of Year Balance		\$14,183	\$29,217	\$45,141	\$61,993	\$79,816	\$98,653	\$118,548	\$139,547	\$161,701	\$185,057	\$209,670	\$235,593	\$262,882	\$291,597	\$321,798	\$353,549	\$386,915	\$421,965	\$458,770	,
Replacement Fund (For Funding Replacement	of 50% of Svs	tem).																			
Deposits	. 01 30/0 01 343	\$28.366	\$29,217	\$30,094	\$30.997	\$31,926	\$32,884	\$33,871	\$34.887	\$35,933	\$37,011	\$38,122	\$39.265	\$40,443	\$41.657	\$42,906	\$44.194	\$45.519	\$46,885	\$48,292	,
Interest		\$0	1 - 7	\$1,753	\$2,708	\$3,720	\$4,789	\$5,919	\$7,113	\$8,373	\$9,702	\$11,103	\$12,580	\$14,136	\$15,773	\$17,496	\$19,308	\$21,213	\$23,215	\$25,318	
End of Year Balance		\$28,366		\$90,281	\$123,986	\$159,632	\$197,305	\$237,095	\$279,095	\$323,401	\$370,115	\$419,340	\$471,185	\$525,764	\$583,194	\$643,596	\$707,098	\$773,830	\$843,930	\$917,540	
Replacement Fund (For Funding Replacement	of 100% of 5	(stopp):																			
Deposits	. 01 100% OF Sy	\$56,732	\$58,434	\$60,187	\$61,993	\$63,853	\$65,768	\$67,741	\$69,774	\$71,867	\$74,023	\$76,244	\$78,531	\$80,887	\$83,313	\$85,813	\$88,387	\$91,039	\$93,770	\$96,583	2
Interest		\$30,732		\$3,506	\$5.417	\$7,439	\$9,578	\$11,838	\$14.226	\$16.746	\$19,404	\$22,207	\$25.160	\$28,271	\$31,546	\$34,992	\$38,557	\$42,426	\$46,430	\$50,636	
End of Year Balance		\$56,732		\$180,562	\$247,972	\$319,264	\$394,610	\$474,190	\$558,190	\$646,802	\$740,229	\$838,680	\$942,371	\$1,051,529	\$1,166,388	\$1,287,193	\$1,414,196	\$1,547,660	\$1,687,860	\$1,835,079	
			. ,	. ,		. ,	. ,		. ,		. ,	. ,									
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	3
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,24	/
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$853	L
Maintenance and Small Repairs ⁷		\$8,100	\$8,343	\$8,593	\$8,851	\$9,117	\$9,390	\$9,672	\$9,962	\$10,261	\$10,569	\$10,886	\$11,212	\$11,549	\$11,895	\$12,252	\$12,620	\$12,998	\$13,388	\$13,790	J
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	2
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	J
Total O&M Expenses		\$14,220	\$14,647	\$15,086	\$15,539	\$16,005	\$16,485	\$16,979	\$17,489	\$18,013	\$18,554	\$19,110	\$19,684	\$20,274	\$20,883	\$21,509	\$22,154	\$22,819	\$23,503	\$24,209	J
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$17,026	\$17,537	\$18,063	\$18,605	\$19,163	\$19,738	\$20,330	\$20,940	\$21,569	\$22,216	\$22,882	\$23,569	\$24,276	\$25,004	\$25,754	\$26,527	\$27,322	\$28,142	\$28,986	ز
4-Week Annual Pumping Duration ⁸		\$20,252	\$20,860	\$21,486	\$22,130	\$22,794	\$23,478	\$24,182	\$24,908	\$25,655	\$26,425	\$27,218	\$28,034	\$28,875	\$29,741	\$30,634	\$31,553	\$32,499	\$33,474	\$34,478	3
6-Week Annual Pumping Duration ⁸		\$23,478	\$24,183	\$24,908	\$25,655	\$26,425	\$27,218	\$28,034	\$28,875	\$29,742	\$30,634	\$31,553	\$32,499	\$33,474	\$34,479	\$35,513	\$36,578	\$37,676	\$38,806	\$39,970	J
8-Week Annual Pumping Duration ⁸		\$26.704	\$27,505	\$28,331	\$29,180	\$30,056	\$30,958	\$31,886	\$32,843	\$33,828	\$34,843	\$35,888	\$36,965	\$38,074	\$39,216	\$40,393	\$41,604	\$42,853	\$44.138	\$45,462	
		<i>4</i> =0,704	<i>4</i> 27,505	<i>4</i> 20,001	<i>410,100</i>	<i>200,000</i>	<i>400,000</i>	<i>401,000</i>	<i>202,040</i>	<i>400,010</i>	<i>40 .,040</i>	<i>ç</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	<i>400,000</i>	<i>400,074</i>	<i>ç</i> ,,,10	÷.0,050	÷ .2,504	÷.=,555	<i></i>	÷,40	
Voor	0	1	2	3	4	ŗ	6	7	•	9	10	11	12	13	14	15	16	17	18	19	Γ
Year	U	1	2	3	4	5	o	'	ð	э	10	11	12	13	14	15	10	17	19	19	

NOTES:
1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

8) Assumes pumping power costs, or power rates, increase at the assumed rate of inflation.

Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of

20	21	22	23
\$24,870	\$25,616	\$26,385	\$27,176
\$13,763	\$14,922	\$16,138	\$17,414
\$497,403	\$537,941	\$580,464	\$625,055
\$49,740	\$51,233	\$52,769	\$54,353
\$27,526	\$29,844	\$32,276	\$34,828
\$994,806	\$1,075,883	\$1,160,929	\$1,250,109
\$99,481	\$102,465	\$105,539	\$108,705
\$55,052	\$59,688	\$64,553	\$69,656
\$1,989,612	\$2,151,765	\$2,321,857	\$2,500,218
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$14,203	\$14,630	\$15,068	\$15,520
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$24,935	\$25,683	\$26,453	\$27,247
\$29,856	\$30,752	\$31,674	\$32,624
\$35,513	\$36,578	\$37,675	\$38,806
\$35,513	\$30,378	\$43,677	\$38,800
\$41,169 \$46,826	\$42,404 \$48,231	\$43,677 \$49,678	\$44,987 \$51,168
340,62 6	240,231	\$45,078	Ş51,168
20	21	22	23

\$27,992	\$28,831	\$29,696	\$30,587	\$31,505	\$32,450	\$33,423	\$34,426	\$35,459	\$36,523	\$37,618	\$38,747	\$39,909	\$41,107	\$42,340	\$43,610	\$44,918	\$46,266	\$47,654	\$49,083	\$50,556	\$52,073	\$53,635	j
\$18,752	\$20,154	\$21,623	\$23,163	\$24,776	\$26,464	\$28,231	\$30,081	\$32,016	\$34,041	\$36,157	\$38,371	\$40,684	\$43,102	\$45,628	\$48,267	\$51,024	\$53,902	\$56,907	\$60,044	\$63,318	\$66,734	\$70,298	1
\$671,798	\$720,783	\$772,103	\$825,853	\$882,133	\$941,047	\$1,002,702	\$1,067,209	\$1,134,684	\$1,205,248	\$1,279,023	\$1,356,141	\$1,436,734	\$1,520,943	\$1,608,911	\$1,700,788	\$1,796,730	\$1,896,898	\$2,001,459	\$2,110,586	\$2,224,459	\$2,343,266	\$2,467,198	3
\$55,983	\$57,663	\$59,393	\$61,174	\$63,010	\$64,900	\$66,847	\$68,852	\$70,918	\$73,045	\$75,237	\$77,494	\$79,819	\$82,213	\$84,680	\$87,220	\$89,837	\$92,532	\$95,308	\$98,167	\$101,112	\$104,145	\$107,270)
\$37,503	\$40,308	\$43,247	\$46,326	\$49,551	\$52,928	\$56,463	\$60,162	\$64,033	\$68,081	\$72,315	\$76,741	\$81,368	\$86,204	\$91,257	\$96,535	\$102,047	\$107,804	\$113,814	\$120,088	\$126,635	\$133,468	\$140,596	
\$1,343,596	\$1,441,566	\$1,544,206	\$1,651,706	\$1,764,267	\$1,882,095	\$2,005,404	\$2,134,419	\$2,269,369	\$2,410,495	\$2,558,047	\$2,712,282	\$2,873,469	\$3,041,886	\$3,217,823	\$3,401,577	\$3,593,461	\$3,793,797	\$4,002,918	\$4,221,172	\$4,448,919	\$4,686,532	\$4,934,398	
\$111,966	\$115,325	\$118,785	\$122,349	\$126,019	\$129,800	\$133,694	\$137,704	\$141,836	\$146,091	\$150,473	\$154,988	\$159,637	\$164,426	\$169,359	\$174,440	\$179,673	\$185,063	\$190,615	\$196,334	\$202,224	\$208,290	\$214,539	,
\$75,007	\$80,616	\$86,494	\$92,652	\$99,102	\$105,856	\$112,926	\$120,324	\$128,065	\$136,162	\$144,630	\$153,483	\$162,737	\$172,408	\$182,513	\$193,069	\$204,095	\$215,608	\$227,628	\$240,175	\$253,270	\$266,935	\$281,192	
\$2,687,191	\$2,883,132	\$3,088,411	\$3,303,412	\$3,528,534	\$3,764,189	\$4,010,808	\$4,268,837	\$4,538,738	\$4,820,991	\$5,116,094	\$5,424,564	\$5,746,938	\$6,083,772	\$6,435,645	\$6,803,154	\$7,186,922	\$7,587,592	\$8,005,835	\$8,442,344	\$8,897,838	\$9,373,063	\$9,868,794	\$
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	
\$15,986	\$16,466	\$16,960	\$17,468	\$17,992	\$18,532	\$19,088	\$19,661	\$20,251	\$20,858	\$21,484	\$22,128	\$22,792	\$23,476	\$24,180	\$24,906	\$25,653	\$26,423	\$27,215	\$28,032	\$28,873	\$29,739	\$30,631	
\$1,974 \$0	\$2,033 \$0	\$2,094 \$0	\$2,157 \$0	\$2,221 \$0	\$2,288 \$0	\$2,357 \$0	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,652 \$0	\$2,732 \$0	\$2,814 \$0	\$2,898 \$0	\$2,985 \$0	\$3,075 \$0	\$3,167 \$0	\$3,262 \$0	\$3,360 \$0	\$3,461 \$0	\$3,565 \$0	\$3,671 \$0	\$3,782 \$0	
\$28,064	\$0 \$28,906	\$ 29,774	\$30,667	\$31,587	\$32,534	\$33,510	\$34,516	\$35,551	\$36,618	\$37,716	\$38,848	\$40,013	\$41,214	\$42,450	\$43,723	\$45,035	\$46,386	\$47,778	\$49,211	\$50,687	\$52,208	\$53,774	
\$33,603	\$34,611	\$35,650	\$36,719	\$37,821	\$38,955	\$40,124	\$41,328	\$42,567	\$43,844	\$45,160	\$46,515	\$47,910	\$49,347	\$50,828	\$52,353	\$53,923	\$55,541	\$57,207	\$58,923	\$60,691	\$62,512	\$64,387	
\$39,970	\$41,169	\$42,404	\$43,676	\$44,986	\$46,336	\$47,726	\$49,158	\$50,633	\$52,152	\$53,716	\$55,328	\$56,987	\$58,697	\$60,458	\$62,272	\$64,140	\$66,064	\$68,046	\$70,087	\$72,190	\$74,356	\$76,586	
\$46,337	\$47,727	\$49,158	\$50,633	\$52,152	\$53,717	\$55,328	\$56,988	\$58,698	\$60,459	\$62,272	\$64,141	\$66,065	\$68,047	\$70,088	\$72,191	\$74,356	\$76,587	\$78,885	\$81,251	\$83,689	\$86,200	\$88,786	
\$52,703	\$54,284	\$55,913	\$57,590	\$59,318	\$61,097	\$62,930	\$64,818	\$66,763	\$68,766	\$70,829	\$72,954	\$75,142	\$77,396	\$79,718	\$82,110	\$84,573	\$87,110	\$89,724	\$92,415	\$95,188	\$98,043	\$100,985	
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	Γ

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

47	48	49	50
\$55,244	\$56,901	\$58,608	\$60,366
\$74,016	\$77,894	\$81,938	\$86,154
\$2,596,458	\$2,731,253	\$2,871,799	\$3,018,319
\$110,488	\$113,802	\$117,216	\$120,733
\$148,032	\$155,788	\$163,875	\$172,308
\$5,192,917	\$5,462,507	\$5,743,598	\$6,036,639
\$220,975	\$227,604	\$234,433	\$241,466
\$296,064	\$311,575	\$327,750	\$344,616
\$10,385,833	\$10,925,013	\$11,487,196	\$12,073,277
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$31,550	\$32,496	\$33,471	\$34,475
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$55 <i>,</i> 388	\$57 <i>,</i> 049	\$58,761	\$60,523
\$66,319	\$68,308	\$70,358	\$72,468
\$78,884	\$81,250	\$83 <i>,</i> 688	\$86,199
\$91,449	\$94,193	\$97 <i>,</i> 018	\$99,929
\$104,014	\$107,135	\$110,349	\$113,659
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 2, Design Flow Rate = 20 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$3,051,000 Total Capit	al Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	-		CONNENT COST	1010112 0001
To Replace	25%	After Life of Project		\$3,343,824
To Replace	50%	After Life of Project		\$6,687,649
To Replace	100%	After Life of Project	\$3,051,000	\$13,375,297
Disposal and Re	moval Cost			
To Replace	25%	After Life of Project		\$83,294
To Replace	50%	After Life of Project		\$166,588
To Replace	100%	After Life of Project	\$76,000	\$333,177
Total Replacem	ent Cost:			
To Replace	25%	After Life of Project		\$3,427,119
				1.

\$3,127,000

Annual Deposit I	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$30,383	
To Replace	50%	After Life of Project	\$60,766	
To Replace	100%	After Life of Project	\$121,532	
eposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$16,104	
To Replace	50%	After Life of Project	\$32,208	
To Replace	100%	After Life of Project	\$64,416	
Deposit Require	d at Year 2	5 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$32,736	
To Replace	50%	After Life of Project	\$65,472	
To Replace	100%	After Life of Project	\$130,945	
Deposit Require	d at Year 5	0 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$68,542	
To Replace	50%	After Life of Project	\$137,085	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$781,750	\$766,000	\$857,843	\$2,405,593
50% Replacement	\$1,563,500	\$766,000	\$857,843	\$3,187,343
100% Replacement	\$3,127,000	\$766,000	\$857,843	\$4,750,843
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$781,750	\$766,000	\$1,020,381	\$2,568,131
50% Replacement	\$1,563,500	\$766,000	\$1,020,381	\$3,349,881
100% Replacement	\$3,127,000	\$766,000	\$1,020,381	\$4,913,381
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$781,750	\$766,000	\$1,182,919	\$2,730,669
50% Replacement	\$1,563,500	\$766,000	\$1,182,919	\$3,512,419
100% Replacement	\$3,127,000	\$766,000	\$1,182,919	\$5,075,919
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ition:	
25% Replacement	\$781,750	\$766,000	\$1,345,457	\$2,893,207
50% Replacement	\$1,563,500	\$766,000	\$1,345,457	\$3,674,957
100% Replacement	\$3,127,000	\$766,000	\$1,345,457	\$5,238,457

LIFE CYCLE COST

LIFE CYCLE COSTS:		-	-								-						-				_
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

To Replace 100% After Life of Project

REPLACEMENT FUND SUMMARY

To Replace 50% After Life of Project

To Replace 100% After Life of Project

· · · · · · · · · · · · · · · · · · ·																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$3,051,000																							
Replacement Fund (For Funding Replaceme	nt of 25% of Sy	stem):																						
Deposits		\$16,104	\$16,587	\$17,085	\$17,597	\$18,125	\$18,669	\$19,229	\$19,806	\$20,400	\$21,012	\$21,642	\$22,292	\$22,961	\$23,649	\$24,359	\$25,090	\$25,842	\$26,618	\$27,416	\$28,239	\$29,086	\$29,958	\$30,857
Interest		\$0	\$483	\$995	\$1,538	\$2,112	\$2,719	\$3,360	\$4,038	\$4,753	\$5,508	\$6,304	\$7,142	\$8,025	\$8,955	\$9,933	\$10,961	\$12,043	\$13,180	\$14,373	\$15,627	\$16,943	\$18,324	
End of Year Balance		\$16,104	\$33,174	\$51,254	\$70,389	\$90,626	\$112,014	\$134,604	\$158,448	\$183,601	\$210,121	\$238,067	\$267,501	\$298,487	\$331,091	\$365,382	\$401,433	\$439,319	\$479,116	\$520,905	\$564,771	\$610,800	\$659,082	\$709,712
Replacement Fund (For Funding Replaceme	nt of 50% of Sy	stem):																						
Deposits		\$32,208	\$33,174	\$34,170	\$35,195	\$36,250	\$37,338	\$38,458	\$39,612	\$40,800	\$42,024	\$43,285	\$44,584	\$45,921	\$47,299	\$48,718	\$50,179	\$51,685	\$53,235	\$54,832	\$56,477	\$58,171	\$59,917	\$61,714
Interest		\$0	\$966	\$1,990	\$3,075	\$4,223	\$5,438	\$6,721	\$8,076	\$9,507	\$11,016	\$12,607	\$14,284	\$16,050	\$17,909	\$19,865	\$21,923	\$24,086	\$26,359	\$28,747	\$31,254	\$33,886	\$36,648	\$39,545
End of Year Balance		\$32,208	\$66,349	\$102,509	\$140,779	\$181,252	\$224,028	\$269,207	\$316,895	\$367,202	\$420,243	\$476,135	\$535,003	\$596,974	\$662,182	\$730,765	\$802,867	\$878,637	\$958,231	\$1,041,811	\$1,129,542	\$1,221,600	\$1,318,164	\$1,419,423
Replacement Fund (For Funding Replaceme	nt of 100% of S	ystem):																						
Deposits		\$64,416	\$66,349	\$68,339	\$70,389	\$72,501	\$74,676	\$76,916	\$79,224	\$81,601	\$84,049	\$86,570	\$89,167	\$91,842	\$94,597	\$97,435	\$100,358	\$103,369	\$106,470	\$109,664	\$112,954	\$116,343	\$119,833	\$123,428
Interest		\$0	\$1,932	\$3,981	\$6,151	\$8,447	\$10,875	\$13,442	\$16,152	\$19,014	\$22,032	\$25,215	\$28,568	\$32,100	\$35,818	\$39,731	\$43,846	\$48,172	\$52,718	\$57,494	\$62,509	\$67,773	\$73,296	\$79,090
End of Year Balance		\$64,416	\$132,697	\$205,017	\$281,557	\$362,505	\$448,056	\$538,414	\$633,790	\$734,405	\$840,485	\$952,270	\$1,070,005	\$1,193,947	\$1,324,363	\$1,461,529	\$1,605,733	\$1,757,275	\$1,916,463	\$2,083,621	\$2,259,084	\$2,443,199	\$2,636,328	\$2,838,846
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$9,200	\$9,476	\$9,760	\$10,053	\$10,355	\$10.665	\$10,985	\$11,315	\$11,654	\$12,004	\$12,364	\$12,735	\$13,117	\$13,511	\$13,916	\$14,333	\$14,763	\$15,206	\$15,662	\$16,132	\$16,616	\$17,115	\$17,628
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$15,320	\$15,780	\$16,253	\$16,741	\$17,243	\$17,760	\$18,293	\$18,842	\$19,407	\$19,989	\$20,589	\$21,206	\$21,843	\$22,498	\$23,173	\$23,868	\$24,584	\$25,322	\$26,081	\$26,864	\$27,670	\$28,500	\$29,355
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$17,157	\$17,672	\$18,202	\$18,748	\$19,310	\$19,890	\$20,486	\$21,101	\$21,734	\$22,386	\$23,057	\$23,749	\$24,462	\$25,195	\$25,951	\$26,730	\$27,532	\$28,358	\$29,208	\$30,085	\$30,987	\$31,917	\$32,874
4-Week Annual Pumping Duration ⁸		\$20,408	\$21,020	\$21,650	\$22,300	\$22,969	\$23,658	\$24,368	\$25,099	\$25,852	\$26,627	\$27,426	\$28,249	\$29,096	\$29,969	\$30,868	\$31,794	\$32,748	\$33,731	\$34,743	\$35,785	\$36,858	\$37,964	\$39,103
6-Week Annual Pumping Duration ⁸		\$23,658	\$24,368	\$25,099	\$25,852	\$26,628	\$27,427	\$28,249	\$29,097	\$29,970	\$30,869	\$31,795	\$32,749	\$33,731	\$34,743	\$35,785	\$36,859	\$37,965	\$39,104	\$40,277	\$41,485	\$42,730	\$44,012	\$45,332
8-Week Annual Pumping Duration ⁸		\$26,909	\$27,716	\$28,548	\$29,404	\$30,286	\$31,195	\$32,131	\$33,095	\$34,088	\$35,110	\$36,164	\$37,249	\$38,366	\$39,517	\$40,702	\$41,924	\$43,181	\$44,477	\$45,811	\$47,185	\$48,601	\$50,059	
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

\$274,169

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$6,854,237

\$13,708,474

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study
Life Cycle Cost Analysis (Cont.)

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
\$31,783 \$21,291 \$762,786	\$32,736 \$22,884 \$818,406	\$33,718 \$24,552 \$876,676	\$34,730 \$26,300 \$937,706	\$35,772 \$28,131 \$1,001,609	\$36,845 \$30,048 \$1,068,502	\$37,950 \$32,055 \$1,138,508	\$39,089 \$34,155 \$1,211,752	\$40,261 \$36,353 \$1,288,366	\$41,469 \$38,651 \$1,368,486	\$42,713 \$41,055 \$1,452,254	\$43,995 \$43,568 \$1,539,816	\$45,315 \$46,194 \$1,631,325	\$46,674 \$48,940 \$1,726,939	\$48,074 \$51,808 \$1,826,821	\$49,516 \$54,805 \$1,931,142	\$51,002 \$57,934 \$2,040,079		\$54,108 \$64,614 \$2,272,535	\$55,731 \$68,176 \$2,396,443	\$57,403 \$71,893 \$2,525,739	\$59,125 \$75,772 \$2,660,637	\$60,899 \$79,819 \$2,801,355	\$62,726 \$84,041 \$2,948,121	\$64,608 \$88,444 \$3,101,173	\$66,546 \$93,035 \$3,260,754	\$68,542 \$97,823 \$3,427,119
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\$63,565 \$42,583 \$1,525,571	\$65,472 \$45,767 \$1,636,811	\$67,437 \$49,104 \$1,753,352	\$69,460 \$52,601 \$1,875,412	\$71,543 \$56,262 \$2,003,218	\$73,690 \$60,097 \$2,137,004	\$75,900 \$64,110 \$2,277,015	\$78,178 \$68,310 \$2,423,503	\$80,523 \$72,705 \$2,576,731	\$82,939 \$77,302 \$2,736,971	\$85,427 \$82,109 \$2,904,507	\$87,989 \$87,135 \$3,079,632	\$90,629 \$92,389 \$3,262,650	\$93,348 \$97,879 \$3,453,877	\$96,148 \$103,616 \$3,653,642	\$99,033 \$109,609 \$3,862,284	\$102,004 \$115,869 \$4,080,157	\$122,405	\$108,216 \$129,229 \$4,545,070	\$111,462 \$136,352 \$4,792,885	\$114,806 \$143,787 \$5,051,478	\$118,251 \$151,544 \$5,321,273	\$121,798 \$159,638 \$5,602,709	\$168,081	\$129,216 \$176,887 \$6,202,345	\$133,092 \$186,070 \$6,521,507	\$195,645
\$127,131 \$85,165 \$3,051,143	\$130,945 \$91,534 \$3,273,622	\$134,873 \$98,209 \$3,506,704	\$138,919 \$105,201 \$3,750,824	\$143,087 \$112,525 \$4,006,436	\$147,380 \$120,193 \$4,274,009	\$151,801 \$128,220 \$4,554,030	\$156,355 \$136,621 \$4,847,006	\$161,046 \$145,410 \$5,153,462	\$165,877 \$154,604 \$5,473,942	\$170,853 \$164,218 \$5,809,014	\$175,979 \$174,270 \$6,159,263	\$184,778	\$186,696 \$195,759 \$6,907,755	\$192,297 \$207,233 \$7,307,284	\$198,066 \$219,219 \$7,724,569	\$204,008 \$231,737 \$8,160,314	\$244,809	\$216,432 \$258,458 \$9,090,141	\$222,925 \$272,704 \$9,585,770	\$229,613 \$287,573 \$10,102,955	\$236,501 \$303,089 \$10,642,545	\$243,596 \$319,276 \$11,205,417	\$250,904 \$336,163 \$11,792,484	\$258,431 \$353,775 \$12,404,689	\$266,184 \$372,141 \$13,043,014	\$391,290
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	. ,	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584		\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1,948	\$2,006	\$2,066	
\$18,157	\$18,702	\$19,263	\$19,841	\$20,436	\$21,049	\$21,680	\$22,331	\$23,001	\$23,691	\$24,401	\$25,134	\$25,888	\$26,664	\$27,464	\$28,288	\$29,137	\$30,011	\$30,911	\$31,838	\$32,794	\$33,777	\$34,791	\$35,834	\$36,909	\$38,017	\$39,157
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782		\$4,012	\$4,132	\$4,256
\$0 \$30,235	\$0 \$31,142	\$0 \$32,077	\$0 \$33,039	\$0 \$34,030	\$0 \$35,051	\$0 \$36,103	\$0 \$37,186	\$0 \$38,301	\$0 \$39,450	\$0 \$40,634	\$0 \$41,853	\$0 \$43,108	\$0 \$44,402	\$0 \$45,734	\$0 \$47,106	\$0 \$48,519	\$0 \$49,974	\$0 \$51,474	\$0 \$53,018	\$0 \$54,608	\$0 \$56,247	\$0 \$57,934		\$0 \$61,462	\$0 \$63,306	
\$33,861	\$34,876	\$35,923	\$37,000	\$38,110	\$39,254	\$40,431	\$41,644	\$42,894	\$44,180	\$45,506	\$46,871	\$48,277	\$49,725	\$51,217	\$52,754	\$54,336	\$55,966	\$57,645	\$59,375	\$61,156	\$62,991	\$64,880	\$66,827	\$68,832	\$70,896	\$73,023
\$40,276	\$41,484	\$42,729	\$44,011	\$45,331	\$46,691	\$48,092	\$49,535	\$51,021	\$52,551	\$54,128	\$55,752		\$59,147	\$60,921	\$62,749	\$64,631		\$68,568	\$70,625	\$72,743	\$74,926	\$77,173		\$81,873	\$84,329	
\$46,692	\$48,093	\$49,535	\$51,021	\$52,552	\$54,129	\$55,753	\$57,425	\$59,148	\$60,922	\$62,750	\$64,632	\$66,571	\$68,569	\$70,626	\$72,744	\$74,927	\$77,175	\$79,490	\$81,874	\$84,331	\$86,861	\$89,466	\$92,150	\$94,915	\$97,762	\$100,695
\$53,107	\$54,701	\$56,342	\$58,032	\$59,773	\$61,566	\$63,413	\$65,316	\$67,275	\$69,293	\$71,372	\$73,513	\$75,719	\$77,990	\$80,330	\$82,740	\$85,222	\$87,779	\$90,412	\$93,124	\$95,918	\$98,796	\$101,759	\$104,812	\$107,957	\$111,195	\$114,531
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 3, Design Flow Rate = 20 CFS

To Replace 100% After Life of Project

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$2,677,000 Total Cap	ital Cost
Interest on Repla	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design Li	ife:		50 Years	
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Project	ct Replace	ment Cost:		
To Replace	25%	After Life of Project		\$2,933,929
To Replace	50%	After Life of Project		\$5,867,858
To Replace	100%	After Life of Project	\$2,677,000	\$11,735,716
Disposal and Rei	moval Cost	:		
To Replace	25%	After Life of Project		\$73,430
To Replace	50%	After Life of Project		\$146,861

\$67,000

\$2,744,000

\$293,722

\$3,007,360

\$6,014,719

\$12,029,438

REPLACEMENT	FUND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$26,662	
To Replace	50%	After Life of Project	\$53,323	
To Replace	100%	After Life of Project	\$106,647	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$14,132	
To Replace	50%	After Life of Project	\$28,263	
To Replace	100%	After Life of Project	\$56,526	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$28,727	
To Replace	50%	After Life of Project	\$57,453	
To Replace	100%	After Life of Project	\$114,907	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$60,147	
To Replace	50%	After Life of Project	\$120,294	
To Replace	100%	After Life of Project	\$240,589	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 50th Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$686,000	\$706,000	\$830,673	\$2,222,673
50% Replacement	\$1,372,000	\$706,000	\$830,673	\$2,908,673
100% Replacement	\$2,744,000	\$706,000	\$830,673	\$4,280,673
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$686,000	\$706,000	\$988,041	\$2,380,041
50% Replacement	\$1,372,000	\$706,000	\$988,041	\$3,066,041
100% Replacement	\$2,744,000	\$706,000	\$988,041	\$4,438,041
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$686,000	\$706,000	\$1,145,409	\$2,537,409
50% Replacement	\$1,372,000	\$706,000	\$1,145,409	\$3,223,409
100% Replacement	\$2,744,000	\$706,000	\$1,145,409	\$4,595,409
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$686,000	\$706,000	\$1,302,777	\$2,694,777
50% Replacement	\$1,372,000	\$706,000	\$1,302,777	\$3,380,777
100% Replacement	\$2,744,000	\$706,000	\$1,302,777	\$4,752,777

LIFE CYCLE COSTS:

Total Replacement Cost:

i	LIFE CICLE COSIS:																					_
	Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	1
							•															-

LIFE CYCLE COSTS:																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$2,677,000																							
Replacement Fund (For Funding Replacement	nt of 25% of Sy	stem):																						
Deposits		\$14,132	\$14,556	\$14,992	\$15,442	\$15,905	\$16,382	\$16,874	\$17,380	\$17,901	\$18,439	\$18,992	\$19,561	\$20,148	\$20,753	\$21,375	\$22,017	\$22,677	\$23,357	\$24,058	\$24,780	\$25,523	\$26,289	\$27,078
Interest		\$0	\$424	\$873	\$1,349	\$1,853	\$2,386	\$2,949	\$3,544	\$4,171	\$4,833	\$5,532	\$6,267	\$7,042	\$7,858	\$8,716	\$9,619	\$10,568	\$11,565	\$12,613	\$13,713	\$14,868	\$16,080	\$17,351
End of Year Balance		\$14,132	\$29,111	\$44,977	\$61,768	\$79,526	\$98,294	\$118,117	\$139,041	\$161,113	\$184,385	\$208,909	\$234,737	\$261,928	\$290,538	\$320,630	\$352,265	\$385,510	\$420,433	\$457,104	\$495,597	\$535,988	\$578,357	\$622,785
Replacement Fund (For Funding Replacement	nt of 50% of Sy	stem):																						
Deposits		\$28,263	\$29,111	\$29,984	\$30,884	\$31,810	\$32,765	\$33,748	\$34,760	\$35,803	\$36,877	\$37,983	\$39,123	\$40,297	\$41,505	\$42,751	\$44,033	\$45,354	\$46,715	\$48,116	\$49,560	\$51,046	\$52,578	\$54,155
Interest		\$0	\$848	\$1,747	\$2,699	\$3,706	\$4,772	\$5,898	\$7,087	\$8,342	\$9,667	\$11,063	\$12,535	\$14,084	\$15,716	\$17,432	\$19,238	\$21,136	\$23,131	\$25,226	\$27,426	\$29,736	\$32,159	\$34,701
End of Year Balance		\$28,263	\$58,222	\$89,953	\$123,536	\$159,052	\$196,589	\$236,234	\$278,081	\$322,227	\$368,771	\$417,817	\$469,475	\$523,855	\$581,076	\$641,259	\$704,530	\$771,020	\$840,866	\$914,208	\$991,194	\$1,071,976	\$1,156,713	\$1,245,570
Replacement Fund (For Funding Replacement	nt of 100% of S	ystem):																						
Deposits		\$56,526	\$58,222	\$59,969	\$61,768	\$63,621	\$65,530	\$67,495	\$69,520	\$71,606	\$73,754	\$75,967	\$78,246	\$80,593	\$83,011	\$85,501	\$88,066	\$90,708	\$93,430	\$96,232	\$99,119	\$102,093	\$105,156	\$108,310
Interest		\$0	\$1,696	\$3,493	\$5,397	\$7,412	\$9,543	\$11,795	\$14,174	\$16,685	\$19,334	\$22,126	\$25,069	\$28,168	\$31,431	\$34,865	\$38,476	\$42,272	\$46,261	\$50,452	\$54,852	\$59,472	\$64,319	\$69,403
End of Year Balance		\$56,526	\$116,444	\$179,907	\$247,072	\$318,105	\$393,178	\$472,468	\$556,163	\$644,454	\$737,541	\$835,634	\$938,949	\$1,047,711	\$1,162,153	\$1,282,519	\$1,409,061	\$1,542,041	\$1,681,731	\$1,828,416	\$1,982,388	\$2,143,952	\$2,313,427	\$2,491,140
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$8.000	\$8,240	\$8,487	\$8.742	\$9,004	\$9,274	\$9,552	\$9,839	\$10,134	\$10,438	\$10,751	\$11,074	\$11,406	\$11,748	\$12,101	\$12,464	\$12,838	\$13,223	\$13,619	\$14.028	\$14,449	\$14,882	\$15,329
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$14,120	\$14,544	\$14,980	\$15,429	\$15,892	\$16,369	\$16,860	\$17,366	\$17,887	\$18,423	\$18,976	\$19,545	\$20,132	\$20,736	\$21,358	\$21,998	\$22,658	\$23,338	\$24,038	\$24,760	\$25,502	\$26,267	\$27,055
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$16,613	\$17,112	\$17,625	\$18,154	\$18,699	\$19,260	\$19,837	\$20,432	\$21,045	\$21,677	\$22,327	\$22,997	\$23,687	\$24,397	\$25,129	\$25,883	\$26,660	\$27,460	\$28,283	\$29,132	\$30,006	\$30,906	\$31,833
4-Week Annual Pumping Duration ⁸		\$19,761	\$20,354	\$20,964	\$21,593	\$22,241	\$22,908	\$23,595	\$24,303	\$25,032	\$25,783	\$26,557	\$27,354	\$28,174	\$29,019	\$29,890	\$30,787	\$31,710	\$32,662	\$33,641	\$34,651	\$35,690	\$36,761	\$37,864
6-Week Annual Pumping Duration ⁸		\$22,908	\$23,595	\$24,303	\$25,032	\$25,783	\$26,557	\$27,354	\$28,174	\$29,019	\$29,890	\$30,787	\$31,710	\$32,662	\$33,641	\$34,651	\$35,690	\$36,761	\$37,864	\$39,000	\$40,170	\$41,375	\$42,616	\$43,894
8-Week Annual Pumping Duration ⁸		\$26,056	\$26,837	\$27,642	\$28,472	\$29,326	\$30,206	\$31,112	\$32,045	\$33,006	\$33,997	\$35,016	\$36,067	\$37,149	\$38,263	\$39,411	\$40,594	\$41,812	\$43,066	\$44,358	\$45,689	\$47,059	\$48,471	\$49,925
		<u> </u>	<u> </u>	<u> </u>	<u> </u>		I	<u> </u>	<u> </u>				<u> </u>	<u> </u>	<u> </u>		I	<u> </u>						
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
\$27,890 \$18,684 \$669,359	\$28,727 \$20,081 \$718,166	\$29,588 \$21,545 \$769,299	\$30,476 \$23,079 \$822,854	\$31,390 \$24,686 \$878,930	\$32,332 \$26,368 \$937,630	\$33,302 \$28,129 \$999,061	\$34,301 \$29,972 \$1,063,334	\$35,330 \$31,900 \$1,130,565	\$36,390 \$33,917 \$1,200,871	\$37,482 \$36,026 \$1,274,379	\$38,606 \$38,231 \$1,351,217	\$39,764 \$40,537 \$1,431,518	\$40,957 \$42,946 \$1,515,421	\$42,186 \$45,463 \$1,603,069	\$43,452 \$48,092 \$1,694,613	\$44,755 \$50,838 \$1,790,207	\$46,098 \$53,706 \$1,890,011	\$47,481 \$56,700 \$1,994,192	\$48,905 \$59,826 \$2,102,923	\$50,372 \$63,088 \$2,216,383	\$51,884 \$66,491 \$2,334,758	\$53,440 \$70,043 \$2,458,240	\$55,043 \$73,747 \$2,587,031	\$56,695 \$77,611 \$2,721,336	\$58,395 \$81,640 \$2,861,372	\$60,147 \$85,841 \$3,007,360
			. ,	. ,																					., ,	
\$55,780 \$37,367 \$1,338,717	\$57,453 \$40,162 \$1,436,332	\$59,177 \$43,090 \$1,538,598	\$60,952 \$46,158 \$1,645,709	\$62,781 \$49,371 \$1,757,861	\$64,664 \$52,736 \$1,875,261	\$66,604 \$56,258 \$1,998,122	\$68,602 \$59,944 \$2,126,668	\$70,660 \$63,800 \$2,261,129	\$72,780 \$67,834 \$2,401,743	\$74,963 \$72,052 \$2,548,758	\$77,212 \$76,463 \$2,702,433	\$79,529 \$81,073 \$2,863,035	\$81,915 \$85,891 \$3,030,841	\$84,372 \$90,925 \$3,206,138	\$86,903 \$96,184 \$3,389,226	\$89,510 \$101,677 \$3,580,413	\$92,196 \$107,412 \$3,780,021	\$94,961 \$113,401 \$3,988,383	\$97,810 \$119,651 \$4,205,845	\$100,745 \$126,175 \$4,432,765	\$103,767 \$132,983 \$4,669,515	\$106,880 \$140,085 \$4,916,480	\$110,086 \$147,494 \$5,174,061	\$113,389 \$155,222 \$5,442,672	\$116,791 \$163,280 \$5,722,742	\$120,294 \$171,682 \$6,014,719
\$111,560 \$74,734 \$2,677,434	\$114,907 \$80,323 \$2,872,663	\$118,354 \$86,180 \$3,077,197	\$121,904 \$92,316 \$3,291,417	\$125,561 \$98,743 \$3,515,721	\$129,328 \$105,472 \$3,750,521	\$133,208 \$112,516 \$3,996,245	\$137,204 \$119,887 \$4,253,337	\$141,321 \$127,600 \$4,522,257	\$145,560 \$135,668 \$4,803,485	\$149,927 \$144,105 \$5,097,517	\$154,425 \$152,926 \$5,404,867	\$159,058 \$162,146 \$5,726,070	\$163,829 \$171,782 \$6,061,682	\$168,744 \$181,850 \$6,412,276	\$173,806 \$192,368 \$6,778,451	\$179,021 \$203,354 \$7,160,825	\$184,391 \$214,825 \$7,560,041	\$189,923 \$226,801 \$7,976,766	\$195,621 \$239,303 \$8,411,689	\$201,489 \$252,351 \$8,865,529	\$207,534 \$265,966 \$9,339,029	\$213,760 \$280,171 \$9,832,960	\$220,173 \$294,989 \$10,348,121	\$226,778 \$310,444 \$10,885,343	\$233,581 \$326,560 \$11,445,485	
\$6,513 \$2,605	\$6,708 \$2,683	\$6,909 \$2,764	\$7,117	\$7,330 \$2,932	\$7,550 \$2,030	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286 \$3,714	\$9,564	\$9,851	\$10,147 \$4,059	\$10,451	\$10,765 \$4,306	\$11,088	\$11,420	\$11,763 \$4,705	\$12,116	\$12,479	\$12,854 \$5,141	\$13,239 \$5,296	\$13,636	
\$2,605 \$987	\$2,085	\$2,764 \$1,047	\$2,847 \$1,078	\$2,952 \$1,111	\$3,020 \$1,144	\$3,111 \$1,178	\$3,204 \$1,214	\$3,300 \$1,250	\$3,399 \$1,288	\$3,501 \$1,326	\$3,606 \$1,366		\$3,826 \$1,449	\$3,940 \$1,493	\$4,039	\$4,180 \$1,584	\$4,506	\$4,435 \$1,680	\$4,568 \$1,730	\$4,705	\$4,846 \$1,836	\$4,992 \$1,891	\$1,948	\$2,006	\$5,455 \$2,066	
\$15,789	\$16,262	\$16,750	\$17,253	\$17,770	\$18,303	\$18,853	\$19,418	\$20,001	\$20,601	\$21,219	\$21,855		\$23,186	\$23,882	\$24,598	\$25,336	\$26,096	\$26,879	\$27,686	\$28,516	\$29,372	\$30,253	\$31,160	\$32,095	\$33,058	
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732		\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	. ,	\$3,565	\$3,671	\$3,782	\$3,895	\$4,012	\$4,132	. ,
\$0 \$27,867	\$0 \$28,703	\$0 \$29,564	\$0 \$30,451	\$0 \$31,365	\$0 \$32,306	\$0 \$33,275	\$0 \$34,273	\$0 \$35,301	\$0 \$36,360	\$0 \$37,451	\$0 \$38,575	\$0 \$39,732	\$0 \$40,924	\$0 \$42,151	\$0 \$43,416	\$0 \$44,718	\$0 \$46,060	\$0 \$47,442	\$0 \$48,865	\$0 \$50,331	\$0 \$51,841	\$0 \$53,396	\$0 \$54,998	\$0 \$56,648	\$0 \$58,347	\$0 \$60,098
\$32,788	\$33,772	\$34,785	\$35,828	\$36,903	\$38,010	\$39,151	\$40,325	\$41,535	\$42,781	\$44,064	\$45,386	\$46,748	\$48,150	\$49,595	\$51,083	\$52,615	\$54,194	\$55,820	\$57,494	\$59,219	\$60,996	\$62,825	\$64,710	\$66,651	\$68,651	\$70,711
\$39,000	\$40,170	\$41,375	\$42,616	\$43,895	\$45,211	\$46,568	\$47,965	\$49,404	\$50,886	\$52,412	\$53,985	\$55,604	\$57,272	\$58,991	\$60,760	\$62,583	\$64,461	\$66,394	\$68,386	\$70,438	\$72,551	\$74,727	\$76,969	\$79,278	\$81,657	\$84,106
\$45,211	\$46,568	\$47,965	\$49,404	\$50,886	\$52,412	\$53,985	\$55,604	\$57,272	\$58,990	\$60,760	\$62,583	\$64,460	\$66,394	\$68,386	\$70,438	\$72,551	\$74,727	\$76,969	\$79,278	\$81,657	\$84,106	\$86,630	\$89,228	\$91,905	\$94,662	\$97,502
\$51,423	\$52,966	\$54,555	\$56,191	\$57,877	\$59,613	\$61,402	\$63,244	\$65,141	\$67,095	\$69,108	\$71,181	\$73,317	\$75,516	\$77,782	\$80,115	\$82,519	\$84,994	\$87,544	\$90,170	\$92,875	\$95,662	\$98,532	\$101,487	\$104,532	\$107,668	\$110,898
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 4, Design Flow Rate = 20 CFS

ASSUMPTIONS:					
Estimated Capita	al Cost:		\$3,826,000	Total Capita	l Cost
Interest on Repla	acement F	und:	3.00%		
Rate of Inflation	:		3.00%		
Project Design Li	ife:		50	Years	
SUMMARY REPI	ACEMENT	COSTS	CURREN	IT COST ²	FUTURE COST ³
Estimated Project			CONNEN	11 0051	TOTORE COST
To Replace	25%	After Life of Project			\$4,193,206
To Replace	50%	After Life of Project			\$8,386,412
To Replace	100%	After Life of Project	\$3.826.000		\$16,772,824

\$96,000

\$3,922,000

REPLACEMENT I	FUND SUIV	IMARY		
Annual Deposit I	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$38,108	
To Replace	50%	After Life of Project	\$76,215	
To Replace	100%	After Life of Project	\$152,430	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$20,198	
To Replace	50%	After Life of Project	\$40,397	
To Replace	100%	After Life of Project	\$80,793	
Deposit Require	d at Year 2	5 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	To Replace 25% After Life of Project To Replace 50% After Life of Project To Replace 100% After Life of Project eposit Required at Year 1 (Assume Deposits Incr To Replace 25% After Life of Project To Replace 25% After Life of Project To Replace 50% After Life of Project To Replace 100% After Life of Project To Replace 100% After Life of Project eposit Required at Year 25 (Assume Deposits Incr To Replace 25% After Life of Project To Replace 25% After Life of Project To Replace 25% After Life of Project To Replace 50% To Replace 100% After Life of Project To Replace 100% After Life of Project		\$41,059	
To Replace	50%	After Life of Project	\$82,118	
To Replace	100%	After Life of Project	\$164,236	
Deposit Require	d at Year 5	0 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$85,968	
Annual Deposit Required (Assume Equal Deposit N To Replace 25% After Life of Project To Replace 50% After Life of Project To Replace 50% After Life of Project To Replace 100% After Life of Project Deposit Required at Year 1 (Assume Deposits Incr To Replace 25% To Replace 25% After Life of Project To Replace 25% After Life of Project To Replace 50% After Life of Project To Replace 100% After Life of Project Deposit Required at Year 25 (Assume Deposits Incr To Replace 25% To Replace 25% After Life of Project To Replace 50% After Life of Project To Replace 100% After Life of Project Deposit Required at Year 50 (Assume Deposits Incr To Replace 25% Deposit Required at Year 50 (Assume Deposits Incr To Replace 25% To Replace 25% After Life of Project			\$171,937	
Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image Image <thimage< th=""> <thi< td=""><td>After Life of Project</td><td>\$343,874</td><td></td></thi<></thimage<>		After Life of Project	\$343,874	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$980,500	\$881,000	\$781,767	\$2,643,267
50% Replacement	\$1,961,000	\$881,000	\$781,767	\$3,623,767
100% Replacement	\$3,922,000	\$881,000	\$781,767	\$5,584,767
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$980,500	\$881,000	\$929,830	\$2,791,330
50% Replacement	\$1,961,000	\$881,000	\$929,830	\$3,771,830
100% Replacement	\$3,922,000	\$881,000	\$929,830	\$5,732,830
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$980,500	\$881,000	\$1,077,892	\$2,939,392
50% Replacement	\$1,961,000	\$881,000	\$1,077,892	\$3,919,892
100% Replacement	\$3,922,000	\$881,000	\$1,077,892	\$5,880,892
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ition:	
25% Replacement	\$980,500	\$881,000	\$1,225,955	\$3,087,455
50% Replacement	\$1,961,000	\$881,000	\$1,225,955	\$4,067,955
100% Replacement	\$3,922,000	\$881,000	\$1,225,955	\$6,028,955

LIFE CYCLE COSTS	S:
------------------	----

Year 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19	U	IFE CYCLE COSTS:																
	Y	/ear	0	1	2	3	4	5	6	7	8	9	11	13	15	16		

Disposal and Removal Cost:

 To Replace
 25%
 After Life of Project

 To Replace
 50%
 After Life of Project

To Replace 100% After Life of Project

Total Replacement Cost: To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

LIFE CYCLE COSTS:																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$3,826,000																							
Replacement Fund (For Funding Replacement	nt of 25% of Sys	tem):																						
Deposits		\$20,198	\$20,804	\$21,428	\$22,071	\$22,733	\$23,415	\$24,118	\$24,841	\$25,587	\$26,354	\$27,145	\$27,959	\$28,798	\$29,662	\$30,552	\$31,468	\$32,412	\$33,385	\$34,386	\$35,418	\$36,480	\$37,575	\$38,702
Interest		\$0		\$1,248	\$1,929	\$2,649	\$3,410	\$4,215	\$5,065	\$5,962	\$6,908	\$7,906	\$8,958	\$10,065	\$11,231	\$12,458	\$13,748	\$15,105	\$16,530	\$18,028	\$19,600	\$21,251	\$22,983	\$24,799
End of Year Balance		\$20,198	\$41,608	\$64,285	\$88,285	\$113,667	\$140,492	\$168,825	\$198,731	\$230,279	\$263,542	\$298,593	\$335,510	\$374,373	\$415,266	\$458,276	\$503,493	\$551,010	\$600,925	\$653,339	\$708,357	\$766,088	\$826,645	\$890,147
Replacement Fund (For Funding Replacement	nt of 50% of Sys	tem):																						
Deposits		\$40,397	\$41,608	\$42,857	\$44,142	\$45,467	\$46,831	\$48,236	\$49,683	\$51,173	\$52,708	\$54,290	\$55,918	\$57,596	\$59,324	\$61,103	\$62,937	\$64,825	\$66,769	\$68,773	\$70,836	\$72,961	\$75,150	\$77,404
Interest		\$0	\$1,212	\$2,497	\$3,857	\$5,297	\$6,820	\$8,430	\$10,129	\$11,924	\$13,817	\$15,813	\$17,916	\$20,131	\$22,462	\$24,916	\$27,497	\$30,210	\$33,061	\$36,055	\$39,200	\$42,501	\$45,965	\$49,599
End of Year Balance		\$40,397	\$83,217	\$128,570	\$176,570	\$227,334	\$280,984	\$337,650	\$397,462	\$460,559	\$527,084	\$597,186	\$671,020	\$748,747	\$830,533	\$916,552	\$1,006,985	\$1,102,020	\$1,201,850	\$1,306,678	\$1,416,714	\$1,532,176	\$1,653,291	\$1,780,294
Replacement Fund (For Funding Replacement	nt of 100% of Sy	stem):																						
Deposits		\$80,793	\$83,217	\$85,714	\$88,285	\$90,933	\$93,661	\$96,471	\$99,365	\$102,346	\$105,417	\$108,579	\$111,837	\$115,192	\$118,648	\$122,207	\$125,873	\$129,649	\$133,539	\$137,545	\$141,671	\$145,922	\$150,299	\$154,808
Interest		\$0	\$2,424	\$4,993	\$7,714	\$10,594	\$13,640	\$16,859	\$20,259	\$23,848	\$27,634	\$31,625	\$35,831	\$40,261	\$44,925	\$49,832	\$54,993	\$60,419	\$66,121	\$72,111	\$78,401	\$85,003	\$91,931	\$99,197
End of Year Balance		\$80,793	\$166,434	\$257,141	\$353,140	\$454,667	\$561,969	\$675,299	\$794,924	\$921,118	\$1,054,168	\$1,194,372	\$1,342,040	\$1,497,493	\$1,661,065	\$1,833,104	\$2,013,971	\$2,204,039	\$2,403,699	\$2,613,355	\$2,833,427	\$3,064,352	\$3,306,581	\$3,560,587
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$11,500	\$11,845	\$12,200	\$12,566	\$12,943	\$13.332	\$13,732	\$14,144	\$14,568	\$15,005	\$15,455	\$15,919	\$16,396	\$16.888	\$17,395	\$17,917	\$18.454	\$19,008	\$19,578	\$20.165	\$20,770	\$21,393	\$22,035
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$17,620	\$18,149	\$18,693	\$19,254	\$19,831	\$20,426	\$21,039	\$21,670	\$22,320	\$22,990	\$23,680	\$24,390	\$25,122	\$25,876	\$26,652	\$27,451	\$28,275	\$29,123	\$29,997	\$30,897	\$31,824	\$32,778	\$33,762
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$15,635	\$16,104	\$16,588	\$17,085	\$17,598	\$18,126	\$18,669	\$19,230	\$19,806	\$20,401	\$21,013	\$21,643	\$22,292	\$22,961	\$23,650	\$24,359	\$25,090	\$25,843	\$26,618	\$27,417	\$28,239	\$29,086	\$29,959
4-Week Annual Pumping Duration ⁸		\$18,597	\$19,154	\$19,729	\$20,321	\$20,931	\$21,559	\$22,205	\$22,871	\$23,558	\$24,264	\$24,992	\$25,742	\$26,514	\$27,310	\$28,129	\$28,973	\$29,842	\$30,737	\$31,659	\$32,609	\$33,588	\$34,595	\$35,633
6-Week Annual Pumping Duration ⁸		\$21,558	\$22,205	\$22,871	\$23,557	\$24,264	\$24,991	\$25,741	\$26,513	\$27,309	\$28,128	\$28,972	\$29,841	\$30,736	\$31,658	\$32,608	\$33,586	\$34,594	\$35,632	\$36,701	\$37.802	\$38,936	\$40,104	\$41,307
8-Week Annual Pumping Duration ⁸		\$24,519	\$25,255	\$26,012	\$26,793	\$27,596	\$28,424	\$29,277	\$30,155	\$31,060	\$31,992	\$32,952	\$33,940	\$34,958	\$36,007	\$37,087	\$38,200	\$39,346	\$40,526	\$41,742	\$42,994	\$44,284	\$45,613	\$46,981
	r																r					r	——	
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$105,214 \$210,427

\$420,855

\$4,298,420

\$8,596,840

\$17,193,679

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study	
Life Cycle Cost Analysis (Cont.)	

r				1				1												1			1			
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
¢20.002	¢44.050	¢42.201	642 550	¢44.966	Ć46 242	ć 17 500	ć 40 027	650 407	652.012	652 572	ĆEE 400	656 025	Ć50 540	¢60.207	¢62.405	¢62.060	ĆCE 000	¢67.064	¢c0.000	ć71 007	674 457	¢76 202	670 (72)	601 000	¢02.464	ÉRE OCO
\$39,863 \$26,704	\$41,059 \$28,701	\$42,291 \$30,794	\$43,559 \$32,987	\$44,866 \$35,283	\$46,212 \$37,688	\$47,599 \$40,205	\$49,027 \$42,839	\$50,497 \$45,595	\$52,012 \$48.477	\$53,573 \$51,492	\$55,180 \$54,644	\$56,835 \$57,939	\$58,540 \$61,382	\$60,297 \$64,980	\$62,105 \$68,738	\$63,969 \$72,663	\$65,888 \$76,762	\$67,864 \$81,042	\$69,900 \$85,509	\$71,997 \$90,171	\$74,157 \$95,036	\$76,382 \$100,112	\$78,673 \$105,407	\$81,033 \$110,929	\$83,464 \$116,688	\$85,968 \$122,693
	\$1,026,475	\$1,099,560	\$1,176,106	. ,				. ,	\$1,716,406	\$1,821,471			\$2,165,991	\$2,291,267				. ,		\$3,167,876	\$3,337,069	\$3,513,563	\$3,697,643	\$3,889,606		
\$79,726	\$82,118	\$84,582	\$87,119	\$89,733	\$92,425	\$95,197	\$98,053	\$100,995	\$104.025	\$107,145	\$110,360	\$113,670	\$117,081	\$120,593	\$124,211	\$127,937	\$131,775	\$135,728	\$139,800	\$143,994	\$148,314	\$152,764	\$157.347	\$162,067	\$166.929	\$171,937
\$53,409	\$57,403	\$61,588	\$65,974	\$70,566	\$75,375	\$80,409	\$85,678	\$91,189	\$96,955	\$102,984	\$109,288	\$115,878	\$122,764	\$129,959	\$137,476	\$145,327	\$153,525	\$162,084	\$171,018	\$180,342	\$190,073	\$200,224	\$210,814	\$221,859	\$233,376	. ,
\$1,913,429	\$2,052,949	\$2,199,119	\$2,352,212	\$2,512,511	\$2,680,311	\$2,855,917	\$3,039,648	\$3,231,832	\$3,432,812	\$3,642,941	\$3,862,589	\$4,092,137	\$4,331,982	\$4,582,534	\$4,844,221	\$5,117,485	\$5,402,785	\$5,700,597	\$6,011,415	\$6,335,752	\$6,674,139	\$7,027,127	\$7,395,287	\$7,779,212	\$8,179,518	\$8,596,840
\$159,452	\$164,236	\$169,163	\$174,238	\$179,465	\$184,849	\$190,394	\$196,106	\$201,989	\$208,049	\$214,291	\$220,719	\$227,341	\$234,161	\$241,186	\$248,422	\$255,874	\$263,550	\$271,457	\$279,601	\$287,989	\$296,628	\$305,527	\$314,693	\$324,134	\$333,858	\$343,874
\$106,818	\$114,806	\$123,177	\$131,947	\$141,133	\$150,751	\$160,819	\$171,355	\$182,379	\$193,910	\$205,969	\$218,576	\$231,755	\$245,528	\$259,919	\$274,952	\$290,653	\$307,049	\$324,167	\$342,036	\$360,685	\$380,145	\$400,448	\$421,628	\$443,717	\$466,753	1 ,
\$3,826,857	\$4,105,898	\$4,398,238	\$4,704,423	\$5,025,021	\$5,360,621	\$5,711,834	\$6,079,295	\$6,463,664	\$6,865,623	\$7,285,882	\$7,725,178	\$8,184,274	\$8,663,963	\$9,165,068	\$9,688,442	\$10,234,970	\$10,805,569	\$11,401,193	\$12,022,830	\$12,671,503	\$13,348,277	\$14,054,252	\$14,790,573	\$15,558,424	\$16,359,034	\$17,193,679
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1,948	\$2,006	\$2,066	\$2,128
\$22,696	\$23,377	\$24,078	\$24,801	\$25,545	\$26,311	\$27,101	\$27,914	\$28,751	\$29,613	\$30,502	\$31,417	\$32,359	\$33,330	\$34,330	\$35,360	\$36,421	\$37,513	\$38,639	\$39,798	\$40,992	\$42,222	\$43,488	\$44,793	\$46,137	\$47,521	\$48,947
\$1,974 \$0	\$2,033 \$0	\$2,094 \$0	\$2,157	\$2,221 \$0	\$2,288 \$0	\$2,357	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,652 \$0	\$2,732 \$0	\$2,814 \$0	\$2,898	\$2,985 \$0	\$3,075 \$0	\$3,167	\$3,262 \$0	\$3,360 \$0	\$3,461 \$0	\$3,565 \$0	\$3,671 \$0	\$3,782 \$0	\$3,895 \$0	\$4,012 \$0	\$4,132 \$0	\$4,256 \$0
ېں \$34,775	\$35,818	\$0 \$36,892	\$0 \$37,999	\$39,139	\$40,313	\$0 \$41,523	\$42,768	\$0 \$44,051	\$45,373	\$46,734	\$48,136		\$0 \$51,068	\$0 \$52,600	\$0 \$54,178	\$0 \$55,803	ېں \$57,477	\$59,201	\$60,977	\$62,807	ېں \$64,691	\$66,632	\$68.631	\$0 \$70,690	\$0 \$72,810	
	+,	+,		,	1.0,000	<i>•••••••••••••••••••••••••••••••••••••</i>	<i>Ţ,</i>	<i></i>	<i>•••••••</i>		<i>,,</i>	<i></i> ,		<i></i> ,	<i>te ij=</i> e			+,					,		<i></i>	<i></i>
\$30,858	\$31,783	\$32,737	\$33,719	\$34,731	\$35,773	\$36,846	\$37,951	\$39,090	\$40,262	\$41,470	\$42,714	\$43,996	\$45,316	\$46,675	\$48,075	\$49,518	\$51,003	\$52,533	\$54,109	\$55,732	\$57,404	\$59,127	\$60,900	\$62,727	\$64,609	\$66,547
\$36,702	\$31,783	\$38,937	\$40,105	\$41,308	\$42,548	\$43,824	\$45,139	\$46,493	\$40,282	\$49,324	\$50,804	\$52,328	\$53,898	\$55,515	\$48,073	\$58,896	\$60,663	\$62,483	\$64,357	\$66,288	\$68,277	\$70,325	\$72,435	\$74,608	\$76,846	\$79,151
\$30,702	\$43,823	\$38,537 \$45,137	\$46,491	\$47,886	\$49,323	\$50,802	\$52,327	\$40,493 \$53,896	\$55,513	\$4 <i>3</i> ,324 \$57,179	\$58,894	\$60,661	\$62,481	\$64,355	\$66,286	\$68,274	\$70,323	\$72,433	\$74,605	\$76,843	\$79,149	\$70,323	\$83,969	\$86,488	\$89,082	\$91,755
\$48,391	\$49,842	\$51,338	\$52,878	\$54,464	\$56,098	\$57,781	\$59,514	\$61,300	\$63,139	\$65,033	\$66,984	\$68,993	\$71,063	\$73,195	\$75,391	\$77,653	\$79,982	\$82,382	\$84,853	\$87,399	\$90,021	\$92,721	\$95,503	\$98,368	. ,	
																								-		
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 5, Design Flow Rate = 20 CFS

ASSUMPTIONS:				
Estimated Capit	al Cost:		\$4,403,000 Total Capita	al Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REP	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replace	ment Cost:		
To Replace	25%	After Life of Project		\$4,825,585
To Replace	50%	After Life of Project		\$9,651,169
To Replace	100%	After Life of Project	\$4,403,000	\$19,302,338
Disposal and Re	moval Cost			
To Replace	25%	After Life of Project		\$120,557
To Replace	50%	After Life of Project		\$241,115
To Replace	100%	After Life of Project	\$110,000	\$482,230
Total Replacem	ent Cost:			
To Replace	25%	After Life of Project		\$4,946,142
To Replace	50%	After Life of Project		\$9,892,284
			** ***	

\$4,513,000

REPLACEMENT I	FUND SUM	IMARY		
Annual Deposit I	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$43,850	
To Replace	50%	After Life of Project	\$87,700	
To Replace	100%	After Life of Project	\$175,400	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$23,242	
To Replace	50%	After Life of Project	\$46,484	
To Replace	100%	After Life of Project	\$92,968	
Deposit Require	d at Year 2	5 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$47,246	
To Replace	50%	After Life of Project	\$94,492	
To Replace	100%	After Life of Project	\$188,984	
Deposit Require	d at Year 5	0 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$98,923	
To Replace	50%	After Life of Project	\$197,846	
To Replace	100%	After Life of Project	\$395,691	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,128,250	\$966,000	\$742,643	\$2,836,893
50% Replacement	\$2,256,500	\$966,000	\$742,643	\$3,965,143
100% Replacement	\$4,513,000	\$966,000	\$742,643	\$6,221,643
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,128,250	\$966,000	\$883,261	\$2,977,511
50% Replacement	\$2,256,500	\$966,000	\$883,261	\$4,105,761
100% Replacement	\$4,513,000	\$966,000	\$883,261	\$6,362,261
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,128,250	\$966,000	\$1,023,879	\$3,118,129
50% Replacement	\$2,256,500	\$966,000	\$1,023,879	\$4,246,379
100% Replacement	\$4,513,000	\$966,000	\$1,023,879	\$6,502,879
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$1,128,250	\$966,000	\$1,164,497	\$3,258,747
50% Replacement	\$2,256,500	\$966,000	\$1,164,497	\$4,386,997
100% Replacement	\$4,513,000	\$966,000	\$1,164,497	\$6,643,497

LIFE CYCLE COSTS:

1	LIFE CICLE COSIS:					1		1			1						1					_
	Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	
																						-

To Replace 100% After Life of Project

Year Capital Expenses: Replacement Fund (For Funding Replacement Deposits Interest End of Year Balance	0 \$4,403,000 nt of 25% of Sy	1	2	3	4	5	6	7																
Replacement Fund (For Funding Replacement Deposits Interest								-	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Deposits Interest	nt of 25% of Sy																							
Interest		stem):																						
		\$23,242	\$23,939	\$24,657	\$25,397	\$26,159	\$26,944	\$27,752	\$28,585	\$29,442	\$30,325	\$31,235	\$32,172	\$33,137	\$34,132	\$35,156	\$36,210	\$37,297	\$38,415	\$39,568	\$40,755	\$41,978	\$43,237	\$44,534
End of Year Balance		\$0	\$697	\$1,436	\$2,219	\$3,048	\$3,924	\$4,850	\$5,828	\$6,860	\$7,949	\$9,098	\$10,308	\$11,582	\$12,924	\$14,335	\$15,820	\$17,381	\$19,021	\$20,744	\$22,554	\$24,453	\$26,446	\$28,536
		\$23,242	\$47,878	\$73,972	\$101,588	\$130,795	\$161,663	\$194,265	\$228,677	\$264,980	\$303,255	\$343,588	\$386,068	\$430,787	\$477,842	\$527,333	\$579,363	\$634,041	\$691,477	\$751,789	\$815,098	\$881,528	\$951,211	\$1,024,282
Replacement Fund (For Funding Replacement	nt of 50% of Sy	stem):																						
Deposits		\$46,484	\$47,878	\$49,315	\$50,794	\$52,318	\$53,888	\$55,504	\$57,169	\$58,884	\$60,651	\$62,470	\$64,345	\$66,275	\$68,263	\$70,311	\$72,420	\$74,593	\$76,831	\$79,136	\$81,510	\$83,955	\$86,474	\$89,068
Interest		\$0	\$1,395	\$2,873	\$4,438	\$6,095	\$7,848	\$9,700	\$11,656	\$13,721	\$15,899	\$18,195	\$20,615	\$23,164	\$25,847	\$28,671	\$31,640	\$34,762	\$38,042	\$41,489	\$45,107	\$48,906	\$52,892	\$57,073
End of Year Balance		\$46,484	\$95,757	\$147,944	\$203,177	\$261,590	\$323,325	\$388,529	\$457,355	\$529,960	\$606,509	\$687,175	\$772,135	\$861,574	\$955,684	\$1,054,666	\$1,158,726	\$1,268,081	\$1,382,954	\$1,503,579	\$1,630,196	\$1,763,057	\$1,902,422	\$2,048,563
Replacement Fund (For Funding Replacement	nt of 100% of S	ystem):																						
Deposits		\$92,968	\$95,757	\$98,630	\$101,588	\$104,636	\$107,775	\$111,008	\$114,339	\$117,769	\$121,302	\$124,941	\$128,689	\$132,550	\$136,526	\$140,622	\$144,841	\$149,186	\$153,662	\$158,271	\$163,020	\$167,910	\$172,947	\$178,136
Interest		\$0	\$2,789	\$5,745	\$8,877	\$12,191	\$15,695	\$19,400	\$23,312	\$27,441	\$31,798	\$36,391	\$41,231	\$46,328	\$51,694	\$57,341	\$63,280	\$69,524	\$76,085	\$82,977	\$90,215	\$97,812	\$105,783	\$114,145
End of Year Balance		\$92,968	\$191,514	\$295,889	\$406,354	\$523,180	\$646,651	\$777,059	\$914,709	\$1,059,919	\$1,213,019	\$1,374,350	\$1,544,270	\$1,723,148	\$1,911,369	\$2,109,332	\$2,317,453	\$2,536,162	\$2,765,909	\$3,007,158	\$3,260,392	\$3,526,114	\$3,804,845	\$4,097,126
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$13,200	\$13,596	\$14,004	\$14,424	\$14,857	\$15,302	\$15,761	\$16,234	\$16,721	\$17,223	\$17,740	\$18,272	\$18,820	\$19,385	\$19,966	\$20,565	\$21,182	\$21,818	\$22,472	\$23,146	\$23,841	\$24,556	\$25,293
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$19,320	\$19,900	\$20,497	\$21,111	\$21,745	\$22,397	\$23,069	\$23,761	\$24,474	\$25,208	\$25,964	\$26,743	\$27,546	\$28,372	\$29,223	\$30,100	\$31,003	\$31,933	\$32,891	\$33,878	\$34,894	\$35,941	\$37,019
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$14,853	\$15,298	\$15,757	\$16,230	\$16,717	\$17,219	\$17,735	\$18,267	\$18,815	\$19,380	\$19,961	\$20,560	\$21,177	\$21,812	\$22,466	\$23,140	\$23,834	\$24,550	\$25,286	\$26,045	\$26,826	\$27,631	\$28,460
4-Week Annual Pumping Duration ⁸		\$17,665	\$18,195	\$18,741	\$19,303	\$19,882	\$20,479	\$21,093	\$21,726	\$22,378	\$23,049	\$23,741	\$24,453	\$25,186	\$25,942	\$26,720	\$27,522	\$28,347	\$29,198	\$30,074	\$30,976	\$31,905	\$32,863	\$33,848
6-Week Annual Pumping Duration ⁸		\$20,478	\$21,092	\$21,725	\$22,376	\$23,048	\$23,739	\$24,451	\$25,185	\$25,940	\$26,719	\$27,520	\$28,346	\$29,196	\$30,072	\$30,974	\$31,903	\$32,861	\$33,846	\$34,862	\$35,908	\$36,985	\$38,094	\$39,237
8-Week Annual Pumping Duration ⁸		\$23,290	\$23,989	\$24,708	\$25,450	\$26,213	\$26,999	\$27,809	\$28,644	\$29,503	\$30,388	\$31,300	\$32,239	\$33,206	\$34,202	\$35,228	\$36,285	\$37,374	\$38,495	\$39,650	\$40,839	\$42,064	\$43,326	\$44,626
ГГ									<u> </u>											<u> </u>				
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$19,784,568

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

\$45,870 \$30,728	\$47,246 \$33,026	\$48,663 \$35,435	\$50,123 \$37,958	\$51,627 \$40,600	\$53,176 \$43,367	\$54,771 \$46,263	\$56,414 \$49,294	\$58,107 \$52,465	\$59,850 \$55,782	\$61,645 \$59,251	\$63,495 \$62,878	\$65,400 \$66,670	\$67,362 \$70,632	\$69,382 \$74,771	\$71,464 \$79,096	\$73,608 \$83,613	\$75,816 \$88,329	\$78,091 \$93,254	\$80,433 \$98,394	\$82,846 \$103,759	\$85,332 \$109,357	\$87,892 \$115,198	3
\$1,100,880	\$1,181,152	\$1,265,251	\$1,353,331	\$1,445,558	\$1,542,101	\$1,643,135	\$1,748,844	\$1,859,416	\$1,975,048	\$2,095,945	\$2,222,318	\$2,354,387	\$2,492,381	\$2,636,535	\$2,787,095	\$2,944,315	\$3,108,461	\$3,279,805	\$3,458,633	\$3,645,238	\$3,839,927	\$4,043,016	Ş
\$91,740	\$94,492	\$97,327	\$100,247	\$103,254	\$106,352	\$109,542	\$112,829	\$116,213	\$119,700	\$123,291	\$126,990	\$130,799	\$134,723	\$138,765	\$142,928	\$147,216	\$151,632	\$156,181	\$160,867	\$165,693	\$170,663	\$175,783	5
\$61,457	\$66,053	\$70,869	\$75,915	\$81,200	\$86,734	\$92,526	\$98,588	\$104,931	\$111,565	\$118,503	\$125,757	\$133,339	\$141,263	\$149,543	\$158,192	\$167,226	\$176,659	\$186,508	\$196,788	\$207,518	\$218,714	\$230,396	j i
\$2,201,760	\$2,362,305	\$2,530,501	\$2,706,663	\$2,891,117	\$3,084,202	\$3,286,271	\$3,497,687	\$3,718,832	\$3,950,096	\$4,191,890	\$4,444,636	\$4,708,775	\$4,984,761	\$5,273,069	\$5,574,189	\$5,888,631	\$6,216,922	\$6,559,611	\$6,917,266	\$7,290,476	\$7,679,854	\$8,086,033	3\$
\$183,480	\$188,984	\$194,654	\$200,494	\$206,508	\$212,704	\$219,085	\$225,657	\$232,427	\$239,400	\$246,582	\$253,979	\$261,599	\$269,447	\$277,530	\$285,856	\$294,432	\$303,264	\$312,362	\$321,733	\$331,385	\$341,327	\$351,567	,
\$122,914	\$132,106	\$141,738	\$151,830	\$162,400	\$173,467	\$185,052	\$197,176	\$209,861	\$223,130	\$237,006	\$251,513	\$266,678	\$282,526	\$299,086	\$316,384	\$334,451	\$353,318	\$373,015	\$393,577	\$415,036	\$437,429	\$460,791	
\$4,403,520	\$4,724,610	\$5,061,002	\$5,413,326	\$5,782,234	\$6,168,405	\$6,572,541	\$6,995,375	\$7,437,663	\$7,900,193	\$8,383,780	\$8,889,273	\$9,417,550	\$9,969,523	\$10,546,138	\$11,148,378	\$11,777,261	\$12,433,844	\$13,119,221	\$13,834,531	\$14,580,953	\$15,359,708	\$16,172,066	, \$1
ĆC 512	\$6,708	\$6,909	67 117	67.220	67.550	\$7,777	ć9 010	\$8,250	\$8,498	60 750	ć0.01F	\$9,286	60 F.C.4	\$9,851	\$10,147	¢10.4F1	\$10,765	ć11 099	¢11 420	\$11,763	\$12,116	\$12,479	,
\$6,513 \$2,605	\$0,708	\$6,909	\$7,117 \$2,847	\$7,330 \$2,932	\$7,550 \$3,020	\$3,111	\$8,010 \$3,204	\$8,250	\$8,498 \$3,399	\$8,753 \$3,501	\$9,015 \$3,606	\$9,280	\$9,564 \$3,826	\$9,851 \$3,940	\$10,147 \$4,059	\$10,451 \$4,180	\$10,765	\$11,088 \$4,435	\$11,420 \$4,568	\$11,765	\$12,116	\$12,479 \$4,992	
\$2,003 \$987	\$2,083	\$2,704 \$1,047	\$2,847	\$2,932	\$3,020	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$3,000	\$3,714 \$1,407	\$3,820 \$1,449	\$1,493	\$4,033	\$1,584	\$4,500	\$1,680	\$1,730	\$4,703	\$1,836	\$4,992 \$1,891	
\$26,051	\$26,833	\$27,638	\$28,467	\$29,321	\$30,201	\$31,107	\$32,040	\$33,001	\$33,991	\$35,011	\$36,061	\$37,143	\$38,257	\$39,405	\$40,587	\$41,805	\$43,059	\$44,351	\$45,681	\$47,052	\$48,463	\$49,917	
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
\$38,130	\$39,274	\$40,452	\$41,665	\$42,915	\$44,203	\$45,529	\$46,895	\$48,302	\$49,751	\$51,243	\$52,780	\$54,364	\$55,995	\$57,675	\$59,405	\$61,187	\$63,023	\$64,913	\$66,861	\$68,866	\$70,932	\$73,060	i -
\$29,313	\$30,193	\$31,099	\$32,032	\$32,992	\$33,982	\$35,002	\$36,052	\$37,133	\$38,247	\$39,395	\$40,577	\$41,794	\$43,048	\$44,339	\$45,669	\$47,039	\$48,451	\$49,904	\$51,401	\$52,943	\$54,532	\$56,168	3
\$34,864	\$35,910	\$36,987	\$38,097	\$39,240	\$40,417	\$41,629	\$42,878	\$44,164	\$45,489	\$46,854	\$48,260	\$49,708	\$51,199	\$52,735	\$54,317	\$55,946	\$57,625	\$59,353	\$61,134	\$62,968	\$64,857	\$66,803	
\$40,414	\$41,627	\$42,876	\$44,162	\$45,487	\$46,851	\$48,257	\$49,704	\$51,196	\$52,731	\$54,313	\$55,943	\$57.621	\$59,350	\$61,130	\$62,964	\$64,853	\$66,799	\$68,803	\$70,867	\$72,993	\$75,182	\$77,438	
\$45,965	\$47,344	\$48,764	\$50,227	\$51,734	\$53,286	\$54,884	\$56,531	\$58,227	\$59,974	\$61,773	\$63,626	\$65,535	\$67,501	\$69,526	\$71,612	\$73,760	\$75,973	\$78,252	\$80,599	\$83,017	\$85,508	\$88,073	
24	25	26	27	28	20	20	31	22	33	34	35	26	27	20	20	40	41	42	43	44	45	46	
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

47	48	49	50
\$90,528	\$93,244	\$96,042	\$98,923
\$121,290	\$127,645	\$134,272	\$141,181
\$4,254,835	\$4,475,725	\$4,706,038	\$4,946,142
\$181,057	\$186,489	\$192,083	\$197,846
\$242,581	\$255,290	\$268,543	\$282,362
\$8,509,671	\$8,951,449	\$9,412,076	\$9,892,284
\$362,114	\$372,977	\$384,166	\$395,691
\$485,162	\$510,580	\$537,087	\$564,725
\$17,019,341	\$17,902,899	\$18,824,152	\$19,784,568
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$51,415	\$52,957	\$54,546	\$56,182
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$75,252	\$77,510	\$79,835	\$82,230
\$57,853	\$59,588	\$61,376	\$63,217
\$68,807	\$70,871	\$72,997	\$75,187
\$79,761	\$82,154	\$84,619	\$87,157
\$90,715	\$93,437	\$96,240	\$99,127
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 1, Design Flow Rate = 40 CFS

ASSUMPTIONS:					
Estimated Capita	al Cost:		\$3,899,000	Total Capita	l Cost
Interest on Repl	acement F	und:	3.00%		
Rate of Inflation	:		3.00%		
Project Design L	ife:		50	Years	
SUMMARY REPI	LACEMENT	COSTS:	CURREN	IT COST ²	FUTURE COST ³
Estimated Proje	ct Replacer	ment Cost:			
To Replace	25%	After Life of Project			\$4,273,212
To Replace	50%	After Life of Project			\$8,546,425
To Replace	100%	After Life of Project	\$3.899.000		\$17.092.850

\$98,000

\$3,997,000

REPLACEMENT I	FUND SUM	IMARY		
Annual Deposit I	Required (Assume Equal Deposit Made	e Each Year):	
To Replace	25%	After Life of Project	\$38,836	
To Replace	50%	After Life of Project	\$77,673	
To Replace	100%	After Life of Project	\$155,345	
Deposit Require	d at Year 1	(Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$20,585	
To Replace	50%	After Life of Project	\$41,169	
To Replace	100%	After Life of Project	\$82,338	
Deposit Require	d at Year 2	5 (Assume Deposits Increa	se at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$41,844	
To Replace	50%	After Life of Project	\$83,688	
To Replace	100%	After Life of Project	\$167,377	
Deposit Require	d at Year 5	0 (Assume Deposits Increas	se at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$87,612	
To Replace	50%	After Life of Project	\$175,225	
To Replace	100%	After Life of Project	\$350,449	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$999,250	\$891,000	\$1,760,966	\$3,651,216
50% Replacement	\$1,998,500	\$891,000	\$1,760,966	\$4,650,466
100% Replacement	\$3,997,000	\$891,000	\$1,760,966	\$6,648,966
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$999,250	\$891,000	\$2,095,348	\$3,985,598
50% Replacement	\$1,998,500	\$891,000	\$2,095,348	\$4,984,848
100% Replacement	\$3,997,000	\$891,000	\$2,095,348	\$6,983,348
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$999,250	\$891,000	\$2,429,729	\$4,319,979
50% Replacement	\$1,998,500	\$891,000	\$2,429,729	\$5,319,229
100% Replacement	\$3,997,000	\$891,000	\$2,429,729	\$7,317,729
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$999,250	\$891,000	\$2,764,110	\$4,654,360
50% Replacement	\$1,998,500	\$891,000	\$2,764,110	\$5,653,610
100% Replacement	\$3,997,000	\$891,000	\$2,764,110	\$7,652,110

LIFE CYCLE COSTS:

r	LIFE CTCLE COSTS:																					_
	Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	
																						-

Disposal and Removal Cost:

Total Replacement Cost:

 To Replace
 25%
 After Life of Project

 To Replace
 50%
 After Life of Project

To Replace 100% After Life of Project

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

LIFE CYCLE COSTS:																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$3,899,000																							
Replacement Fund (For Funding Replaceme	nt of 25% of Sys	tem):																						
Deposits		\$20,585	\$21,202	\$21,838	\$22,493	\$23,168	\$23,863	\$24,579	\$25,316	\$26,076	\$26,858	\$27,664	\$28,494	\$29,349	\$30,229	\$31,136	\$32,070	\$33,032	\$34,023	\$35,044	\$36,095	\$37,178	\$38,293	\$39,442
Interest		\$0	\$618	\$1,272	\$1,965	\$2,699	\$3,475	\$4,295	\$5,162	\$6,076	\$7,040	\$8,057	\$9,129	\$10,258	\$11,446	\$12,696	\$14,011	\$15,394	\$16,846	\$18,372	\$19,975	\$21,657	\$23,422	\$25,274
End of Year Balance		\$20,585	\$42,404	\$65,514	\$89,973	\$115,840	\$143,179	\$172,053	\$202,531	\$234,683	\$268,582	\$304,303	\$341,926	\$381,532	\$423,207	\$467,040	\$513,121	\$561,547	\$612,416	\$665,833	\$721,903	\$780,738	\$842,453	\$907,169
Replacement Fund (For Funding Replaceme	nt of 50% of Sys	tem):																						
Deposits		\$41,169	\$42,404	\$43,676	\$44,987	\$46,336	\$47,726	\$49,158	\$50,633	\$52,152	\$53,716	\$55,328	\$56,988	\$58,697	\$60,458	\$62,272	\$64,140	\$66,064	\$68,046	\$70,088	\$72,190	\$74,356	\$76,587	\$78,884
Interest		\$0	\$1,235	\$2,544	\$3,931	\$5,398	\$6,950	\$8,591	\$10,323	\$12,152	\$14,081	\$16,115	\$18,258	\$20,516	\$22,892	\$25,392	\$28,022	\$30,787	\$33,693	\$36,745	\$39,950	\$43,314	\$46,844	\$50,547
End of Year Balance		\$41,169	\$84,808	\$131,029	\$179,946	\$231,681	\$286,358	\$344,106	\$405,062	\$469,366	\$537,163	\$608,606	\$683,852	\$763,065	\$846,415	\$934,079	\$1,026,242	\$1,123,093	\$1,224,832	\$1,331,665	\$1,443,805	\$1,561,475	\$1,684,906	\$1,814,338
Replacement Fund (For Funding Replaceme	nt of 100% of S	/stem):																						
Deposits		\$82,338	\$84,808	\$87,353	\$89,973	\$92,672	\$95,453	\$98,316	\$101,266	\$104,304	\$107,433	\$110,656	\$113,975	\$117,395	\$120,916	\$124,544	\$128,280	\$132,129	\$136,092	\$140,175	\$144,381	\$148,712	\$153,173	\$157,769
Interest		\$0	\$2,470	\$5,089	\$7,862	\$10,797	\$13,901	\$17,181	\$20,646	\$24,304	\$28,162	\$32,230	\$36,516	\$41,031	\$45,784	\$50,785	\$56,045	\$61,575	\$67,386	\$73,490	\$79,900	\$86,628	\$93,689	\$101,094
End of Year Balance		\$82,338	\$169,617	\$262,058	\$359,893	\$463,362	\$572,715	\$688,213	\$810,125	\$938,732	\$1,074,327	\$1,217,212	\$1,367,704	\$1,526,130	\$1,692,830	\$1,868,159	\$2,052,484	\$2,246,187	\$2,449,665	\$2,663,330	\$2,887,611	\$3,122,951	\$3,369,813	\$3,628,676
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500		\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$11,700	\$12,051	\$12,413	\$12,785	\$13,168	\$13,564	\$13,970	\$14,390	\$14.821	\$15,266	\$15.724	\$16.196	\$16,681	\$17,182	\$17,697	\$18,228	\$18,775	\$19,338	\$19,918	\$20,516	\$21,132	\$21.765	\$22,418
Administration, Insurance, Accounting		\$1,000		\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$17,820	\$18,355	\$18,905	\$19,472	\$20,057	\$20,658	\$21,278	\$21,916	\$22,574	\$23,251	\$23,949	\$24,667	\$25,407	\$26,169	\$26,954	\$27,763	\$28,596	\$29,454	\$30,337	\$31,247	\$32,185	\$33,150	\$34,145
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$35,219	\$36,276	\$37,364	\$38,485	\$39,640	\$40,829	\$42,054	\$43,315	\$44,615	\$45,953	\$47,332	\$48,752	\$50,214	\$51,721	\$53,272	\$54,871	\$56,517	\$58,212	\$59,959	\$61,757	\$63,610	\$65,518	\$67,484
4-Week Annual Pumping Duration ⁸		\$41,907	\$43,164	\$44,459	\$45,793	\$47,167	\$48,582	\$50,039	\$51,540	\$53,086	\$54,679	\$56,319	\$58,009	\$59,749	\$61,542	\$63,388	\$65,290	\$67,248	\$69,266	\$71,344	\$73,484	\$75,689	\$77,959	\$80,298
6-Week Annual Pumping Duration ⁸		\$48,595	\$50,052	\$51,554	\$53,101	\$54,694	\$56,334	\$58,024	\$59,765	\$61,558	\$63,405	\$65,307	\$67,266	\$69,284	\$71,363	\$73,504	\$75,709	\$77,980	\$80,319	\$82,729	\$85,211	\$87.767	\$90,400	\$93,112
8-Week Annual Pumping Duration ⁸		\$55,282	\$56,941	\$58,649	\$60,408	\$62,221	\$64,087	\$66,010	\$67,990	\$70,030	\$72,131	\$74,295	\$76,524	\$78,819	\$81,184	\$83,619	\$86,128	\$88,712	\$91,373	\$94,114	\$96,938	\$99,846	\$102,841	\$105,926
[]																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$107,406 \$214,811

\$429,623

\$4,380,618

\$8,761,236

\$17,522,472

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

\$27,215 \$975,009	\$29,250	\$31,383	\$33,618	\$35,958	\$38,408	\$40,973	\$43,658	CAC 4C7													60C 0F 4		
\$975,009				+/	<i>\$50,100</i>	J40,575	\$45,056	\$46,467	\$49,405	\$52,477	\$55,689	\$59,047	\$62,556	\$66,222	\$70,052	\$74,053	\$78,230	\$82,592	\$87,144	\$91,896	\$96,854	\$102,027	
	\$1,046,104	\$1,120,586	\$1,198,596	\$1,280,279	\$1,365,783	\$1,455,265	\$1,548,887	\$1,646,817	\$1,749,228	\$1,856,302	\$1,968,226	\$2,085,195	\$2,207,411	\$2,335,083	\$2,468,428	\$2,607,673	\$2,753,051	\$2,904,804	\$3,063,185	\$3,228,455	\$3,400,884	\$3,580,753	i (
\$81,251	\$83,688	\$86,199	\$88,785	\$91,448	\$94,192	\$97,018	\$99,928	\$102,926	\$106,014	\$109,194	\$112,470	\$115,844	\$119,320	\$122,899	\$126,586	\$130,384	\$134,295	\$138,324	\$142,474	\$146,748	\$151,150	\$155,685	,
\$54,430	\$58,501	\$62,766	\$67,235	\$71,916	\$76,817	\$81,947	\$87,316	\$92,933	\$98,809	\$104,954	\$111,378	\$118,094	\$125,112	\$132,445	\$140,105	\$148,106	\$156,460	\$165,183	\$174,288	\$183,791	\$193,707	\$204,053	j –
\$1,950,019	\$2,092,208	\$2,241,173	\$2,397,193	\$2,560,557	\$2,731,566	\$2,910,530	\$3,097,774	\$3,293,634	\$3,498,457	\$3,712,605	\$3,936,453	\$4,170,391	\$4,414,822	\$4,670,166	\$4,936,857	\$5,215,346	\$5,506,101	\$5,809,609	\$6,126,370	\$6,456,910	\$6,801,767	\$7,161,505	; \$
\$162,502	\$167,377	\$172,398	\$177,570	\$182,897	\$188,384	\$194,035	\$199,856	\$205,852	\$212,028	\$218,389	\$224,940	\$231,688	\$238,639	\$245,798	\$253,172	\$260,767	\$268,590	\$276,648	\$284,947	\$293,496	\$302,301	\$311,370	
\$108,860	\$117,001	\$125,532	\$134,470	\$143,832	\$153,633	\$163,894	\$174,632	\$185,866	\$197,618	\$209,907	\$222,756	\$236,187	\$250,223	\$264,889	\$280,210	\$296,211	\$312,921	\$330,366	\$348,577	\$367,582	\$387,415	\$408,106	
\$3,900,037	\$4,184,415	\$4,482,345	\$4,794,386	\$5,121,114	\$5,463,131	\$5,821,061	\$6,195,549	\$6,587,268	\$6,996,913	\$7,425,209	\$7,872,906	\$8,340,781	\$8,829,644	\$9,340,331	\$9,873,713	\$10,430,692	\$11,012,203	\$11,619,217	\$12,252,741	\$12,913,819	\$13,603,534	\$14,323,010	Ş1
ĆC 540	ćc 700	ćc 000	67.447	67 220	67.550	ć	ć0.010	60.250	ć0 400	ćo 750	Ć0.015	ć0 200	60 5 6 4	ć0.051	610 117	610 AF1	610 705	ć11.000	ć11 420	611 700	¢12.11C	642.470	
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	
\$23,091	\$23,784	\$24,497	\$25,232	\$25,989	\$26,769	\$27,572	\$28,399	\$29,251	\$30,128	\$31,032	\$31,963	\$32,922	\$33,910	\$34,927	\$35,975	\$37,054	\$38,166	\$39,311	\$40,490	\$41,705	\$42,956	\$44,245	,
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	:
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	j –
\$35,169	\$36,224	\$37,311	\$38,430	\$39,583	\$40,771	\$41,994	\$43,254	\$44,551	\$45,888	\$47,265	\$48,683	\$50,143	\$51,647	\$53,197	\$54,793	\$56,436	\$58,130	\$59,873	\$61,670	\$63,520	\$65,425	\$67,388	1
\$69,508	\$71,594	\$73,741	\$75,954	\$78,232	\$80,579	\$82,997	\$85,487	\$88,051	\$90,693	\$93,413	\$96,216	\$99,102	\$102,075	\$105,138	\$108,292	\$111,541	\$114.887	\$118,333	\$121,883	\$125,540	\$129.306	\$133,185	
										. ,					. ,					. ,			
\$82,707	\$85,188	\$87,744	\$90,376	\$93,087	\$95,880	\$98,756	\$101,719	\$104,771	\$107,914	\$111,151	\$114,486	\$117,920	\$121,458	\$125,102	\$128,855	\$132,720	\$136,702	\$140,803	\$145,027	\$149,378	\$153,859	\$158,475	
\$95,906	\$98,783	\$101,746	\$104,799	\$107,943	\$111,181	\$114,516	\$117,952	\$121,490	\$125,135	\$128,889	\$132,756	\$136,738	\$140,841	\$145,066	\$149,418	\$153,900	\$158,517	\$163,273	\$168,171	\$173,216	\$178,413	\$183,765	
\$109,104	\$112,377	\$115,749	\$119,221	\$122,798	\$126,482	\$130,276	\$134,184	\$138,210	\$142,356	\$146,627	\$151,026	\$155,557	\$160,223	\$165,030	\$169,981	\$175,080	\$180,333	\$185,743	\$191,315	\$197,054	\$202,966	\$209,055)
															<u> </u>								Г
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

\$40,625 \$41,844 \$43,099 \$44,392 \$45,724 \$47,096 \$48,509 \$49,964 \$51,463 \$53,007 \$54,597 \$56,235 \$57,922 \$59,660 \$61,450 \$63,293 \$65,192 \$67,148 \$69,162 \$71,237 \$73,374 \$75,575 \$77,842

47	48	49	50
\$80,178	\$82,583	\$85,061	\$87,612
\$107,423	\$113,051	\$118,920	\$125,039
\$3,768,353	\$3,963,987	\$4,167,967	\$4,380,618
\$160,355	\$165,166	\$170,121	\$175,225
\$214,845	\$226,101	\$237,839	\$250,078
\$7,536,706	\$7,927,973	\$8,335,933	\$8,761,236
\$320,711	\$330,332	\$340,242	\$350,449
\$429,690	\$452,202	\$475,678	\$500,156
\$15,073,411	\$15,855,946	\$16,671,867	\$17,522,472
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$45,572	\$46,939	\$48,347	\$49,798
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$69,410	\$71,492	\$73,637	\$75,846
\$137,181	\$141,296	\$145,535	\$149,901
\$163,229	\$168,126	\$173,170	\$178,365
\$189,278	\$194,956	\$200,805	\$206,829
\$215,327	\$221,786	\$228,440	\$235,293
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 2, Design Flow Rate = 40 CFS

ASSUMPTIONS:				
Estimated Capit	al Cost:		\$4,434,000 Total Capita	al Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REP	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replacer	ment Cost:		
To Replace	25%	After Life of Project		\$4,859,560
To Replace	50%	After Life of Project		\$9,719,120
To Replace	100%	After Life of Project	\$4,434,000	\$19,438,239
Disposal and Re	moval Cost			
To Replace	25%	After Life of Project		\$121,653
To Replace	50%	After Life of Project		\$243,307
To Replace	100%	After Life of Project	\$111,000	\$486,614
Total Replacem	ent Cost:			
To Replace	25%	After Life of Project		\$4,981,213

\$4,545,000

\$4,434,000

REPLACEMENT	FUND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$44,161	
To Replace	50%	After Life of Project	\$88,322	
To Replace	100%	After Life of Project	\$176,644	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$23,407	
To Replace	50%	After Life of Project	\$46,813	
To Replace	100%	After Life of Project	\$93,627	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$47,581	
To Replace	50%	After Life of Project	\$95,162	
To Replace	100%	After Life of Project	\$190,324	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$99,624	
To Replace	50%	After Life of Project	\$199,249	
To Replace	100%	After Life of Project	\$398,497	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,136,250	\$971,000	\$1,778,355	\$3,885,605
50% Replacement	\$2,272,500	\$971,000	\$1,778,355	\$5,021,855
100% Replacement	\$4,545,000	\$971,000	\$1,778,355	\$7,294,355
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,136,250	\$971,000	\$2,116,045	\$4,223,295
50% Replacement	\$2,272,500	\$971,000	\$2,116,045	\$5,359,545
100% Replacement	\$4,545,000	\$971,000	\$2,116,045	\$7,632,045
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,136,250	\$971,000	\$2,453,735	\$4,560,985
50% Replacement	\$2,272,500	\$971,000	\$2,453,735	\$5,697,235
100% Replacement	\$4,545,000	\$971,000	\$2,453,735	\$7,969,735
Assuming the Pumping Power Costs for an 8	-week Annual	Operating Dura	ition:	
25% Replacement	\$1,136,250	\$971,000	\$2,791,425	\$4,898,675
50% Replacement	\$2,272,500	\$971,000	\$2,791,425	\$6,034,925
100% Replacement	\$4,545,000	\$971,000	\$2,791,425	\$8,307,425

LIFE CYCLE COSTS:																					
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	L

Capital Expenses:

To Replace 50% After Life of Project

To Replace 100% After Life of Project

	,																				
Deposits		\$23,407	\$24,109	\$24,832	\$25,577	\$26,345	\$27,135	\$27,949	\$28,787	\$29,651	\$30,540	\$31,457	\$32,400	\$33,372	\$34,374	\$35,405	\$36,467	\$37,561	\$38,688	\$39,848	
Interest		\$0	\$702	\$1,447	\$2,235	\$3,069	\$3,952	\$4,884	\$5,869	\$6,909	\$8,006	\$9,162	\$10,381	\$11,664	\$13,015	\$14,437	\$15,932	\$17,504	\$19,156	\$20,891	
End of Year Balance		\$23,407	\$48,218	\$74,497	\$102,309	\$131,723	\$162,809	\$195,642	\$230,299	\$266,859	\$305,405	\$346,024	\$388,805	\$433,842	\$481,230	\$531,072	\$583,471	\$638,536	\$696,380	\$757,120	Į.
Replacement Fund (For Funding Replacement	t of 50% of Sy	stem):																			
Deposits		\$46,813	\$48,218	\$49,664	\$51,154	\$52,689	\$54,270	\$55,898	\$57,575	\$59,302	\$61,081	\$62,913	\$64,801	\$66,745	\$68,747	\$70,810	\$72,934	\$75,122	\$77,376	\$79,697	
Interest		\$0	\$1,404	\$2,893	\$4,470	\$6,139	\$7,903	\$9,769	\$11,739	\$13,818	\$16,012	\$18,324	\$20,761	\$23,328	\$26,030	\$28,874	\$31,864	\$35,008	\$38,312	\$41,783	
End of Year Balance		\$46,813	\$96,436	\$148,993	\$204,617	\$263,445	\$325,618	\$391,284	\$460,598	\$533,717	\$610,810	\$692,048	\$777,610	\$867,683	\$962,461	\$1,062,144	\$1,166,942	\$1,277,073	\$1,392,760	\$1,514,240	
Replacement Fund (For Funding Replacement	t of 100% of s	(vetom):																			
Deposits	101 100 % 01 3	\$93,627	\$96,436	\$99,329	\$102,309	\$105,378	\$108,539	\$111,796	\$115,149	\$118.604	\$122,162	\$125,827	\$129,602	\$133,490	\$137.494	\$141,619	\$145,868	\$150,244	\$154,751	\$159,394	
Interest		\$0		\$5,786	\$8,940	\$12,277	\$15,807	\$19,537	\$23,477	\$27,636	\$32,023	\$36,649	\$41,523	\$46,657	\$52,061	\$57,748	\$63,729	\$130,244	\$76,624	\$83,566	
End of Year Balance		\$93,627	\$192,872	\$297,987	\$409,235	\$526,890	\$651,236	\$782,569	\$921,195	\$1.067.435	\$1,221,620	\$1.384.095	\$1,555,220	\$1,735,366	\$1,924,922	\$2,124,289	\$2,333,885	\$2,554,145	\$2,785,521	\$3,028,480	
		<i>\$50,01</i>	<i>\</i>	<i><i><i>q</i>_<i>37,307</i></i></i>	<i>v</i> ,200	<i>4020,050</i>	<i>4001)</i> 200	<i><i>ų</i>, 02,000</i>	<i>40</i> 1 ,100	<i>,</i>	<i>\</i>	<i>\</i>	<i>↓</i> 1 ,000, 11 0	<i>_,,,,,,,,,,,,,</i> ,,,,,,,,,,,,,,,,,,,,,,,	<i>v</i> _,,	¥=)== 1)=00	<i>42,000,000</i>	¥2,000 1,12 10	<i>_,,,.</i>	<i>\$0,020,100</i>	
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$13,300	\$13,699	\$14,110	\$14,533	\$14,969	\$15,418	\$15,881	\$16,357	\$16,848	\$17,353	\$17,874	\$18,410	\$18,963	\$19,531	\$20,117	\$20,721	\$21,343	\$21,983	\$22,642	
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$C	1
Total O&M Expenses		\$19,420	\$20,003	\$20,603	\$21,221	\$21,857	\$22,513	\$23,188	\$23,884	\$24,601	\$ 25,33 9	\$26,099	\$26,882	\$27,688	\$28,519	\$29,374	\$30,256	\$31,163	\$32,098	\$33,061	
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$35,567	\$36,634	\$37,733	\$38,865	\$40,031	\$41,232	\$42,469	\$43,743	\$45,055	\$46,407	\$47,799	\$49,233	\$50,710	\$52,231	\$53,798	\$55,412	\$57,075	\$58,787	\$60,551	
4-Week Annual Pumping Duration ⁸		\$42,321	\$43.591	\$44.898	\$46.245	\$47,633	\$49,062	\$50,533	\$52,049	\$53,611	\$55,219	\$56,876	\$58,582	\$60,339	\$62,150	\$64,014	\$65,935	\$67,913	\$69,950	\$72,049	,
6-Week Annual Pumping Duration ⁸		\$49,075	\$50.547	\$52,063	\$53,625	\$55,234	\$56,891	\$58,598	\$60,356	\$62,166	\$64,031	\$65,952	\$67,931	\$69,969	\$72,068	\$74,230	\$76,457	\$78,750	\$81,113	\$83,546	
8-Week Annual Pumping Duration ⁸		\$55,829	\$57,503	\$59,228	\$61.005	\$62,835	\$64.721	\$66,662	\$68,662	\$70,722	\$72,844	\$75,029	\$77,280	\$79,598	\$81,986	\$84.446	\$86.979	\$89,588	\$92,276	\$95.044	
o week Annual Fullping Duration		433,823	, 1 05	<i>433,</i> 220	201,003	202,03 3	90 7 ,721	900,00Z	900,00Z	<i>,,,,,,,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	<i>ې ۲ د</i> ,044	Ş7 3,023	<i>,,,</i> ,200	000,01¢	301,300	704,440	<i>300,313</i>	205,500	<i>332,21</i> 0	49 9 ,044	
																					Γ
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$9,962,426

\$19,924,853

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
\$41,044	\$42,275	\$43,543	\$44,850
\$22,714	\$24,626	\$26,633	\$28,739
\$820,878	\$887,779	\$957,956	\$1,031,544
<i></i>	<i></i>		<i>,,,,,,,,,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,
\$82,088	\$84,550	\$87,087	\$89,700
\$45,427	\$49,253	\$53,267	\$57,477
\$1,641,755	\$1,775,558	\$1,915,912	\$2,063,089
\$164,176	\$169,101	\$174,174	\$179,399
\$90,854	\$98,505	\$106,533	\$114,955
\$3,283,510	\$3,551,116	\$3,831,824	\$4,126,177
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$23,322	\$24,021	\$24,742	\$25,484
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$34,053	\$35,075	\$36,127	\$37,211
\$62,367	\$64,238	\$66,165	\$68,150
\$74,210	\$76,436	\$78,729	\$81,091
\$86,053	\$88,634	\$91,293	\$94,032
\$97,896	\$100,832	\$103,857	\$106,973
20	21	22	23
20	21	~~~	25

\$46,195 \$30,946	\$47,581 \$33,261	\$49,009 \$35,686	\$50,479 \$38,227	\$51,993 \$40,888	\$53,553 \$43,674	\$55,160 \$46,591	\$56,814 \$49,644	\$58,519 \$52,837	\$60,274 \$56,178	\$62,083 \$59,672	\$63,945 \$63,324	\$65,863 \$67,142	\$67,839 \$71,132	\$69,874 \$75,302	\$71,971 \$79,657	\$74,130 \$84,206	\$76,354 \$88,956	\$78,644 \$93,915	\$81,004 \$99,092	\$83,434 \$104,495	\$85,937 \$110,133	\$88,515 \$116,015	
\$1,108,686	\$1,189,528	\$1,274,222	\$1,362,927	\$1,455,808	\$1,553,036	\$1,654,786	\$1,761,244	\$1,872,600	\$1,989,052	\$2,110,807	\$2,238,076	\$2,371,081	\$2,510,053	\$2,655,229	\$2,806,857	\$2,965,192	\$3,130,502	\$3,303,061	\$3,483,157	\$3,671,085	\$3,867,154	\$4,071,684	4\$
\$92,390 \$61,893	\$95,162 \$66,521	\$98,017 \$71,372	\$100,958 \$76,453	\$103,986 \$81,776	\$107,106 \$87,348	\$110,319 \$93,182	\$113,629 \$99,287	\$117,038 \$105,675	\$120,549 \$112,356	\$124,165 \$119,343	\$127,890 \$126,648	\$131,727 \$134,285	\$135,679 \$142,265	\$139,749 \$150,603	\$143,941 \$159,314	\$148,260 \$168,411	\$152,707 \$177,912	\$157,289 \$187,830	\$162,007 \$198,184	\$166,867 \$208,989	\$171,874 \$220,265	\$177,030 \$232,029	
\$2,217,372	\$00,521 \$ 2,379,055	\$71,372 \$ 2,548,44 4	\$76,433 \$ 2,725,855	\$81,776 \$2,911,617	\$3,106,071	\$95,182 \$ 3,309,572	\$99,287 \$3,522,488	\$105,675 \$3,745,200	\$3,978,105	\$119,545 \$ 4,221,613	\$120,048 \$4,476,152	\$134,285 \$4,742,163	\$142,205 \$5,020,106	\$150,605 \$5,310,458	\$159,514 \$5,613,713	\$5,930,384	\$6,261,003	\$6,606,122	\$6,966,313	\$208,989 \$7,342,170	\$220,265 \$ 7,734,309	\$232,025 \$8,143,368	
\$184,781 \$123,785	\$190,324 \$133,042	\$196,034 \$142,743	\$201,915 \$152,907	\$207,973 \$163,551	\$214,212 \$174,697	\$220,638 \$186,364	\$227,257 \$198,574	\$234,075 \$211,349	\$241,097 \$224,712	\$248,330 \$238,686	\$255,780 \$253,297	\$263,454 \$268,569	\$271,357 \$284,530	\$279,498 \$301,206	\$287,883 \$318,628	\$296,519 \$336,823	\$305,415 \$355,823	\$314,577 \$375,660	\$324,015 \$396,367	\$333,735 \$417,979	\$343,747 \$440.530	\$354,059 \$464,059	
\$4,434,744	\$155,042 \$4,758,110	\$5,096,888	\$132,507 \$5,451,710	\$5,823,234	\$6,212,142	\$6,619,145	\$ 7,044,976	\$7,490,401	\$7,956,210	\$258,080 \$ 8,443,227	\$233,237 \$8,952,303	. ,	. ,			\$11,860,769				\$14,684,341		. ,	
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	
\$2,605 \$987	\$2,683 \$1,016	\$2,764 \$1,047	\$2,847 \$1,078	\$2,932 \$1,111	\$3,020 \$1,144	\$3,111 \$1,178	\$3,204 \$1,214	\$3,300 \$1,250	\$3,399 \$1,288	\$3,501 \$1,326	\$3,606 \$1,366	\$3,714 \$1,407	\$3,826 \$1,449	\$3,940 \$1,493	\$4,059 \$1,537	\$4,180 \$1,584	\$4,306 \$1,631	\$4,435 \$1.680	\$4,568 \$1,730	\$4,705 \$1,782	\$4,846 \$1,836	\$4,992 \$1,891	
\$26,249	\$27,036	\$27,847	\$28,683	\$29,543	\$30,429	\$31,342	\$32,283	\$33,251	\$34,249	\$35,276	\$36,334	\$37,424	\$38,547	\$39,704	\$40,895	\$42,121	\$43,385	\$44,687	\$46,027	\$47,408	\$48,830	\$50,295	
\$1,974 \$0	\$2,033 \$0	\$2,094 \$0	\$2,157 \$0	\$2,221 \$0	\$2,288 \$0	\$2,357 \$0	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,652 \$0	\$2,732 \$0	\$2,814 \$0	\$2,898 \$0	\$2,985 \$0	\$3,075 \$0	\$3,167 \$0	\$3,262 \$0	\$3,360 \$0	\$3,461 \$0	\$3,565 \$0	\$3,671 \$0	\$3,782 \$0	
\$38,327	\$39,477	\$40,661	\$41,881	\$43,137	\$44,432	\$45,765	\$47,137	\$48,552	\$50,008	\$51,508	\$53,054	\$54,645	\$56 , 285	\$57,973	\$59,712	\$61,504	\$63,349	\$65,249	\$67,207	\$69,223	\$71,300	\$73,439	
\$70,195	\$72,301	\$74,470	\$76,704	\$79,005	\$81,375	\$83,816	\$86,331	\$88,921	\$91,588	\$94,336	\$97,166	\$100,081	\$103,083	\$106,176	\$109,361	\$112,642	\$116,021	\$119,502	\$123,087	\$126,780	\$130,583	\$134,500	D
\$83,524 \$96,853	\$86,030 \$99,759	\$88,611 \$102,752	\$91,269 \$105,834	\$94,007 \$109.009	\$96,827 \$112,279	\$99,732 \$115,648	\$102,724 \$119,117	\$105,806 \$122,691	\$108,980 \$126,371	\$112,249 \$130,163	\$115,617 \$134,067	\$119,085 \$138.089	\$122,658 \$142,232	\$126,337 \$146,499	\$130,128 \$150,894	\$134,031 \$155,421	\$138,052 \$160.084	\$142,194 \$164.886	\$146,460 \$169.833	\$150,854 \$174.928	\$155,379 \$180,175	\$160,041 \$185,581	
\$96,853 \$110,182	\$99,759 \$113,488	\$102,752 \$116,892	\$105,834 \$120,399	\$109,009 \$124,011	\$112,279 \$127,732	\$115,648 \$131,564	\$119,117 \$135,510	\$122,691 \$139,576	\$126,371 \$143,763	\$130,163 \$148,076	\$134,067 \$152,518	\$138,089 \$157,094	\$142,232 \$161,807	\$146,499 \$166,661	\$150,894 \$171,661	\$155,421 \$176,810	\$180,084 \$182,115	\$164,886 \$187,578	\$169,833 \$193,205	\$174,928 \$199,002	\$180,175 \$204,972	\$185,581 \$211,121	
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	Γ

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

47	48	49	50
601 170	ć02.005	ćoc 722	¢00.634
\$91,170 \$122,151	\$93,905 \$128,550	\$96,723 \$135,224	\$99,624 \$142,182
\$4,285,005	\$4,507,460	\$4,739,407	\$4,981,213
<i>ų</i> 1 <u>,</u> 200,000	<i>ϕ</i> ., <i>σσ</i> , ,	<i>ϕ</i> (<i>)</i> (05) (07	<i>↓</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
\$182,341	\$187,811	\$193,445	\$199,249
\$244,301	\$257,100	\$270,448	\$284,364
\$8,570,009	\$9,014,920	\$9,478,813	\$9,962,426
\$364,681	\$375,622	\$386,890	\$398,497
\$488,602	\$514,201	\$540,895	\$568,729
\$17,140,019	\$18,029,842	\$18,957,627	\$19,924,853
*** ***			
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$51,804	\$53,358	\$54,959	\$56,608
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$75,642	\$77,911	\$80,248	\$82,656
\$138,535	\$142,691	\$146,972	\$151,381
\$164,842	\$169,787	\$174,881	\$180,127
\$191,148	\$196,883	\$202,789	\$208,873
\$217,454	\$223,978	\$230,697	\$237,618
			î
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 3, Design Flow Rate = 40 CFS

ASSUMPTIONS:					
Estimated Capit	al Cost:		\$3,970,000	Total Capita	l Cost
Interest on Repl	acement F	und:	3.00%		
Rate of Inflation	:		3.00%		
Project Design L	ife:		50	Years	
SUMMARY REP	LACEMENT	COSTS:	CURREN	IT COST ²	FUTURE COST ³
Estimated Proje	ct Replacer	nent Cost:			
To Replace	25%	After Life of Project			\$4,351,027
To Replace	50%	After Life of Project			\$8,702,053
To Replace	100%	After Life of Project	\$3,970,000		\$17,404,107

\$99,000

\$4,069,000

REPLACEMENT	FUND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Mad	le Each Year):	
To Replace	25%	After Life of Project	\$39,536	
To Replace	50%	After Life of Project	\$79,072	
To Replace	100%	After Life of Project	\$158,144	
Deposit Require	d at Year 1	(Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$20,955	
To Replace	50%	After Life of Project	\$41,911	
To Replace	100%	After Life of Project	\$83,821	
Deposit Require	d at Year 2	5 (Assume Deposits Increa	ase at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$42,598	
To Replace	50%	After Life of Project	\$85,196	
To Replace	100%	After Life of Project	\$170,392	
Deposit Require	d at Year 5	0 (Assume Deposits Increa	ase at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$89,191	
To Replace	50%	After Life of Project	\$178,381	
To Replace	100%	After Life of Project	\$356,762	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 50th Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,017,250	\$901,000	\$1,724,015	\$3,642,265
50% Replacement	\$2,034,500	\$901,000	\$1,724,015	\$4,659,515
100% Replacement	\$4,069,000	\$901,000	\$1,724,015	\$6,694,015
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,017,250	\$901,000	\$2,051,366	\$3,969,616
50% Replacement	\$2,034,500	\$901,000	\$2,051,366	\$4,986,866
100% Replacement	\$4,069,000	\$901,000	\$2,051,366	\$7,021,366
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,017,250	\$901,000	\$2,378,716	\$4,296,966
50% Replacement	\$2,034,500	\$901,000	\$2,378,716	\$5,314,216
100% Replacement	\$4,069,000	\$901,000	\$2,378,716	\$7,348,716
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ition:	
25% Replacement	\$1,017,250	\$901,000	\$2,706,067	\$4,624,317
50% Replacement	\$2,034,500	\$901,000	\$2,706,067	\$5,641,567
100% Replacement	\$4,069,000	\$901,000	\$2,706,067	\$7,676,067

LIFE CYCLE COSTS:

LIFE CTCLE COSTS:			-	-				-													_
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Disposal and Removal Cost:

Total Replacement Cost:

 To Replace
 25%
 After Life of Project

 To Replace
 50%
 After Life of Project

To Replace 100% After Life of Project

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

LIFE CYCLE COSTS:																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$3,970,000																							
Replacement Fund (For Funding Replaceme	nt of 25% of Sys	tem):																						
Deposits		\$20,955	\$21,584	\$22,232	\$22,898	\$23,585	\$24,293	\$25,022	\$25,772	\$26,546	\$27,342	\$28,162	\$29,007	\$29,877	\$30,774	\$31,697	\$32,648	\$33,627	\$34,636	\$35,675	\$36,745	\$37,848	\$38,983	\$40,153
Interest		\$0		\$1,295	\$2,001	\$2,748	\$3,538	\$4,373	\$5,255	\$6,185	\$7,167	\$8,203	\$9,294	\$10,443	\$11,652	\$12,925	\$14,264	\$15,671	\$17,150	\$18,703	\$20,335	\$22,047	\$23,844	\$25,729
End of Year Balance		\$20,955	\$43,168	\$66,695	\$91,594	\$117,927	\$145,758	\$175,152	\$206,179	\$238,910	\$273,420	\$309,785	\$348,085	\$388,405	\$430,831	\$475,453	\$522,364	\$571,662	\$623,448	\$677,826	\$734,907	\$794,801	\$857,629	\$923,510
Replacement Fund (For Funding Replaceme	nt of 50% of Sys	tem):																						
Deposits		\$41,911	\$43,168	\$44,463	\$45,797	\$47,171	\$48,586	\$50,044	\$51,545	\$53,091	\$54,684	\$56,324	\$58,014	\$59,755	\$61,547	\$63,394	\$65,296	\$67,254	\$69,272	\$71,350	\$73,491	\$75,695	\$77,966	\$80,305
Interest		\$0	\$1,257	\$2,590	\$4,002	\$5,496	\$7,076	\$8,745	\$10,509	\$12,371	\$14,335	\$16,405	\$18,587	\$20,885	\$23,304	\$25,850	\$28,527	\$31,342	\$34,300	\$37,407	\$40,670	\$44,094	\$47,688	\$51,458
End of Year Balance		\$41,911	\$86,336	\$133,389	\$183,188	\$235,854	\$291,516	\$350,305	\$412,359	\$477,821	\$546,840	\$619,569	\$696,171	\$776,810	\$861,662	\$950,905	\$1,044,728	\$1,143,324	\$1,246,896	\$1,355,653	\$1,469,813	\$1,589,603	\$1,715,257	\$1,847,020
Replacement Fund (For Funding Replaceme	nt of 100% of S	/stem):																						
Deposits		\$83,821	\$86,336	\$88,926	\$91,594	\$94,342	\$97,172	\$100,087	\$103,090	\$106,182	\$109,368	\$112,649	\$116,028	\$119,509	\$123,095	\$126,787	\$130,591	\$134,509	\$138,544	\$142,700	\$146,981	\$151,391	\$155,932	\$160,610
Interest		\$0	\$2,515	\$5,180	\$8,003	\$10,991	\$14,151	\$17,491	\$21,018	\$24,742	\$28,669	\$32,810	\$37,174	\$41,770	\$46,609	\$51,700	\$57,054	\$62,684	\$68,599	\$74,814	\$81,339	\$88,189	\$95,376	\$102,915
End of Year Balance		\$83,821	\$172,672	\$266,778	\$366,376	\$471,709	\$583,032	\$700,610	\$824,718	\$955,642	\$1,093,679	\$1,239,139	\$1,392,341	\$1,553,621	\$1,723,324	\$1,901,811	\$2,089,456	\$2,286,649	\$2,493,792	\$2,711,306	\$2,939,627	\$3,179,206	\$3,430,515	\$3,694,041
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$11,900	\$12,257	\$12,625	\$13.003	\$13,394	\$13.795	\$14,209	\$14,635	\$15,075	\$15,527	\$15,993	\$16.472	\$16.967	\$17.476	\$18,000	\$18,540	\$19,096	\$19.669	\$20,259	\$20.867	\$21,493	\$22,138	\$22,802
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$18,020	\$18,561	\$19,117	\$19,691	\$20,282	\$20,890	\$21,517	\$22,162	\$22,827	\$23,512	\$24,217	\$24,944	\$25,692	\$26,463	\$27,257	\$28,075	\$28,917	\$29,784	\$30,678	\$31,598	\$32,546	\$33,523	\$34,528
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$34,480	\$35,515	\$36,580	\$37,678	\$38,808	\$39,972	\$41,171	\$42,406	\$43,679	\$44,989	\$46,339	\$47,729	\$49,161	\$50,635	\$52,155	\$53,719	\$55,331	\$56,991	\$58,700	\$60,461	\$62,275	\$64,144	\$66,068
4-Week Annual Pumping Duration ⁸		\$41,027	\$42,258	\$43,526	\$44,832	\$46,177	\$47,562	\$48,989	\$50,458	\$51,972	\$53,531	\$55,137	\$56,791	\$58,495	\$60,250	\$62,058	\$63,919	\$65,837	\$67,812	\$69,846	\$71,942	\$74,100	\$76,323	\$78,613
6-Week Annual Pumping Duration ⁸		\$47,574	\$49,002	\$50,472	\$51,986	\$53,545	\$55,152	\$56,806	\$58,510	\$60,266	\$62,074	\$63,936	\$65,854	\$67,830	\$69,865	\$71,960	\$74,119	\$76,343	\$78,633	\$80,992	\$83,422	\$85,925	\$88,502	\$91,157
8-Week Annual Pumping Duration ⁸		\$54,121		\$57,417	\$59,140	\$60,914	\$62,741	\$64,624	\$66,562	\$68,559	\$70,616	\$72,735	\$74,917	\$77,164	\$79,479	\$81,863	\$84,319	\$86,849	\$89,454	\$92,138	\$94,902	\$97,749	\$100,682	\$103,702
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Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$108,502 \$217,003

\$434,007

\$4,459,528

\$8,919,057

\$17,838,114

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

\$41,357

\$41,357	\$42,598	\$43,876	\$45,192	\$46,548	\$47,944	\$49,383	\$50,864	\$52,390	\$53,962	\$55,581	\$57,248	\$58,965	\$60,734	\$02,550	\$64,433	\$66,366	\$08,357	\$70,408	\$72,520	\$74,696	\$76,937	\$79,245	
\$27,705 \$992,573	\$29,777 \$1.064.948	\$31,948 \$1.140.772	\$34,223 \$1,220,187	\$36,606 \$1.303.341	\$39,100 \$1.390.385	\$41,712 \$1.481.480	\$44,444 \$1.576.788	\$47,304 \$1.676.482	\$50,294 \$1,780,738	\$53,422 \$1,889,741	\$56,692 \$2,003,681	\$60,110 \$2,122,757	\$63,683 \$2.247.174	\$67,415 \$2.377.146	\$71,314 \$2,512,893	\$75,387 \$2.654.646	\$79,639 \$2.802.643	\$84,079 \$2.957.130	\$88,714 \$3,118,364	\$93,551 \$3.286.610	\$98,598 \$3,462,145	\$103,864 \$3,645,25 4	
Ş992,573	ŞI,064,948	\$1,140,772	\$1,220,187	\$1,505,541	\$1,390,385	\$1,481,480	\$1,570,788	Ş1,070,482	\$1,780,758	\$1,889,741	\$2,003,681	\$2,122,757	<i>\$2,247,174</i>	\$2,377,140	\$2,512,895	ŞZ,054,040	ŞZ,8UZ,643	\$2,957,130	Ş 3,118,30 4	\$ 5,200,01 0	Ş 5,402,14 5	\$3,043,2 34	•
\$82,714	\$85,196	\$87,752	\$90,384	\$93,096	\$95,889	\$98,765	\$101,728	\$104,780	\$107,924	\$111,161	\$114,496	\$117,931	\$121,469	\$125,113	\$128,866	\$132,732	\$136,714	\$140,816	\$145,040	\$149,391	\$153,873	\$158,489	
\$55,411	\$59,554	\$63,897	\$68,446	\$73,211	\$78,200	\$83,423	\$88,889	\$94,607	\$100,589	\$106,844	\$113,384	\$120,221	\$127,365	\$134,830	\$142,629	\$150,774	\$159,279	\$168,159	\$177,428	\$187,102	\$197,197	\$207,729	
\$1,985,145	\$2,129,896	\$2,281,544	\$2,440,375	\$2,606,682	\$2,780,771	\$2,962,959	\$3,153,576	\$3,352,964	\$3,561,476	\$3,779,482	\$4,007,362	\$4,245,514	\$4,494,348	\$4,754,292	\$5,025,787	\$5,309,293	\$5,605,286	\$5,914,260	\$6,236,728	\$6,573,221	\$6,924,291	\$7,290,509	¢
\$165,429	\$170,392	\$175,503	\$180,769	\$186,192	\$191,777	\$197,531	\$203,457	\$209,560	\$215,847	\$222,322	\$228,992	\$235,862	\$242,938	\$250,226	\$257,733	\$265,465	\$273,429	\$281,631	\$290,080	\$298,783	\$307,746	\$316,979	9
\$110,821	\$119,109	\$127,794	\$136,893	\$146,422	\$156,401	\$166,846	\$177,778	\$189,215	\$201,178	\$213,689	\$226,769	\$240,442	\$254,731	\$269,661	\$285,258	\$301,547	\$318,558	\$336,317	\$354,856	\$374,204	\$394,393	\$415,457	
\$3,970,291	\$4,259,791	\$4,563,088	\$4,880,750	\$5,213,364	\$5,561,542	\$5,925,919	\$6,307,153	\$6,705,928	\$7,122,953	\$7,558,964	\$8,014,725	\$8,491,028	\$8,988,697	\$9,508,584	\$10,051,574	\$10,618,586	\$11,210,572	\$11,828,520	\$12,473,456	\$13,146,443	\$13,848,582	\$14,581,019) \$
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	Э
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	2
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	1
\$23,486	\$24,190	\$24,916	\$25,663	\$26,433	\$27,226	\$28,043	\$28,884	\$29,751	\$30,643	\$31,563	\$32,510	\$33,485	\$34,490	\$35,524	\$36,590	\$37,688	\$38,818	\$39,983	\$41,182	\$42,418	\$43,690	\$45,001	1
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	2
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0		
\$35,564	\$36,631	\$37,730	\$38,862	\$40,028	\$41,228	\$42,465	\$43,739	\$45,051	\$46,403	\$47,795	\$49,229	\$50,706	\$52,227	\$53,794	\$55,408	\$57,070	\$58,782	\$60,545	\$62,362	\$64,233	\$66,160	\$68,144	4
\$68,050	\$70,091	\$72,194	\$74,360	\$76,591	\$78,888	\$81,255	\$83,693	\$86,204	\$88,790	\$91,453	\$94,197	\$97,023	\$99,934	\$102,932	\$106,019	\$109,200	\$112,476	\$115,850	\$119,326	\$122,906	\$126,593	\$130,391	1
\$80,971	\$83,400	\$85,902	\$88,479	\$91,134	\$93,868	\$96,684	\$99,584	\$102,572	\$105,649	\$108,818	\$112,083	\$115,445	\$118,909	\$122,476	\$126,150	\$129,935	\$133,833	\$137,848	\$141,983	\$146,243	\$150,630		
	\$85,400 \$96.709	\$85,902 \$99,610	\$102,598	\$91,134	\$95,868 \$108,847	\$96,684	\$99,584 \$115,475	\$102,572	\$105,649	\$106,818	\$112,085	\$113,445	\$118,909	\$122,476	\$126,150	\$129,935	\$155,855	\$157,848	\$141,985	\$146,245	\$150,650	\$155,145	
\$93,892	,																	. ,			, , , , , , , , , , , , , , , , , , , ,	. ,	
\$106,813	\$110,018	\$113,318	\$116,718	\$120,219	\$123,826	\$127,540	\$131,367	\$135,308	\$139,367	\$143,548	\$147,854	\$152,290	\$156,859	\$161,564	\$166,411	\$171,404	\$176,546	\$181,842	\$187,297	\$192,916	\$198,704	\$204,665	,
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	
																						1	

\$53,962 \$55,581

\$57,248 \$58,965 \$60,734 \$62,556

\$64,433 \$66,366

\$68,357 \$70,408 \$72,520 \$74,696 \$76,937 \$79,245

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

\$42,598 \$43,876 \$45,192 \$46,548 \$47,944 \$49,383 \$50,864 \$52,390

47	48	49	50
\$81,622	\$84,071	\$86,593	\$89,191
\$109,358	\$115,087	\$121,062	\$127,291
\$3,836,234	\$4,035,392	\$4,243,046	\$4,459,528
\$163,244	\$168,141	\$173,186	\$178,381
\$218,715	\$230,174	\$242,124	\$254,583
\$7,672,469	\$8,070,784	\$8,486,093	\$8,919,057
\$326,488	\$336,283	\$346,371	\$356,762
\$437,431	\$460,348	\$484,247	\$509,166
\$15,344,937	\$16,141,568	\$16,972,186	\$17,838,114
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$46,351	\$47,742	\$49,174	\$50,649
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$70,189	\$72,294	\$74,463	\$76,697
\$134,302	\$138,331	\$142,481	\$146,756
\$159,803	\$164,597	\$169,535	\$174,621
\$185,304	\$190,863	\$196,589	\$202,487
\$210,805	\$217,129	\$223,643	\$230,352
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 4, Design Flow Rate = 40 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$5,362,000 Total Capita	l Cost
Interest on Repla	acement Fu	und:	3.00%	
Rate of Inflation:			3.00%	
Project Design Li	fe:		50 Years	
SUMMARY REPL	ACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Project	-		CONNENT COST	TOTORE COST
To Replace	25%	After Life of Project		\$5,876,626
To Replace	50%	After Life of Project		\$11,753,252
To Replace	100%	After Life of Project	\$5,362,000	\$23,506,504
Disposal and Rer	noval Cost	:		
To Replace	25%	After Life of Project		\$146,861
To Replace	50%	After Life of Project		\$293,722
To Replace	100%	After Life of Project	\$134,000	\$587,443
Total Replaceme	ent Cost:			
To Replace	25%	After Life of Project		\$6,023,487
To Replace	50%	After Life of Project		\$12,046,974
To Replace	100%	After Life of Project	\$5,496,000	\$24,093,947

\$5,362,000

REPLACEMENT F	UND SUM	MARY		
Annual Deposit F	Required (A	Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$53,401	
To Replace	50%	After Life of Project	\$106,802	
To Replace	100%	After Life of Project	\$213,605	
Deposit Required	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$28,304	
To Replace	50%	After Life of Project	\$56,609	
To Replace	100%	After Life of Project	\$113,218	
Deposit Required	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$57,537	
To Replace	50%	After Life of Project	\$115,074	
To Replace	100%	After Life of Project	\$230,148	
Deposit Required	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$120,470	
To Replace	50%	After Life of Project	\$240,939	
To Replace	100%	After Life of Project	\$481,879	

input cells - Aujust Using Goa	I Seek TOOL to IV	lake Account B	alance at enu o	1
5oth Year Equal to Future Val	ue of Replacem	ent Cost		
OTAL LONG-TERM COST SUMMARY:				
PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$1,374,000	\$1,111,000	\$1,634,899	\$4,119,899
0% Replacement	\$2,748,000	\$1,111,000	\$1,634,899	\$5,493,899
.00% Replacement	\$5,496,000	\$1,111,000	\$1,634,899	\$8,241,899
Assuming the Pumping Power Costs for a 4-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$1,374,000	\$1,111,000	\$1,945,292	\$4,430,292
60% Replacement	\$2,748,000	\$1,111,000	\$1,945,292	\$5,804,292
00% Replacement	\$5,496,000	\$1,111,000	\$1,945,292	\$8,552,292
Assuming the Pumping Power Costs for a 6-v	veek Annual Op	erating Duratio	on:	
25% Replacement	\$1,374,000	\$1,111,000	\$2,255,686	\$4,740,686
60% Replacement	\$2,748,000	\$1,111,000	\$2,255,686	\$6,114,686
.00% Replacement	\$5,496,000	\$1,111,000	\$2,255,686	\$8,862,686
Assuming the Pumping Power Costs for an 8-	week Annual O	perating Durat	ion:	
5% Replacement	\$1,374,000	\$1,111,000	\$2,566,079	\$5,051,079
50% Replacement	\$2,748,000	\$1,111,000	\$2,566,079	\$6,425,079
.00% Replacement	\$5,496,000	\$1,111,000	\$2,566,079	\$9,173,079

Input Cells - Assumed or Given Values

As 25 50

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LIFE CYCLE COSTS:

LIFE CICLE COSIS:																					
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

		,																			
Replacement Fund (For Funding Replacement of 2)	25% of Syst	- /	620.454	620.020	¢20.020	624.057	622.042	¢22 707	624.014	625 055	626.024	ć20.020	¢20.400	¢40.255	644 FCC	¢ 42,042	ć 44 007	Ć 45 430	¢46 700	¢ 40, 400	
Deposits		\$28,304 \$0	\$29,154	\$30,028 \$1.749	\$30,929	\$31,857	\$32,813 \$4,779	\$33,797	\$34,811 \$7.097	\$35,855 \$8.355	\$36,931	\$38,039	\$39,180	\$40,355 \$14.105	\$41,566	\$42,813	\$44,097	\$45,420	\$46,783	\$48,186 \$25,263	
Interest End of Year Balance		\$0 \$28.304	\$849 \$58.307	\$1,749 \$90.084	\$2,703 \$123.716	\$3,711 \$159,284	\$4,779 \$196.875	\$5,906 \$236,579	\$7,097 \$278.487	\$8,355 \$322.697	\$9,681 \$369,308	\$11,079 \$418.426	\$12,553 \$470,159	\$14,105 \$524.619	\$15,739 \$581.924	\$17,458 \$642,194	\$19,266 \$705,557	\$21,167 \$772.144	\$23,164 \$842.092	\$25,263 \$915,541	
		320,30 4	\$38,507	\$50,084	\$125,710	ŞIJJ,204	3130,873	3230,375	<i>3210,</i> 407	3322,097	<i>3303,30</i> 8	3410,420	3470,133	Ş324,019	<i>3301,32</i> 4	3042,1 94	\$705,557	\$772,144	3042,U32	3913,341	
Replacement Fund (For Funding Replacement of 5	50% of Syst	em):																			
Deposits		\$56,609	\$58,307	\$60,056	\$61,858	\$63,714	\$65,625	\$67,594	\$69,622	\$71,710	\$73,862	\$76,077	\$78,360	\$80,711	\$83,132	\$85,626	\$88,195	\$90,841	\$93,566	\$96,373	
Interest		\$0	\$1,698	\$3,498	\$5,405	\$7,423	\$9,557	\$11,813	\$14,195	\$16,709	\$19,362	\$22,158	\$25,106	\$28,210	\$31,477	\$34,915	\$38,532	\$42,333	\$46,329	\$50,525	
End of Year Balance		\$56,609	\$116,614	\$180,169	\$247,432	\$318,569	\$393,751	\$473,157	\$556,973	\$645,393	\$738,616	\$836,852	\$940,318	\$1,049,238	\$1,163,847	\$1,284,388	\$1,411,115	\$1,544,289	\$1,684,183	\$1,831,081	
Replacement Fund (For Funding Replacement of 1	100% of Sys	stem):																			
Deposits	, i	\$113,218	\$116,614	\$120,113	\$123,716	\$127,427	\$131,250	\$135,188	\$139,243	\$143,421	\$147,723	\$152,155	\$156,720	\$161,421	\$166,264	\$171,252	\$176,389	\$181,681	\$187,131	\$192,745	
Interest		\$0	\$3,397	\$6,997	\$10,810	\$14,846	\$19,114	\$23,625	\$28,389	\$33,418	\$38,724	\$44,317	\$50,211	\$56,419	\$62,954	\$69,831	\$77,063	\$84,667	\$92,657	\$101,051	
End of Year Balance		\$113,218	\$233,228	\$360,338	\$494,864	\$637,137	\$787,501	\$946,314	\$1,113,947	\$1,290,786	\$1,477,233	\$1,673,705	\$1,880,636	\$2,098,476	\$2,327,694	\$2,568,777	\$2,822,229	\$3,088,577	\$3,368,366	\$3,662,162	
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$16,100	\$16,583	\$17,080	\$17,593	\$18,121	\$18,664	\$19,224	\$19,801	\$20,395	\$21,007	\$21,637	\$22,286	\$22,955	\$23,643	\$24,353	\$25,083	\$25,836	\$26,611	\$27,409	
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
Total O&M Expenses		\$22,220	\$22,887	\$23,573	\$24,280	\$25,009	\$25,759	\$26,532	\$27,328	\$28,148	\$28,992	\$29,862	\$30,758	\$31,680	\$32,631	\$33,610	\$34,618	\$35,657	\$36,726	\$37,828	
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$32,698	\$33,679	\$34,689	\$35,730	\$36,802	\$37,906	\$39,043	\$40,214	\$41,421	\$42,663	\$43,943	\$45,262	\$46,619	\$48,018	\$49,459	\$50,942	\$52,471	\$54,045	\$55,666	
4-Week Annual Pumping Duration ⁸		\$38,906	\$40,073	\$41,275	\$42,513	\$43,789	\$45,103	\$46,456	\$47,849	\$49,285	\$50,763	\$52,286	\$53,855	\$55,470	\$57,135	\$58,849	\$60,614	\$62,432	\$64,305	\$66,235	
6-Week Annual Pumping Duration ⁸		\$45,114	\$46,467	\$47,861	\$49,297	\$50,776	\$52,299	\$53,868	\$55,484	\$57,149	\$58,863	\$60,629	\$62,448	\$64,321	\$66,251	\$68,239	\$70,286	\$72,394	\$74,566	\$76,803	
8-Week Annual Pumping Duration ⁸		\$51,322	\$52,861	\$54,447	\$56,080	\$57,763	\$59,496	\$61,281	\$63,119	\$65,013	\$66,963	\$68,972	\$71,041	\$73,172	\$75,367	\$77,629	\$79,957	\$82,356	\$84,827	\$87,372	
						-	•					•		-		-					
						_		-											40		Ē
Year	0	1	2	3	4	5	ь	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:
1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

8) Assumes pumping power costs, or power rates, increase at the assumed rate of inflation.

Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of

20	21	22	23
\$49,632	\$51,121	\$52,655	\$54,234
\$27,466	\$29,779	\$32,206	\$34,752
\$992,639	\$1,073,539	\$1,158,400	\$1,247,386
,,			.,,,
\$99,264	\$102,242	\$105,309	\$108,468
\$54,932	\$59,558	\$64,412	\$69,504
\$1,985,278	\$2,147,078	\$2,316,799	\$2,494,771
6100 F30	6204 494	6240 640	6246 027
\$198,528 \$109,865	\$204,484 \$119,117	\$210,618 \$128,825	\$216,937 \$139,008
\$3,970,555	\$4,294,155	\$4,633,598	\$4,989,542
<i>33,370,333</i>	J4,234,133	,033,398	J4,565,542
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$28,231	\$29,078	\$29,951	\$30,849
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$38,963	\$40,132	\$41,336	\$42,576
\$57,336	\$59,056	\$60,828	\$62,653
\$68,222	\$70,268	\$72,376	\$74,548
\$79,107	\$81,480	\$83,925	\$86,443
\$89,993	\$92,692	\$95,473	\$98,337
20	21	22	23

\$55,861	\$57,537	\$59,263	\$61,041	\$62,872	\$64,758	\$66,701	\$68,702	\$70,763	\$72,886	\$75,073	\$77,325	\$79,645	\$82,034	\$84,495	\$87,030	\$89,641	\$92,330	\$95,100	\$97,953	\$100,892	\$103,918	\$107,036	j
\$37,422	\$40,220	\$43,153	\$46,225	\$49,443	\$52,813	\$56,340	\$60,031	\$63,893	\$67,933	\$72,157	\$76,574	\$81,191	\$86,016	\$91,058	\$96,324	\$101,825	\$107,569	\$113,566	\$119,826	\$126,359	\$133,177	\$140,290	j i
\$1,340,668	\$1,438,425	\$1,540,841	\$1,648,108	\$1,760,423	\$1,877,994	\$2,001,035	\$2,129,768	\$2,264,425	\$2,405,244	\$2,552,474	\$2,706,373	\$2,867,209	\$3,035,259	\$3,210,812	\$3,394,166	\$3,585,632	\$3,785,531	\$3,994,197	\$4,211,976	\$4,439,226	\$4,676,322	\$4,923,647	Ş
6144 700	6445.074	6140 526	¢122.002	6425 745	6420 547	¢122.402	6127 404	6444 527	64.45 772	6450.446	6454 CEO	6450 200	6464 060	Ć160.000	6474.000	6170 202	6404 CC0	¢100.200	6105 00C	6201 702	\$207.837	6244.072	
\$111,722 \$74,843	\$115,074 \$80,440	\$118,526 \$86,306	\$122,082 \$92,450	\$125,745 \$98,886	\$129,517 \$105,625	\$133,402 \$112,680	\$137,404 \$120,062	\$141,527 \$127,786	\$145,772 \$135,865	\$150,146 \$144,315	\$154,650 \$153,148	\$159,289 \$162,382	\$164,068 \$172,033	\$168,990 \$182,116	\$174,060 \$192,649	\$179,282 \$203,650	\$184,660 \$215,138	\$190,200 \$227,132	\$195,906 \$239,652	\$201,783 \$252,719	\$207,837 \$266,354	\$214,072 \$280,579	
\$2,681,337	\$2,876,851	\$3,081,683	\$3,296,215	\$3,520,846	\$3,755,988	\$4,002,070	\$120,002 \$4,259,537	\$4,528,850	\$135,805 \$4,810,487	\$5,104,948	\$5,412,746	\$5,734,418	\$6,070,518	\$6,421,624	\$6,788,332	\$7,171,264	\$7,571,062	\$7,988,394	\$ 8,423,951	\$8,878,453	\$9,352,643		
\$223,445	\$230,148	\$237,053	\$244,164	\$251,489	\$259,034	\$266,805	\$274,809	\$283,053	\$291,545	\$300,291	\$309,300	\$318,579	\$328,136	\$337,980	\$348,120	\$358,563	\$369,320	\$380,400	\$391,812	\$403,566	\$415,673	\$428,143	,
\$149,686	\$160,880	\$172,611	\$184,901	\$197,773	\$211,251	\$225,359	\$240,124	\$255,572	\$271,731	\$288,629	\$306,297	\$324,765	\$344,065	\$364,231	\$385,297	\$407,300	\$430,276	\$454,264	\$479,304	\$505,437	\$532,707	\$561,159)
\$5,362,673	\$5,753,702	\$6,163,365	\$6,592,430	\$7,041,692	\$7,511,977	\$8,004,141	\$8,519,074	\$9,057,699	\$9,620,975	\$10,209,895	\$10,825,491	\$11,468,835	\$12,141,036	\$12,843,247	\$13,576,664	\$14,342,527	\$15,142,123	\$15,976,787	\$16,847,902	\$17,756,905	\$18,705,285	\$19,694,587	\$2
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	J
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	
\$31,775	\$32,728	\$33,710	\$34,721	\$35,763	\$36,836	\$37,941	\$39,079	\$40,251	\$41,459	\$42,703	\$43,984	\$45,303	\$46,662	\$48,062	\$49,504	\$50,989	\$52,519	\$54,094	\$55,717	\$57,389	\$59,110	\$60,884	
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	1.5	
\$43,853	\$45,169	\$46,524	\$47,919	\$49,357	\$50,838	\$52,363	\$53,934	\$55,552	\$57,218	\$58,935	\$60,703	\$62,524	\$64,400	\$66,332	\$68,322	\$70,371	\$72,482	\$74,657	\$76,897	\$79,204	\$81,580	\$84,027	
\$64,532	\$66,468	\$68,462	\$70,516	\$72,632	\$74,811	\$77,055	\$79,367	\$81,748	\$84,200	\$86,726	\$89,328	\$92,008	\$94,768	\$97,611	\$100,539	\$103,555	\$106,662	\$109,862	\$113,158	\$116,552	\$120,049	\$123,651	
\$76,784	\$79,088	\$81,460	\$83,904	\$86,421	\$89,014	\$91,684	\$94,435	\$97,268	\$100,186	\$103,191	\$106,287	\$109,476	\$112,760	\$116,143	\$119,627	\$123,216	\$126,912	\$130,720	\$134,641	\$138,681	\$142,841	\$147,126	,
\$89,036	\$91,707	\$94,458	\$97,292	\$100,211	\$103,217	\$106,313	\$109,503	\$112,788	\$116,172	\$119,657	\$123,246	\$126,944	\$130,752	\$134,675	\$138,715	\$142,876	\$147,163	\$151,578	\$156,125	\$160,809	\$165,633	\$170,602	
\$101,288	\$104,326	\$107,456	\$110,680	\$114,000	\$117,420	\$120,943	\$124,571	\$128,308	\$132,157	\$136,122	\$140,206	\$144,412	\$148,744	\$153,207	\$157,803	\$162,537	\$167,413	\$172,435	\$177,608	\$182,937	\$188,425	\$194,078	i.
																							Г
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

47	48	49	50
\$110,247	\$113,554	\$116,961	\$120,470
\$147,709	\$155,448	\$163,518	\$171,933
\$5,181,603	\$5,450,606	\$5,731,085	\$6,023,487
\$220,494	\$227,109	\$233,922	\$240,939
\$295,419	\$310,896	\$327,036	\$343,865
\$10,363,207	\$10,901,211	\$11,462,169	\$12,046,974
\$440,987	\$454,217	\$467,844	\$481,879
\$590,838	\$621,792	\$654,073	\$687,730
\$20,726,412	\$21,802,422	\$22,924,338	\$24,093,947
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$62,710	\$64,592	\$66,529	\$68,525
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$86,548	\$89,144	\$91,819	\$94,573
\$127,360	\$131,181	\$135,116	\$139,170
\$151,540	\$156,086	\$160,769	\$165,592
\$175,720	\$180,991	\$186,421	\$192,014
\$199,900	\$205,897	\$212,074	\$218,436
47	48	49	50
	-	-	

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis Alternative 5, Design Flow Rate = 40 CFS

ASSUMPTIONS:				
Estimated Capit	al Cost:		\$6,153,000 Total Capi	tal Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REP	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replace	ment Cost:		
To Replace	25%	After Life of Project		\$6,743,543
To Replace	50%	After Life of Project		\$13,487,087
To Replace	100%	After Life of Project	\$6,153,000	\$26,974,174
Disposal and Re	moval Cost	:		
To Replace	25%	After Life of Project		\$168,780
To Replace	50%	After Life of Project		\$337,561
To Replace	100%	After Life of Project	\$154,000	\$675.122

\$6,307,000

REPLACEMENT F	UND SUM	IMARY		
Annual Deposit I	Required (Assume Equal Deposit Made I	Each Year):	
To Replace	25%	After Life of Project	\$61,281	
To Replace	50%	After Life of Project	\$122,562	
To Replace	100%	After Life of Project	\$245,125	
Deposit Require	d at Year 1	(Assume Deposits Increase a	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$32,481	
To Replace	50%	After Life of Project	\$64,962	
To Replace	100%	After Life of Project	\$129,924	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$66,027	
To Replace	50%	After Life of Project	\$132,055	
To Replace	100%	After Life of Project	\$264,109	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$138,246	
To Replace	50%	After Life of Project	\$276,493	

	Input Cells - Assumed or Give Input Cells - Adjust Using Goa		Make Account I	E
	5oth Year Equal to Future Va	lue of Replacer	nent Cost	
LONG-	TERM COST SUMMARY:			Ī
	LIE OF LONG-TERM	Replacment		

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,576,750	\$1,231,000	\$1,565,344	\$4,373,094
50% Replacement	\$3,153,500	\$1,231,000	\$1,565,344	\$5,949,844
100% Replacement	\$6,307,000	\$1,231,000	\$1,565,344	\$9,103,344
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,576,750	\$1,231,000	\$1,862,503	\$4,670,253
50% Replacement	\$3,153,500	\$1,231,000	\$1,862,503	\$6,247,003
100% Replacement	\$6,307,000	\$1,231,000	\$1,862,503	\$9,400,503
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,576,750	\$1,231,000	\$2,159,662	\$4,967,412
50% Replacement	\$3,153,500	\$1,231,000	\$2,159,662	\$6,544,162
100% Replacement	\$6,307,000	\$1,231,000	\$2,159,662	\$9,697,662
Assuming the Pumping Power Costs for an 8	-week Annual	Operating Dura	ition:	
25% Replacement	\$1,576,750	\$1,231,000	\$2,456,821	\$5,264,571
50% Replacement	\$3,153,500	\$1,231,000	\$2,456,821	\$6,841,321
100% Replacement	\$6,307,000	\$1,231,000	\$2,456,821	\$9,994,821

LIFE CYCLE COSTS:

Total Replacement Cost:

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

To Replace 100% After Life of Project

LIFE CYCLE COSTS:																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$6,153,000																							
Replacement Fund (For Funding Replaceme	ent of 25% of Sve	tem):																						
Deposits		\$32,481	\$33,455	\$34,459	\$35,493	\$36,558	\$37,654	\$38,784	\$39,948	\$41,146	\$42,380	\$43,652	\$44,961	\$46,310	\$47,700	\$49,131	\$50,604	\$52,123	\$53,686	\$55,297	\$56,956	\$58,664	\$60,424	\$62,237
Interest		\$0	\$974	\$2,007	\$3,101	\$4,259	\$5,484	\$6,778	\$8,145	\$9,587	\$11,109	\$12,714	\$14,405	\$16,186	\$18,061	\$20,034	\$22,109	\$24,290	\$26,583	\$28,991	\$31,519	\$34,173	\$36,959	\$39,880
End of Year Balance		\$32,481	\$66,911	\$103,377	\$141,972	\$182,789	\$225,927	\$271,489	\$319,581	\$370,314	\$423,804	\$480,170	\$539,536	\$602,033	\$667,793	\$736,958	\$809,671	\$886,083	\$966,352	\$1,050,639	\$1,139,114	\$1,231,952	\$1,329,335	\$1,431,452
Replacement Fund (For Funding Replaceme	ent of 50% of Sys	tem):																						
Deposits		\$64,962	\$66,911	\$68,918	\$70,986	\$73,115	\$75,309	\$77,568	\$79,895	\$82,292	\$84,761	\$87,304	\$89,923	\$92,620	\$95,399	\$98,261	\$101,209	\$104,245	\$107,372	\$110,594	\$113,911	\$117,329	\$120,849	\$124,474
Interest		\$0	\$1,949	\$4,015	\$6,203	\$8,518	\$10,967	\$13,556	\$16,289	\$19,175	\$22,219	\$25,428	\$28,810	\$32,372	\$36,122	\$40,068	\$44,217	\$48,580	\$53,165	\$57,981	\$63,038	\$68,347	\$73,917	\$79,760
End of Year Balance		\$64,962	\$133,822	\$206,755	\$283,943	\$365,577	\$451,853	\$542,977	\$639,162	\$740,628	\$847,608	\$960,340	\$1,079,073	\$1,204,066	\$1,335,587	\$1,473,915	\$1,619,341	\$1,772,167	\$1,932,704	\$2,101,279	\$2,278,229	\$2,463,904	\$2,658,670	\$2,862,904
Replacement Fund (For Funding Replaceme	ent of 100% of S	/stem):																						
Deposits		\$129,924	\$133,822	\$137,837	\$141,972	\$146,231	\$150,618	\$155,136	\$159,790	\$164,584	\$169,522	\$174,607	\$179,845	\$185,241	\$190,798	\$196,522	\$202,418	\$208,490	\$214,745	\$221,187	\$227,823	\$234,658	\$241,697	\$248,948
Interest		\$0	\$3,898	\$8,029	\$12,405	\$17,037	\$21,935	\$27,111	\$32,579	\$38,350	\$44,438	\$50,856	\$57,620	\$64,744	\$72,244	\$80,135	\$88,435	\$97,160	\$106,330	\$115,962	\$126,077	\$136,694	\$147,834	\$159,520
End of Year Balance		\$129,924	\$267,644	\$413,510	\$567,887	\$731,154	\$903,707	\$1,085,954	\$1,278,323	\$1,481,257	\$1,695,216	\$1,920,680	\$2,158,146	\$2,408,131	\$2,671,173	\$2,947,830	\$3,238,683	\$3,544,333	\$3,865,408	\$4,202,558	\$4,556,457	\$4,927,809	\$5,317,340	\$5,725,809
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$18,500	\$19,055	\$19,627	\$20,215	\$20,822	\$21,447	\$22,090	\$22,753	\$23,435	\$24,138	\$24,862	\$25,608	\$26,377	\$27,168	\$27,983	\$28,822	\$29,687	\$30,578	\$31,495	\$32,440	\$33,413	\$34,415	\$35,448
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$24,620	\$25,359	\$26,119	\$26,903	\$27,710	\$28,541	\$29,398	\$30,279	\$31,188	\$32,124	\$33,087	\$34,080	\$35,102	\$36,155	\$37,240	\$38,357	\$39,508	\$40,693	\$41,914	\$43,171	\$44,466	\$45,800	\$47,174
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$31,307	\$32,246	\$33,213	\$34,210	\$35,236	\$36,293	\$37,382	\$38,504	\$39,659	\$40,848	\$42,074	\$43,336	\$44,636	\$45,975	\$47,354	\$48,775	\$50,238	\$51,746	\$53,298	\$54,897	\$56,544	\$58,240	\$59 <i>,</i> 987
4-Week Annual Pumping Duration ⁸		\$37,250	\$38,368	\$39,519	\$40,704	\$41,925	\$43,183	\$44,479	\$45,813	\$47,187	\$48,603	\$50,061	\$51,563	\$53,110	\$54,703	\$56,344	\$58,034	\$59,775	\$61,569	\$63,416	\$65,318	\$67,278	\$69,296	\$71,375
6-Week Annual Pumping Duration ⁸		\$43,193	\$44,489	\$45,824	\$47,198	\$48,614	\$50,073	\$51,575	\$53,122	\$54,716	\$56,357	\$58,048	\$59,790	\$61,583	\$63,431	\$65,334	\$67,294	\$69,312	\$71,392	\$73,534	\$75,740	\$78,012	\$80,352	\$82,763
8-Week Annual Pumping Duration ⁸		\$49,136	\$50,611	\$52,129	\$53,693	\$55,303	\$56,963	\$58,671	\$60,432	\$62,245	\$64,112	\$66,035	\$68,016	\$70,057	\$72,158	\$74,323	\$76,553	\$78,850	\$81,215	\$83,651	\$86,161	\$88,746	\$91,408	\$94,150
						_	_	7			10							47			20	24	22	
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

\$552,986

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$6,912,324

\$13,824,648

\$27,649,295

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

8) Assumes pumping power costs, or power rates, increase at the assumed rate of inflation.

Balance at end of

\$64,104

\$64,104 \$42,944	\$66,027 \$46,155	\$68,008 \$49,520	\$70,048 \$53,046	\$72,150 \$56,739	\$74,314 \$60,606	\$76,544 \$64,653	\$78,840 \$68,889	\$81,205 \$73,321	\$83,641 \$77,957	\$86,151 \$82,805	\$88,735 \$87,874	\$91,397 \$93,172	\$94,139 \$98,709	\$96,963 \$104,494	\$99,872 \$110,538	\$102,868 \$116,850	\$105,954 \$123,442	\$109,133 \$130,324	\$112,407 \$137,508	\$115,779 \$145,005	\$119,253 \$152,829	\$122,830	
\$1,538,500	\$1,650,682	\$1,768,211	\$1,891,305	\$2,020,194	\$2,155,115	\$2,296,312	\$2,444,041	\$2,598,568	\$2,760,166	\$2,929,122	\$3,105,730	\$3,290,300	\$3,483,148	\$3,684,605	\$3,895,016	\$4,114,735	\$4,344,131	\$4,583,588	\$4,833,503	\$5,094,287		. ,	
\$128,208	\$132,055	\$136,016	\$140,097	\$144,300	\$148,629	\$153,087	\$157,680	\$162,410	\$167,283	\$172,301	\$177,470	\$182,794	\$188,278	\$193,927	\$199,744	\$205,737	\$211,909	\$218,266	\$224,814	\$231,559	\$238,505	\$245,660	0
\$85,887	\$92,310	\$99,041	\$106,093	\$113,478	\$121,212	\$129,307	\$137,779	\$146,642	\$155,914	\$165,610	\$175,747	\$186,344	\$197,418	\$208,989	\$221,076	\$233,701	\$246,884	\$260,648	\$275,015	\$290,010	\$305,657	\$321,982	2
\$3,077,000	\$3,301,364	\$3,536,422	\$3,782,611	\$4,040,389	\$4,310,229	\$4,592,623	\$4,888,082	\$5,197,135	\$5,520,332	\$5,858,243	\$6,211,461	\$6,580,599	\$6,966,295	\$7,369,211	\$7,790,031	\$8,229,469	\$8,688,262	\$9,167,176	\$9,667,005	\$10,188,574	\$10,732,737	\$11,300,379	9\$
\$256,417	\$264,109	\$272,032	\$280,193	\$288,599	\$297,257	\$306,175	\$315,360	\$324,821	\$334,566	\$344,603	\$354,941	\$365,589	\$376,556	\$387,853	\$399,489	\$411,473	\$423,818	\$436,532	\$449,628	\$463,117	\$477,010	\$491,321	1
\$171,774	\$184,620	\$198,082	\$212,185	\$226,957	\$242,423	\$258,614	\$275,557	\$293,285	\$311,828	\$331,220	\$351,495	\$372,688	\$394,836	\$417,978	\$442,153	\$467,402	\$493,768	\$521,296	\$550,031	\$580,020		. ,	
\$6,154,000	\$6,602,729	\$7,072,843	\$7,565,222	\$8,080,777	\$8,620,458	\$9,185,247	\$9,776,164	\$10,394,270	\$11,040,664	\$11,716,486	\$12,422,921	\$13,161,198	\$13,932,590	\$14,738,421	\$15,580,062	\$16,458,938	\$17,376,524	\$18,334,351	\$19,334,010	\$20,377,147	\$21,465,472	\$22,600,75	7\$
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	9
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	2
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	1
\$36,511	\$37,607	\$38,735	\$39,897	\$41,094	\$42,327	\$43,596	\$44,904	\$46,251	\$47,639	\$49,068	\$50,540	\$52,056	\$53,618	\$55,227	\$56,883	\$58,590	\$60,348	\$62,158	\$64,023	\$65,944	\$67,922	\$69,960	0
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	2
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	0
\$48,590	\$50,047	\$51,549	\$53,095	\$54,688	\$56,329	\$58,019	\$59,759	\$61,552	\$63,399	\$65,300	\$67,260	\$69,277	\$71,356	\$73,496	\$75,701	\$77,972	\$80,311	\$82,721	\$85,202	\$87,758	\$90,391	\$93,103	3
\$61,787	\$63,640	\$65,550	\$67,516	\$69,542	\$71,628	\$73,777	\$75,990	\$78,270	\$80,618	\$83,036	\$85,527	\$88,093	\$90,736	\$93,458	\$96,262	\$99,150	\$102,124	\$105,188	\$108,344	\$111,594	\$114,942	\$118,390	0
\$73,516	\$75,722	\$77,993	\$80,333	\$82,743	\$85,225	\$87,782	\$90,416	\$93,128	\$95,922	\$98,800	\$101,764	\$104,817	\$107,961	\$111,200	\$114,536	\$117,972	\$121,511	\$125,156	\$128,911	\$132,778			
\$75,516	\$75,722	\$90,437	\$93,150	\$95,945	\$98,823	\$101,788	\$104,841	\$107,987	\$111,226	\$114,563	\$101,704	\$104,817	\$125,186	\$128,942	\$132,810	\$136,794	\$140,898	\$145,125	\$149,479	\$153,963		. ,	
											. ,						. ,					. ,	
\$96,975	\$99,884	\$102,881	\$105,967	\$109,146	\$112,421	\$115,793	\$119,267	\$122,845	\$126,530	\$130,326	\$134,236	\$138,263	\$142,411	\$146,683	\$151,084	\$155,616	\$160,285	\$165,093	\$170,046	\$175,148	\$180,402	\$185,814	•
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	
																						ı	

\$66,027 \$68,008 \$70,048 \$72,150 \$74,314 \$76,544 \$78,840 \$81,205 \$83,641 \$86,151 \$88,735 \$91,397 \$94,139 \$96,963 \$99,872 \$102,868 \$105,954 \$109,133 \$112,407 \$115,779 \$119,253 \$122,830

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis (Cont.)

47	48	49	50
\$126,515	\$130,311	\$134,220	\$138,246
\$169,506	\$178,386	\$187,647	\$197,303
\$5,946,210	\$6,254,907	\$6,576,774	\$6,912,324
\$253,030	\$260,621	\$268,440	\$276,493
\$339,011	\$356,773	\$375,294	\$394,606
\$11,892,421	\$12,509,814	\$13,153,549	\$13,824,648
\$506,060	\$521,242	\$536,880	\$552,986
\$678,023	\$713,545	\$750,589	\$789,213
\$23,784,840	\$25,019,628	\$26,307,096	\$27,649,295
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$72,058	\$74,220	\$76,447	\$78,740
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$95,896	\$98,773	\$101,736	\$104,788
\$121,942	\$125,600	\$129,368	\$133,249
\$145,091	\$149,443	\$153,927	\$158,544
\$168,240	\$173,287	\$178,485	\$183,840
\$191,388	\$197,130	\$203,044	\$209,135
47	48	49	50

APPENDIX H PRELIMINARY ANALYSIS – DELIVERY TO IID CANAL

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

ALTERNATIVE: ALTERNATIVE ALIGNMENT 1 - 10 CFS DESIGN FLOW - BOOSTER PUMP FOR EXTENDING DELIVERY TO ICICLE IRRIGATION DISTRICT

SUCTION W	ATER SURFACE ELEVATI	ONS:		PIPE	SUCTION	N PIPING		PS PI	PING		DISCHAR	GE PIPING
ELEV	1146.0 feet	PID Canal Bottom		PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	1148.5 feet	PID Canal (High Flow)		NOM. DIAM. (in)					12	12		20
LWL	1146.5 feet PID Canal (Low Flow)		O.D. (in)					12	12		20	
				I.D. (in)					12	12		20
DISCHARGE	WATER SURFACE ELEVA	ATIONS:		MATERIAL					STEEL	STEEL		HDPE
ELEV	1318.0 feet	IID Canal Bottom		С					110	110		130
HIGH	1320.5 feet	IID Canal (High Flow)	1	LENGTH (feet)					8	8		2,250
LWL	1318.5 feet	IID Canal (Low Flow)	1	K					10	10		10

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2244	5.0	188	152.1
2	4488	10.0	196	317.1
*Assumes	70%	Efficiency		

BY:

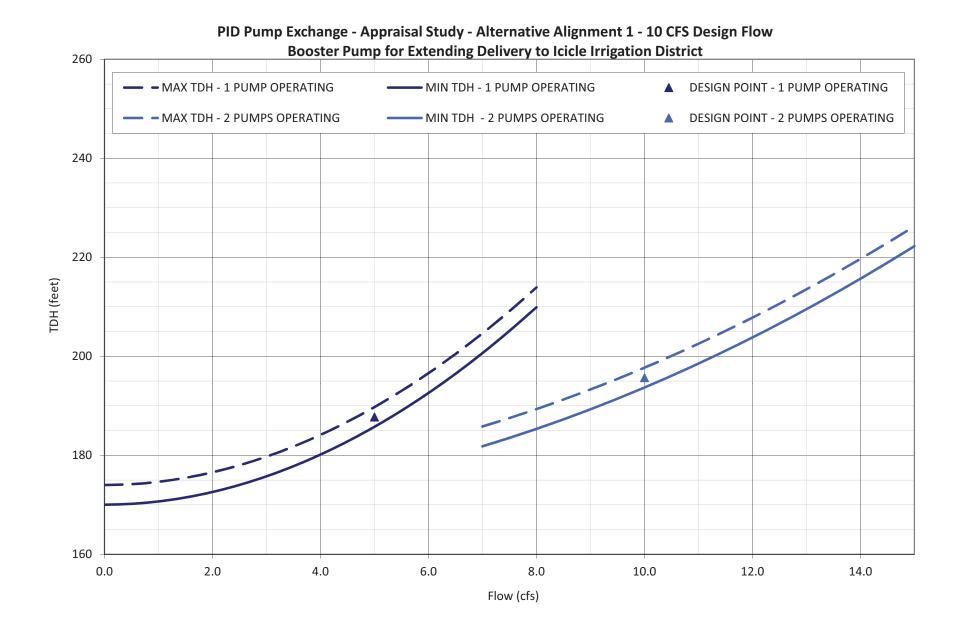
DATE:

David Rice, P.E.

14-Sep-12

TOTA	. FLOW		VELO	CITIES		SUCTION	I LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DYN	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0			170.0	174.0	172.0	170.0	174.0	172.0			
224	0.5	0.64	0.64		0.23			0.0	0.0	0.1	0.0	0.0	0.0			170.2	174.2	172.2	170.1	174.1	172.1			
449	1.0	1.27	1.27		0.46			0.0	0.1	0.5	0.0	0.1	0.0			170.7	174.7	172.7	170.3	174.3	172.3			
673	1.5	1.91	1.91		0.69			0.1	0.2	1.1	0.0	0.3	0.0			171.5	175.5	173.5	170.6	174.6	172.6			
898	2.0	2.55	2.55		0.92			0.1	0.4	2.0	0.0	0.5	0.0			172.6	176.6	174.6	171.0	175.0	173.0			
1122	2.5	3.18	3.18		1.15			0.2	0.6	3.1	0.1	0.8	0.0			174.0	178.0	176.0	171.6	175.6	173.6			
1346	3.0	3.82	3.82		1.37			0.3	0.8	4.5	0.1	1.1	0.0			175.7	179.7	177.7	172.3	176.3	174.3			
1571	3.5	4.46	4.46		1.60			0.4	1.1	6.2	0.1	1.5	0.0			177.8	181.8	179.8	173.1	177.1	175.1			
1795	4.0	5.09	5.09		1.83			0.5	1.4	8.1	0.2	2.0	0.0			180.1	184.1	182.1	174.0	178.0	176.0			
2020	4.5	5.73	5.73		2.06			0.7	1.7	10.2	0.2	2.5	0.1			182.8	186.8	184.8	175.0	179.0	177.0			
2244	5.0	6.37	6.37		2.29			0.8	2.1	12.6	0.2	3.1	0.1			185.8	189.8	187.8	176.1	180.1	178.1			
2468	5.5	7.00	7.00		2.52			1.0	2.5	15.2	0.3	3.8	0.1			189.0	193.0	191.0	177.4	181.4	179.4			
2693	6.0	7.64	7.64		2.75			1.2	3.0	18.1	0.3	4.5	0.1			192.6	196.6	194.6	178.8	182.8	180.8			
2917	6.5	8.27	8.27		2.98			1.4	3.4	21.3	0.4	5.3	0.1			196.5	200.5	198.5	180.2	184.2	182.2			
3142	7.0	8.91	8.91		3.21			1.6	3.9	24.7	0.5	6.2	0.1			200.6	204.6	202.6	181.8	185.8	183.8			
3366	7.5	9.55	9.55		3.44			1.8	4.5	28.3	0.5	7.1	0.1			205.1	209.1	207.1	183.5	187.5	185.5			
3590	8.0	10.18	10.18		3.67			2.1	5.0	32.2	0.6	8.1	0.2			209.9	213.9	211.9	185.3	189.3	187.3			
3815	8.5	10.82	10.82		3.90			2.4	5.6	36.4	0.7	9.1	0.2			215.0	219.0	217.0	187.3	191.3	189.3			
4039	9.0	11.46	11.46		4.12			2.6	6.2	40.8	0.7	10.2	0.2			220.4	224.4	222.4	189.3	193.3	191.3			
4264	9.5	12.09	12.09		4.35			2.9	6.9	45.4	0.8	11.4	0.2			226.1	230.1	228.1	191.4	195.4	193.4			
4488	10.0	12.73	12.73		4.58			3.3	7.6	50.3	0.9	12.6	0.2			232.1	236.1	234.1	193.7	197.7	195.7			
4712	10.5	13.37	13.37		4.81			3.6	8.3	55.5	1.0	13.9	0.3			238.4	242.4	240.4	196.0	200.0	198.0			
4937	11.0	14.00	14.00		5.04			3.9	9.1	60.9	1.1	15.2	0.3			245.0	249.0	247.0	198.5	202.5	200.5			
5161	11.5	14.64	14.64		5.27			4.3	9.8	66.6	1.1	16.6	0.3			251.9	255.9	253.9	201.1	205.1	203.1			
5386	12.0	15.28	15.28		5.50			4.7	10.6	72.5	1.2	18.1	0.3			259.1	263.1	261.1	203.8	207.8	205.8			
5610	12.5	15.91	15.91		5.73			5.1	11.5	78.6	1.3	19.7	0.4			266.6	270.6	268.6	206.6	210.6	208.6			
5834	13.0	16.55	16.55		5.96			5.5	12.3	85.1	1.4	21.3	0.4			274.3	278.3	276.3	209.5	213.5	211.5			
6059	13.5	17.19	17.19		6.19			5.9	13.2	91.7	1.5	22.9	0.4			282.4	286.4	284.4	212.5	216.5	214.5			
6283	14.0	17.82	17.82		6.42			6.4	14.1	98.7	1.6	24.7	0.5			290.8	294.8	292.8	215.7	219.7	217.7			
6508	14.5	18.46	18.46		6.65			6.9	15.1	105.8	1.8	26.5	0.5			299.5	303.5	301.5	218.9	222.9	220.9			
6732	15.0	19.10	19.10		6.87			7.3	16.1	113.2	1.9	28.3	0.5			308.5	312.5	310.5	222.2	226.2	224.2			

ANCHOR QEA, LLC



SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

ALTERNATIVE: ALTERNATIVE ALIGNMENT 1 - 20 CFS DESIGN FLOW - BOOSTER PUMP FOR EXTENDING DELIVERY TO ICICLE IRRIGATION DISTRICT

SUCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTIO	N PIPING		PS PI	PING		DISCHAR	GE PIPING
ELEV	1146.0 feet	PID Canal Bottom	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	1148.5 feet	PID Canal (High Flow)	NOM. DIAM. (in)					14	14		30
LWL	1146.5 feet	PID Canal (Low Flow)	O.D. (in)					14	14		30
			I.D. (in)					14	14		30
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1318.0 feet	IID Canal Bottom	С					110	110		130
HIGH	1320.5 feet	IID Canal (High Flow)	LENGTH (feet)					8	8		2,250
LWL	1318.5 feet	IID Canal (Low Flow)	К					10	10		10

ROPOSED	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2992	6.7	185	199.9
2	5984	13.3	187	404.5
3	8976	20.0	191	617.9
'Assumes	70%	Efficiency		

BY:

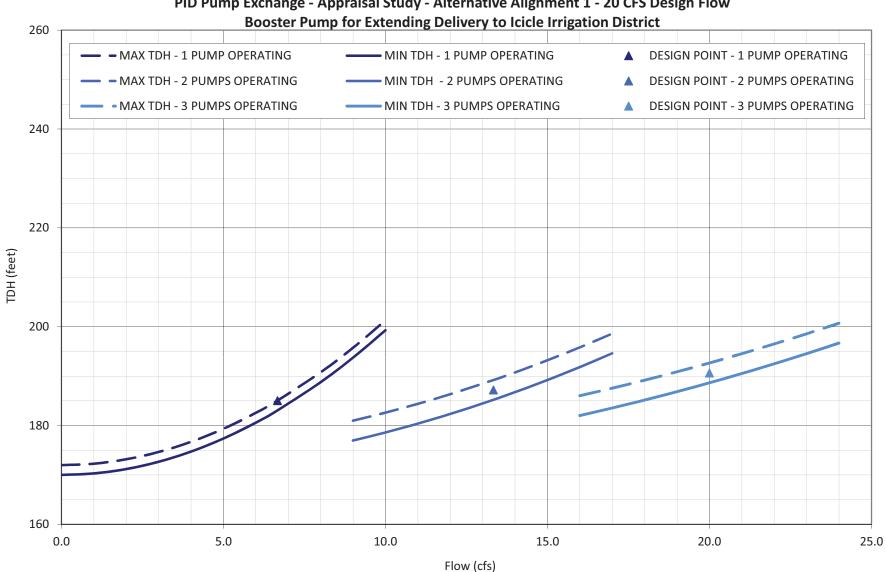
DATE:

David Rice, P.E.

14-Sep-12

TOTAI	FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	S - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	170.0	174.0	172.0	170.0	174.0	172.0	170.0	174.0	172.0
449	1.0	0.94	0.94		0.20			0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	170.3	174.3	172.3	170.1	174.1	172.1	170.1	174.1	172.1
898	2.0	1.87	1.87		0.41			0.0	0.1	1.1	0.0	0.3	0.0	0.1	0.0	171.2	175.2	173.2	170.4	174.4	172.4	170.2	174.2	172.2
1346	3.0	2.81	2.81		0.61			0.1	0.1	2.4	0.0	0.6	0.0	0.3	0.0	172.7	176.7	174.7	170.8	174.8	172.8	170.4	174.4	172.4
1795	4.0	3.74	3.74		0.81			0.1	0.2	4.3	0.1	1.1	0.0	0.5	0.0	174.7	178.7	176.7	171.4	175.4	173.4	170.8	174.8	172.8
2244	5.0	4.68	4.68		1.02			0.2	0.3	6.8	0.1	1.7	0.0	0.8	0.0	177.4	181.4	179.4	172.2	176.2	174.2	171.2	175.2	173.2
2693	6.0	5.61	5.61		1.22			0.2	0.4	9.8	0.2	2.4	0.0	1.1	0.0	180.6	184.6	182.6	173.1	177.1	175.1	171.7	175.7	173.7
2992	6.7	6.24	6.24		1.36			0.3	0.5	12.1	0.2	3.0	0.1	1.3	0.0	183.1	187.1	185.1	173.9	177.9	175.9	172.2	176.2	174.2
3590	8.0	7.48	7.48		1.63			0.4	0.7	17.4	0.3	4.3	0.1	1.9	0.0	188.8	192.8	190.8	175.5	179.5	177.5	173.1	177.1	175.1
4039	9.0	8.42	8.42		1.83			0.5	0.9	22.0	0.3	5.5	0.1	2.4	0.0	193.7	197.7	195.7	177.0	181.0	179.0	173.9	177.9	175.9
4488	10.0	9.35	9.35		2.04			0.6	1.1	27.2	0.4	6.8	0.1	3.0	0.1	199.3	203.3	201.3	178.6	182.6	180.6	174.8	178.8	176.8
4937	11.0	10.29	10.29		2.24			0.8	1.3	32.9	0.5	8.2	0.1	3.7	0.1	205.4	209.4	207.4	180.4	184.4	182.4	175.8	179.8	177.8
5386	12.0	11.22	11.22		2.44			0.9	1.5	39.1	0.6	9.8	0.2	4.3	0.1	212.1	216.1	214.1	182.3	186.3	184.3	176.8	180.8	178.8
5984	13.3	12.47	12.47		2.72			1.1	1.8	48.3	0.7	12.1	0.2	5.4	0.1	222.0	226.0	224.0	185.2	189.2	187.2	178.4	182.4	180.4
6283	14.0	13.09	13.09		2.85			1.3	2.0	53.2	0.8	13.3	0.2	5.9	0.1	227.3	231.3	229.3	186.8	190.8	188.8	179.2	183.2	181.2
6732	15.0	14.03	14.03		3.06			1.4	2.2	61.1	0.9	15.3	0.2	6.8	0.1	235.7	239.7	237.7	189.2	193.2	191.2	180.6	184.6	182.6
7181	16.0	14.97	14.97		3.26			1.6	2.5	69.6	1.0	17.4	0.3	7.7	0.1	244.7	248.7	246.7	191.8	195.8	193.8	182.0	186.0	184.0
7630	17.0	15.90	15.90		3.46			1.9	2.8	78.5	1.1	19.6	0.3	8.7	0.1	254.3	258.3	256.3	194.6	198.6	196.6	183.5	187.5	185.5
8078	18.0	16.84	16.84		3.67			2.1	3.1	88.0	1.2	22.0	0.3	9.8	0.2	264.5	268.5	266.5	197.6	201.6	199.6	185.2	189.2	187.2
8527	19.0	17.77	17.77		3.87			2.3	3.5	98.1	1.4	24.5	0.4	10.9	0.2	275.2	279.2	277.2	200.7	204.7	202.7	186.9	190.9	188.9
8976	20.0	18.71	18.71		4.07			2.6	3.8	108.7	1.5	27.2	0.4	12.1	0.2	286.6	290.6	288.6	204.0	208.0	206.0	188.6	192.6	190.6
9425	21.0	19.64	19.64		4.28			2.8	4.2	119.8	1.6	30.0	0.5	13.3	0.2	298.5	302.5	300.5	207.4	211.4	209.4	190.5	194.5	192.5
9874	22.0	20.58	20.58		4.48			3.1	4.5	131.5	1.8	32.9	0.5	14.6	0.2	310.9	314.9	312.9	211.0	215.0	213.0	192.5	196.5	194.5
10322	23.0	21.51	21.51		4.68			3.4	4.9	143.7	2.0	35.9	0.5	16.0	0.3	324.0	328.0	326.0	214.8	218.8	216.8	194.6	198.6	196.6
10771	24.0	22.45	22.45		4.89			3.7	5.3	156.5	2.1	39.1	0.6	17.4	0.3	337.6	341.6	339.6	218.7	222.7	220.7	196.7	200.7	198.7
11220	25.0	23.38	23.38		5.09			4.0	5.7	169.8	2.3	42.5	0.6	18.9	0.3	351.8	355.8	353.8	222.8	226.8	224.8	198.9	202.9	200.9
11669	26.0	24.32	24.32		5.30			4.4	6.2	183.7	2.4	45.9	0.7	20.4	0.3	366.6	370.6	368.6	227.1	231.1	229.1	201.3	205.3	203.3
12118	27.0	25.25	25.25		5.50			4.7	6.6	198.1	2.6	49.5	0.7	22.0	0.3	382.0	386.0	384.0	231.6	235.6	233.6	203.7	207.7	205.7
12566	28.0	26.19	26.19		5.70			5.1	7.1	213.0	2.8	53.2	0.8	23.7	0.4	397.9	401.9	399.9	236.2	240.2	238.2	206.2	210.2	208.2
13015	29.0	27.12	27.12		5.91			5.4	7.6	228.5	3.0	57.1	0.8	25.4	0.4	414.5	418.5	416.5	240.9	244.9	242.9	208.8	212.8	210.8
13464	30.0	28.06	28.06		6.11			5.8	8.0	244.5	3.2	61.1	0.9	27.2	0.4	431.5	435.5	433.5	245.9	249.9	247.9	211.4	215.4	213.4

Anchor QEA, LLC



PID Pump Exchange - Appraisal Study - Alternative Alignment 1 - 20 CFS Design Flow

SYSTEM CURVE CALCULATION

PROJECT: PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

ALTERNATIVE: ALTERNATIVE ALIGNMENT 1 - 40 CFS DESIGN FLOW - BOOSTER PUMP FOR EXTENDING DELIVERY TO ICICLE IRRIGATION DISTRICT

SUCTION W	ATER SURFACE ELEVAT	IONS:		PIPE	SUCTIO	N PIPING		PS PI	PING		DISCHAR	GE PIPING
ELEV	1146.0 feet	PID Canal Bottom			TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	1148.5 feet	PID Canal (High Flow)		NOM. DIAM. (in)					18	18		36
LWL	1146.5 feet	PID Canal (Low Flow)		O.D. (in)					18	18		36
				I.D. (in)					18	18		36
DISCHARGE	WATER SURFACE ELEV	ATIONS:		MATERIAL					STEEL	STEEL		HDPE
ELEV	1318.0 feet	IID Canal Bottom		С					110	110		130
HIGH	1320.5 feet	IID Canal (High Flow)		LENGTH (feet)					8	8		2,250
LWL	1318.5 feet	IID Canal (Low Flow)		К					10	10		10

PROPOSED	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	5985	13.3	191	413.2
2	11970	26.7	195	841.8
3	17952	40.0	200	1299.6
*Assumes	70%	Efficiency		

BY:

DATE:

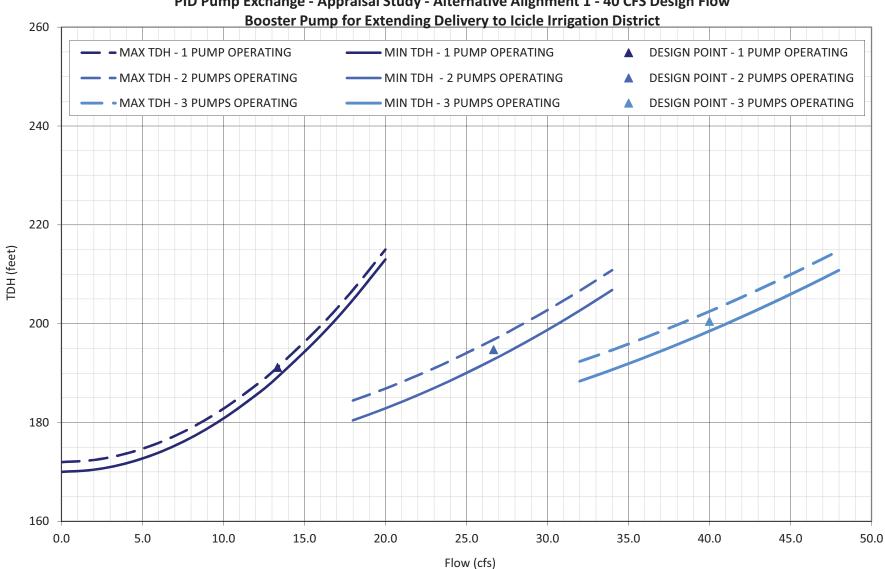
David Rice, P.E.

14-Sep-12

TOTA	L FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00	1	0.00	1		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	170.0	174.0	172.0	170.0	174.0	172.0	170.0	174.0	172.0
898	2.0	1.13	1.13	1	0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	170.4	174.4	172.4	170.1	174.1	172.1	170.1	174.1	172.1
1795	4.0	2.26	2.26	1	0.57			0.0	0.1	1.6	0.0	0.4	0.0	0.2	0.0	171.7	175.7	173.7	170.5	174.5	172.5	170.3	174.3	172.3
2693	6.0	3.39	3.39	1	0.85			0.1	0.2	3.6	0.0	0.9	0.0	0.4	0.0	173.9	177.9	175.9	171.2	175.2	173.2	170.7	174.7	172.7
3590	8.0	4.53	4.53		1.13			0.2	0.3	6.4	0.1	1.6	0.0	0.7	0.0	176.9	180.9	178.9	172.1	176.1	174.1	171.2	175.2	173.2
4488	10.0	5.66	5.66		1.41			0.3	0.4	9.9	0.1	2.5	0.0	1.1	0.0	180.8	184.8	182.8	173.3	177.3	175.3	171.9	175.9	173.9
5386	12.0	6.79	6.79	1	1.70			0.4	0.6	14.3	0.2	3.6	0.0	1.6	0.0	185.5	189.5	187.5	174.7	178.7	176.7	172.7	176.7	174.7
5985	13.3	7.55	7.55		1.89			0.6	0.7	17.7	0.2	4.4	0.1	2.0	0.0	189.2	193.2	191.2	175.8	179.8	177.8	173.3	177.3	175.3
7181	16.0	9.05	9.05		2.26			0.8	1.0	25.5	0.3	6.4	0.1	2.8	0.0	197.6	201.6	199.6	178.3	182.3	180.3	174.7	178.7	176.7
8078	18.0	10.18	10.18	1	2.55			1.0	1.3	32.2	0.4	8.1	0.1	3.6	0.0	204.9	208.9	206.9	180.4	184.4	182.4	175.9	179.9	177.9
8976	20.0	11.32	11.32		2.83			1.2	1.6	39.8	0.4	9.9	0.1	4.4	0.1	213.0	217.0	215.0	182.9	186.9	184.9	177.3	181.3	179.3
9874	22.0	12.45	12.45		3.11			1.5	1.9	48.1	0.5	12.0	0.1	5.3	0.1	222.0	226.0	224.0	185.5	189.5	187.5	178.8	182.8	180.8
10771	24.0	13.58	13.58		3.39			1.8	2.2	57.3	0.6	14.3	0.2	6.4	0.1	231.9	235.9	233.9	188.5	192.5	190.5	180.4	184.4	182.4
11970	26.7	15.09	15.09		3.77			2.2	2.7	70.7	0.8	17.7	0.2	7.9	0.1	246.4	250.4	248.4	192.8	196.8	194.8	182.8	186.8	184.8
12566	28.0	15.84	15.84		3.96			2.4	2.9	77.9	0.8	19.5	0.2	8.7	0.1	254.1	258.1	256.1	195.1	199.1	197.1	184.1	188.1	186.1
13464	30.0	16.97	16.97		4.24			2.8	3.3	89.5	0.9	22.4	0.3	9.9	0.1	266.5	270.5	268.5	198.7	202.7	200.7	186.2	190.2	188.2
14362	32.0	18.11	18.11		4.53			3.2	3.7	101.8	1.1	25.5	0.3	11.3	0.1	279.8	283.8	281.8	202.7	206.7	204.7	188.4	192.4	190.4
15259	34.0	19.24	19.24		4.81			3.6	4.2	114.9	1.2	28.7	0.3	12.8	0.2	293.9	297.9	295.9	206.8	210.8	208.8	190.7	194.7	192.7
16157	36.0	20.37	20.37		5.09			4.0	4.6	128.9	1.3	32.2	0.4	14.3	0.2	308.8	312.8	310.8	211.2	215.2	213.2	193.2	197.2	195.2
17054	38.0	21.50	21.50		5.38			4.5	5.1	143.6	1.5	35.9	0.4	16.0	0.2	324.6	328.6	326.6	215.9	219.9	217.9	195.8	199.8	197.8
17952	40.0	22.63	22.63		5.66			5.0	5.6	159.1	1.6	39.8	0.4	17.7	0.2	341.3	345.3	343.3	220.8	224.8	222.8	198.5	202.5	200.5
18850	42.0	23.76	23.76		5.94			5.5	6.2	175.4	1.7	43.8	0.5	19.5	0.2	358.8	362.8	360.8	226.0	230.0	228.0	201.4	205.4	203.4
19747	44.0	24.90	24.90		6.22			6.0	6.7	192.5	1.9	48.1	0.5	21.4	0.2	377.1	381.1	379.1	231.4	235.4	233.4	204.4	208.4	206.4
20645	46.0	26.03	26.03		6.51			6.6	7.3	210.4	2.1	52.6	0.6	23.4	0.3	396.3	400.3	398.3	237.0	241.0	239.0	207.5	211.5	209.5
21542	48.0	27.16	27.16		6.79			7.2	7.9	229.1	2.2	57.3	0.6	25.5	0.3	416.4	420.4	418.4	242.9	246.9	244.9	210.8	214.8	212.8
22440	50.0	28.29	28.29		7.07			7.8	8.5	248.6	2.4	62.1	0.7	27.6	0.3	437.3	441.3	439.3	249.1	253.1	251.1	214.2	218.2	216.2
23338	52.0	29.42	29.42		7.36			8.4	9.2	268.8	2.6	67.2	0.7	29.9	0.3	459.0	463.0	461.0	255.5	259.5	257.5	217.8	221.8	219.8
24235	54.0	30.55	30.55		7.64			9.1	9.8	289.9	2.8	72.5	0.8	32.2	0.4	481.6	485.6	483.6	262.1	266.1	264.1	221.5	225.5	223.5
25133	56.0	31.69	31.69		7.92			9.7	10.5	311.8	3.0	77.9	0.8	34.6	0.4	505.0	509.0	507.0	269.0	273.0	271.0	225.3	229.3	227.3
26030	58.0	32.82	32.82		8.20			10.5	11.2	334.5	3.2	83.6	0.9	37.2	0.4	529.3	533.3	531.3	276.2	280.2	278.2	229.2	233.2	231.2
26928	60.0	33.95	33.95		8.49			11.2	11.9	357.9	3.4	89.5	0.9	39.8	0.4	554.4	558.4	556.4	283.5	287.5	285.5	233.3	237.3	235.3

PID Pump Exchange - Hydraulic Analysis - Alignment 1 - PID TO IID.xlsx

Anchor QEA, LLC



PID Pump Exchange - Appraisal Study - Alternative Alignment 1 - 40 CFS Design Flow

SYSTEM CURVE CALCULATION

ALTERNATIVE:

PROJECT: PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

ALTERNATIVE ALIGNMENT 5 - 10 CFS DESIGN FLOW - BOOSTER PUMP FOR EXTENDING DELIVERY TO ICICLE IRRIGATION DISTRICT SUCTION WATER SURFACE ELEVATIONS: **IG VS.**))) PE 00 00 ELEV 1158.0 feet PID Canal Bottom PID Canal (High Flow) PID Canal (Low Flow) HWL 1160.5 feet LWL 1158.5 feet

DISCHARGE WATER SURFACE ELEVATIONS:												
ELEV	1319.5 feet	IID Canal Bottom										
HIGH	1322.0 feet	IID Canal (High Flow)										
LWL	1320.0 feet	IID Canal (Low Flow)										

PIPE	SUCTIO	N PIPING		PS PI	IPING		DISCHAR	GE PIPING
PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
NOM. DIAM. (in)					12	12		20
O.D. (in)					12	12		20
I.D. (in)					12	12		20
MATERIAL					STEEL	STEEL		HDPE
С					110	110		130
LENGTH (feet)					8	8		2,700
ĸ		1	1	1	10	10		10

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2244	5.0	178	144.0
2	4488	10.0	187	302.6
*Assumes	70%	Efficiency		

David Rice, P.E.

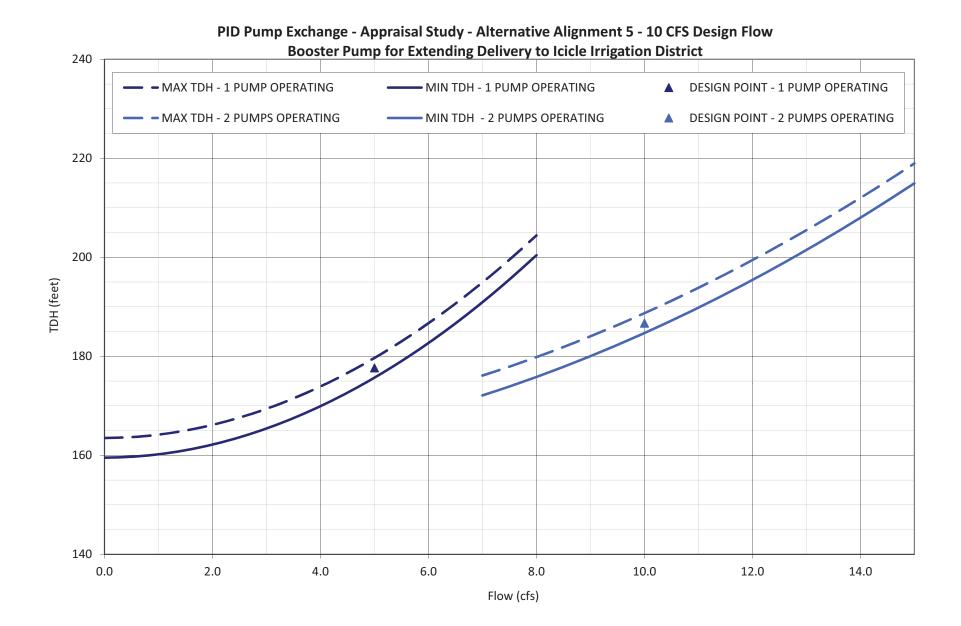
14-Sep-12

BY:

DATE:

TOTAL	FLOW		VELO	CITIES		SUCTION	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	S - 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAI) - 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		12-inch	12-inch	-inch	20-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00	1		0.0	0.0	0.0	0.0	0.0	0.0		1	159.5	163.5	161.5	159.5	163.5	161.5			
224	0.5	0.64	0.64		0.23			0.0	0.0	0.1	0.0	0.0	0.0			159.7	163.7	161.7	159.6	163.6	161.6			
449	1.0	1.27	1.27		0.46			0.0	0.1	0.5	0.0	0.1	0.0			160.2	164.2	162.2	159.8	163.8	161.8			
673	1.5	1.91	1.91		0.69			0.1	0.3	1.1	0.0	0.3	0.0			161.0	165.0	163.0	160.1	164.1	162.1			
898	2.0	2.55	2.55		0.92			0.1	0.5	2.0	0.0	0.5	0.0			162.2	166.2	164.2	160.6	164.6	162.6			
1122	2.5	3.18	3.18		1.15			0.2	0.7	3.1	0.1	0.8	0.0			163.6	167.6	165.6	161.2	165.2	163.2			
1346	3.0	3.82	3.82		1.37			0.3	1.0	4.5	0.1	1.1	0.0			165.4	169.4	167.4	161.9	165.9	163.9			
1571	3.5	4.46	4.46		1.60			0.4	1.3	6.2	0.1	1.5	0.0			167.5	171.5	169.5	162.8	166.8	164.8			
1795	4.0	5.09	5.09		1.83			0.5	1.7	8.1	0.2	2.0	0.0			169.9	173.9	171.9	163.8	167.8	165.8			
2020	4.5	5.73	5.73		2.06			0.7	2.1	10.2	0.2	2.5	0.1			172.6	176.6	174.6	164.8	168.8	166.8			
2244	5.0	6.37	6.37		2.29			0.8	2.5	12.6	0.2	3.1	0.1			175.7	179.7	177.7	166.1	170.1	168.1			
2468	5.5	7.00	7.00		2.52			1.0	3.0	15.2	0.3	3.8	0.1			179.0	183.0	181.0	167.4	171.4	169.4			
2693	6.0	7.64	7.64		2.75			1.2	3.5	18.1	0.3	4.5	0.1			182.7	186.7	184.7	168.8	172.8	170.8			
2917	6.5	8.27	8.27		2.98			1.4	4.1	21.3	0.4	5.3	0.1			186.6	190.6	188.6	170.4	174.4	172.4			
3142	7.0	8.91	8.91		3.21			1.6	4.7	24.7	0.5	6.2	0.1			190.9	194.9	192.9	172.1	176.1	174.1			
3366	7.5	9.55	9.55		3.44			1.8	5.4	28.3	0.5	7.1	0.1			195.5	199.5	197.5	173.9	177.9	175.9			
3590	8.0	10.18	10.18		3.67			2.1	6.0	32.2	0.6	8.1	0.2			200.4	204.4	202.4	175.8	179.8	177.8			
3815	8.5	10.82	10.82		3.90			2.4	6.7	36.4	0.7	9.1	0.2			205.6	209.6	207.6	177.9	181.9	179.9			
4039	9.0	11.46	11.46		4.12			2.6	7.5	40.8	0.7	10.2	0.2			211.1	215.1	213.1	180.0	184.0	182.0			
4264	9.5	12.09	12.09		4.35			2.9	8.3	45.4	0.8	11.4	0.2			217.0	221.0	219.0	182.3	186.3	184.3			
4488	10.0	12.73	12.73		4.58			3.3	9.1	50.3	0.9	12.6	0.2			223.1	227.1	225.1	184.7	188.7	186.7			
4712	10.5	13.37	13.37		4.81			3.6	10.0	55.5	1.0	13.9	0.3			229.5	233.5	231.5	187.2	191.2	189.2			
4937	11.0	14.00	14.00		5.04			3.9	10.9	60.9	1.1	15.2	0.3			236.3	240.3	238.3	189.8	193.8	191.8			
5161	11.5	14.64	14.64		5.27			4.3	11.8	66.6	1.1	16.6	0.3			243.3	247.3	245.3	192.6	196.6	194.6			
5386	12.0	15.28	15.28		5.50			4.7	12.8	72.5	1.2	18.1	0.3			250.7	254.7	252.7	195.4	199.4	197.4			
5610	12.5	15.91	15.91		5.73			5.1	13.8	78.6	1.3	19.7	0.4			258.3	262.3	260.3	198.4	202.4	200.4			
5834	13.0	16.55	16.55		5.96			5.5	14.8	85.1	1.4	21.3	0.4			266.3	270.3	268.3	201.5	205.5	203.5			
6059	13.5	17.19	17.19		6.19			5.9	15.9	91.7	1.5	22.9	0.4			274.6	278.6	276.6	204.7	208.7	206.7			
6283	14.0	17.82	17.82		6.42			6.4	17.0	98.7	1.6	24.7	0.5			283.2	287.2	285.2	208.0	212.0	210.0			
6508	14.5	18.46	18.46		6.65			6.9	18.1	105.8	1.8	26.5	0.5			292.1	296.1	294.1	211.4	215.4	213.4			
6732	15.0	19.10	19.10		6.87			7.3	19.3	113.2	1.9	28.3	0.5			301.2	305.2	303.2	215.0	219.0	217.0			

ANCHOR QEA, LLC



Anchor QEA, LLC

SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

ALTERNATIVE: ALTERNATIVE ALIGNMENT 5 - 20 CFS DESIGN FLOW - BOOSTER PUMP FOR EXTENDING DELIVERY TO ICICLE IRRIGATION DISTRICT

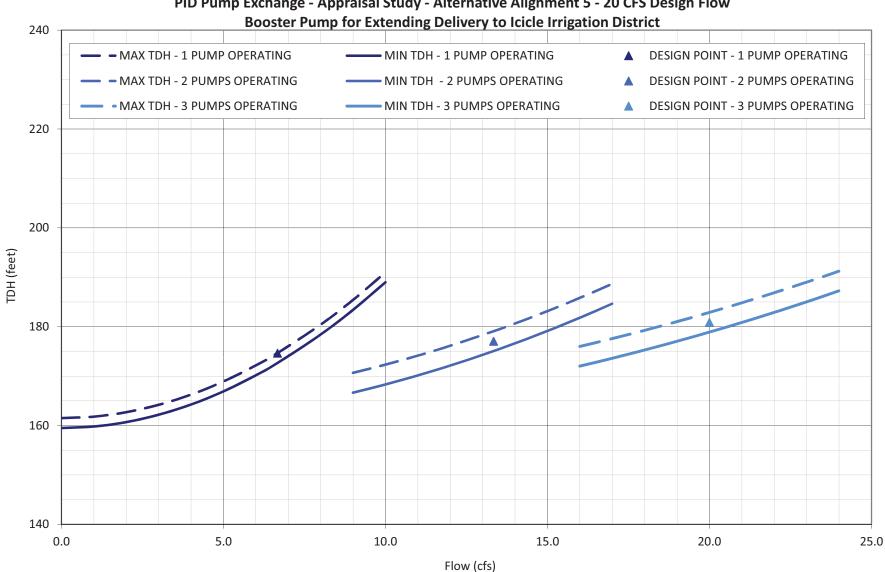
SUCTION W	ATER SURFACE ELEVAT	ONS:	PIPE	SUCTIO	N PIPING		PS PI	PING		DISCHAR	GE PIPING
ELEV	1158.0 feet	PID Canal Bottom	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.
HWL	1160.5 feet	PID Canal (High Flow)	NOM. DIAM. (in)					14	14		30
LWL	1158.5 feet	PID Canal (Low Flow)	O.D. (in)					14	14		30
			I.D. (in)					14	14		30
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE
ELEV	1319.5 feet	IID Canal Bottom	С					110	110		130
HIGH	1322.0 feet	IID Canal (High Flow)	LENGTH (feet)					8	8		2,690
LWL	1320.0 feet	IID Canal (Low Flow)	К					10	10		10

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	2992	6.7	175	188.7
2	5985	13.3	177	382.7
3	8976	20.0	181	586.3
*Assumes	70%	Efficiency		

TOTAL	L FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		14-inch	14-inch	-inch	30-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	159.5	163.5	161.5	159.5	163.5	161.5	159.5	163.5	161.5
449	1.0	0.94	0.94		0.20			0.0	0.0	0.3	0.0	0.1	0.0	0.0	0.0	159.8	163.8	161.8	159.6	163.6	161.6	159.6	163.6	161.6
898	2.0	1.87	1.87		0.41			0.0	0.1	1.1	0.0	0.3	0.0	0.1	0.0	160.7	164.7	162.7	159.9	163.9	161.9	159.7	163.7	161.7
1346	3.0	2.81	2.81		0.61			0.1	0.1	2.4	0.0	0.6	0.0	0.3	0.0	162.2	166.2	164.2	160.3	164.3	162.3	160.0	164.0	162.0
1795	4.0	3.74	3.74		0.81			0.1	0.2	4.3	0.1	1.1	0.0	0.5	0.0	164.3	168.3	166.3	160.9	164.9	162.9	160.3	164.3	162.3
2244	5.0	4.68	4.68		1.02			0.2	0.3	6.8	0.1	1.7	0.0	0.8	0.0	166.9	170.9	168.9	161.7	165.7	163.7	160.8	164.8	162.8
2693	6.0	5.61	5.61		1.22			0.2	0.5	9.8	0.2	2.4	0.0	1.1	0.0	170.2	174.2	172.2	162.7	166.7	164.7	161.3	165.3	163.3
2992	6.7	6.24	6.24		1.36			0.3	0.6	12.1	0.2	3.0	0.1	1.3	0.0	172.7	176.7	174.7	163.5	167.5	165.5	161.7	165.7	163.7
3590	8.0	7.48	7.48		1.63			0.4	0.8	17.4	0.3	4.3	0.1	1.9	0.0	178.4	182.4	180.4	165.2	169.2	167.2	162.7	166.7	164.7
4039	9.0	8.42	8.42		1.83			0.5	1.0	22.0	0.3	5.5	0.1	2.4	0.0	183.4	187.4	185.4	166.7	170.7	168.7	163.5	167.5	165.5
4488	10.0	9.35	9.35		2.04			0.6	1.3	27.2	0.4	6.8	0.1	3.0	0.1	189.0	193.0	191.0	168.3	172.3	170.3	164.5	168.5	166.5
4937	11.0	10.29	10.29		2.24			0.8	1.5	32.9	0.5	8.2	0.1	3.7	0.1	195.2	199.2	197.2	170.1	174.1	172.1	165.5	169.5	167.5
5386	12.0	11.22	11.22		2.44			0.9	1.8	39.1	0.6	9.8	0.2	4.3	0.1	201.9	205.9	203.9	172.1	176.1	174.1	166.6	170.6	168.6
5985	13.3	12.47	12.47		2.72			1.1	2.1	48.3	0.7	12.1	0.2	5.4	0.1	211.8	215.8	213.8	175.1	179.1	177.1	168.3	172.3	170.3
6283	14.0	13.09	13.09		2.85			1.3	2.3	53.2	0.8	13.3	0.2	5.9	0.1	217.1	221.1	219.1	176.6	180.6	178.6	169.1	173.1	171.1
6732	15.0	14.03	14.03		3.06			1.4	2.7	61.1	0.9	15.3	0.2	6.8	0.1	225.6	229.6	227.6	179.1	183.1	181.1	170.5	174.5	172.5
7181	16.0	14.97	14.97		3.26			1.6	3.0	69.6	1.0	17.4	0.3	7.7	0.1	234.7	238.7	236.7	181.8	185.8	183.8	172.0	176.0	174.0
7630	17.0	15.90	15.90		3.46			1.9	3.4	78.5	1.1	19.6	0.3	8.7	0.1	244.4	248.4	246.4	184.7	188.7	186.7	173.6	177.6	175.6
8078	18.0	16.84	16.84		3.67			2.1	3.7	88.0	1.2	22.0	0.3	9.8	0.2	254.6	258.6	256.6	187.7	191.7	189.7	175.3	179.3	177.3
8527	19.0	17.77	17.77		3.87			2.3	4.1	98.1	1.4	24.5	0.4	10.9	0.2	265.4	269.4	267.4	190.9	194.9	192.9	177.0	181.0	179.0
8976	20.0	18.71	18.71		4.07			2.6	4.5	108.7	1.5	27.2	0.4	12.1	0.2	276.8	280.8	278.8	194.2	198.2	196.2	178.9	182.9	180.9
9425	21.0	19.64	19.64		4.28			2.8	5.0	119.8	1.6	30.0	0.5	13.3	0.2	288.8	292.8	290.8	197.7	201.7	199.7	180.8	184.8	182.8
9874	22.0	20.58	20.58		4.48			3.1	5.4	131.5	1.8	32.9	0.5	14.6	0.2	301.3	305.3	303.3	201.4	205.4	203.4	182.9	186.9	184.9
10322	23.0	21.51	21.51		4.68			3.4	5.9	143.7	2.0	35.9	0.5	16.0	0.3	314.5	318.5	316.5	205.3	209.3	207.3	185.0	189.0	187.0
10771	24.0	22.45	22.45		4.89			3.7	6.4	156.5	2.1	39.1	0.6	17.4	0.3	328.2	332.2	330.2	209.3	213.3	211.3	187.2	191.2	189.2
11220	25.0	23.38	23.38		5.09			4.0	6.9	169.8	2.3	42.5	0.6	18.9	0.3	342.5	346.5	344.5	213.5	217.5	215.5	189.6	193.6	191.6
11669	26.0	24.32	24.32		5.30			4.4	7.4	183.7	2.4	45.9	0.7	20.4	0.3	357.3	361.3	359.3	217.8	221.8	219.8	192.0	196.0	194.0
12118	27.0	25.25	25.25		5.50			4.7	7.9	198.1	2.6	49.5	0.7	22.0	0.3	372.8	376.8	374.8	222.4	226.4	224.4	194.5	198.5	196.5
12566	28.0	26.19	26.19		5.70			5.1	8.5	213.0	2.8	53.2	0.8	23.7	0.4	388.8	392.8	390.8	227.0	231.0	229.0	197.0	201.0	199.0
13015	29.0	27.12	27.12		5.91			5.4	9.0	228.5	3.0	57.1	0.8	25.4	0.4	405.4	409.4	407.4	231.9	235.9	233.9	199.7	203.7	201.7
13464	30.0	28.06	28.06		6.11			5.8	9.6	244.5	3.2	61.1	0.9	27.2	0.4	422.6	426.6	424.6	236.9	240.9	238.9	202.5	206.5	204.5

BY: David Rice, P.E.

DATE: 14-Sep-12



SYSTEM CURVE CALCULATION PROJECT: PESHAS PESHASTIN IRRIGATION DISTRICT PUMP EXCHANGE - APPRAISAL STUDY

ALTERNATIVE: ALTERNATIVE ALIGNMENT 5 - 40 CFS DESIGN FLOW - BOOSTER PUMP FOR EXTENDING DELIVERY TO ICICLE IRRIGATION DISTRICT

SUCTION W	ATER SURFACE ELEVAT	IONS:	PIPE	SUCTION PIPING						DISCHARGE PIPING		
ELEV	1158.0 feet	PID Canal Bottom	PROPERTIES	TRANS.	TRANS.	HEADER	TO PUMP	FR. PUMP	HEADER	TRANS.	TRANS.	
HWL	1160.5 feet	PID Canal (High Flow)	NOM. DIAM. (in)					18	18		36	
LWL	1158.5 feet	PID Canal (Low Flow)	O.D. (in)					18	18		36	
			I.D. (in)					18	18		36	
DISCHARGE	WATER SURFACE ELEV	ATIONS:	MATERIAL					STEEL	STEEL		HDPE	
ELEV	1319.5 feet	IID Canal Bottom	С					110	110		130	
HIGH	1322.0 feet	IID Canal (High Flow)	LENGTH (feet)					8	8		2,690	
LWL	1320.0 feet	IID Canal (Low Flow)	K					10	10		10	

PROPOSED D	DESIGN POIN	TS:		
PUMPS	FLOW	FLOW	TDH	POWER
ON	(GPM)	(CFS)	(FT)	(HP)*
1	5985	13.3	181	390.8
2	11970	26.7	185	798.7
3	17952	40.0	191	1238.7
*Assumes	70%	Efficiency		

BY:

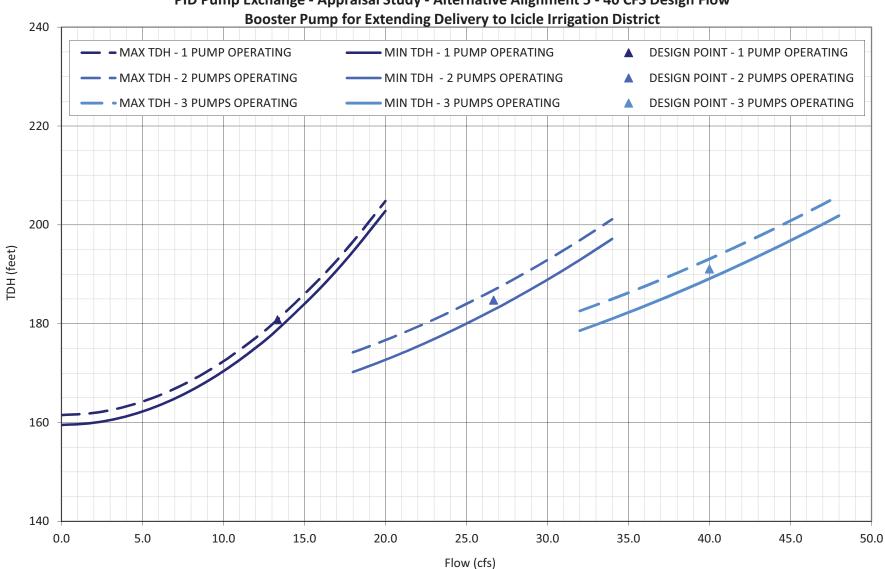
DATE:

David Rice, P.E.

14-Sep-12

TOTAL	. FLOW		VELO	CITIES		SUCTIO	N LOSSES	DISCHAR	GE LOSSES	PS LOSSES	- 1 PUMP	PS LOSSES	- 2 PUMPS	PS LOSSES	- 3 PUMPS	TOTAL DY	NAMIC HEAD	- 1 PUMP	TOTAL DY	NAMIC HEAD	- 2 PUMPS	TOTAL DY	NAMIC HEAD	- 3 PUMPS
		18-inch	18-inch	-inch	36-inch	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	Minor	Friction	MIN	MAX	DESIGN	MIN	MAX	DESIGN	MIN	MAX	DESIGN
(gpm)	(cfs)	(fps)	(fps)	(fps)	(fps)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
0	0.0	0.00	0.00		0.00	1		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	159.5	163.5	161.5	159.5	163.5	161.5	159.5	163.5	161.5
898	2.0	1.13	1.13		0.28			0.0	0.0	0.4	0.0	0.1	0.0	0.0	0.0	159.9	163.9	161.9	159.6	163.6	161.6	159.6	163.6	161.6
1795	4.0	2.26	2.26		0.57			0.0	0.1	1.6	0.0	0.4	0.0	0.2	0.0	161.3	165.3	163.3	160.0	164.0	162.0	159.8	163.8	161.8
2693	6.0	3.39	3.39		0.85			0.1	0.2	3.6	0.0	0.9	0.0	0.4	0.0	163.4	167.4	165.4	160.7	164.7	162.7	160.2	164.2	162.2
3590	8.0	4.53	4.53		1.13			0.2	0.3	6.4	0.1	1.6	0.0	0.7	0.0	166.5	170.5	168.5	161.7	165.7	163.7	160.8	164.8	162.8
4488	10.0	5.66	5.66		1.41			0.3	0.5	9.9	0.1	2.5	0.0	1.1	0.0	170.4	174.4	172.4	162.8	166.8	164.8	161.4	165.4	163.4
5386	12.0	6.79	6.79		1.70			0.4	0.7	14.3	0.2	3.6	0.0	1.6	0.0	175.2	179.2	177.2	164.3	168.3	166.3	162.3	166.3	164.3
5985	13.3	7.55	7.55		1.89			0.6	0.9	17.7	0.2	4.4	0.1	2.0	0.0	178.8	182.8	180.8	165.4	169.4	167.4	162.9	166.9	164.9
7181	16.0	9.05	9.05		2.26			0.8	1.2	25.5	0.3	6.4	0.1	2.8	0.0	187.3	191.3	189.3	168.0	172.0	170.0	164.4	168.4	166.4
8078	18.0	10.18	10.18		2.55			1.0	1.5	32.2	0.4	8.1	0.1	3.6	0.0	194.6	198.6	196.6	170.2	174.2	172.2	165.7	169.7	167.7
8976	20.0	11.32	11.32		2.83			1.2	1.9	39.8	0.4	9.9	0.1	4.4	0.1	202.8	206.8	204.8	172.7	176.7	174.7	167.1	171.1	169.1
9874	22.0	12.45	12.45		3.11			1.5	2.2	48.1	0.5	12.0	0.1	5.3	0.1	211.9	215.9	213.9	175.4	179.4	177.4	168.6	172.6	170.6
10771	24.0	13.58	13.58		3.39			1.8	2.6	57.3	0.6	14.3	0.2	6.4	0.1	221.8	225.8	223.8	178.4	182.4	180.4	170.4	174.4	172.4
11970	26.7	15.09	15.09		3.77			2.2	3.2	70.7	0.8	17.7	0.2	7.9	0.1	236.4	240.4	238.4	182.8	186.8	184.8	172.9	176.9	174.9
12566	28.0	15.84	15.84		3.96			2.4	3.5	77.9	0.8	19.5	0.2	8.7	0.1	244.2	248.2	246.2	185.1	189.1	187.1	174.2	178.2	176.2
13464	30.0	16.97	16.97		4.24			2.8	4.0	89.5	0.9	22.4	0.3	9.9	0.1	256.7	260.7	258.7	188.9	192.9	190.9	176.3	180.3	178.3
14362	32.0	18.11	18.11		4.53			3.2	4.5	101.8	1.1	25.5	0.3	11.3	0.1	270.0	274.0	272.0	192.9	196.9	194.9	178.6	182.6	180.6
15259	34.0	19.24	19.24		4.81			3.6	5.0	114.9	1.2	28.7	0.3	12.8	0.2	284.2	288.2	286.2	197.1	201.1	199.1	181.0	185.0	183.0
16157	36.0	20.37	20.37		5.09			4.0	5.5	128.9	1.3	32.2	0.4	14.3	0.2	299.2	303.2	301.2	201.6	205.6	203.6	183.6	187.6	185.6
17054	38.0	21.50	21.50		5.38			4.5	6.1	143.6	1.5	35.9	0.4	16.0	0.2	315.1	319.1	317.1	206.4	210.4	208.4	186.3	190.3	188.3
17952	40.0	22.63	22.63		5.66			5.0	6.7	159.1	1.6	39.8	0.4	17.7	0.2	331.9	335.9	333.9	211.4	215.4	213.4	189.1	193.1	191.1
18850	42.0	23.76	23.76		5.94			5.5	7.4	175.4	1.7	43.8	0.5	19.5	0.2	349.5	353.5	351.5	216.7	220.7	218.7	192.1	196.1	194.1
19747	44.0	24.90	24.90		6.22			6.0	8.0	192.5	1.9	48.1	0.5	21.4	0.2	367.9	371.9	369.9	222.2	226.2	224.2	195.2	199.2	197.2
20645	46.0	26.03	26.03		6.51			6.6	8.7	210.4	2.1	52.6	0.6	23.4	0.3	387.2	391.2	389.2	228.0	232.0	230.0	198.4	202.4	200.4
21542	48.0	27.16	27.16		6.79			7.2	9.4	229.1	2.2	57.3	0.6	25.5	0.3	407.4	411.4	409.4	234.0	238.0	236.0	201.8	205.8	203.8
22440	50.0	28.29	28.29		7.07			7.8	10.2	248.6	2.4	62.1	0.7	27.6	0.3	428.4	432.4	430.4	240.3	244.3	242.3	205.4	209.4	207.4
23338	52.0	29.42	29.42		7.36			8.4	10.9	268.8	2.6	67.2	0.7	29.9	0.3	450.3	454.3	452.3	246.8	250.8	248.8	209.1	213.1	211.1
24235	54.0	30.55	30.55		7.64			9.1	11.7	289.9	2.8	72.5	0.8	32.2	0.4	473.0	477.0	475.0	253.6	257.6	255.6	212.9	216.9	214.9
25133	56.0	31.69	31.69		7.92			9.7	12.6	311.8	3.0	77.9	0.8	34.6	0.4	496.6	500.6	498.6	260.6	264.6	262.6	216.8	220.8	218.8
26030	58.0	32.82	32.82		8.20			10.5	13.4	334.5	3.2	83.6	0.9	37.2	0.4	521.0	525.0	523.0	267.8	271.8	269.8	220.9	224.9	222.9
26928	60.0	33.95	33.95		8.49			11.2	14.3	357.9	3.4	89.5	0.9	39.8	0.4	546.3	550.3	548.3	275.4	279.4	277.4	225.2	229.2	227.2

Anchor QEA, LLC



PID Pump Exchange - Appraisal Study - Alternative Alignment 5 - 40 CFS Design Flow

Peshastin Irrigation District Pump Exchange Appraisal Study D. Rice Opinion of Probable Costs - Pumping From PID Canal to IID Division 3A Canal 20-Sep-12 Design Flow Rate = 10 CFS 20-Sep-12

			ALTER	NATIVE 1	ALTER	NATIVE 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST
Micellaneous Site Work						
Diversion and care of water	LS	VARIES	1	\$5,000	1	\$5,000
Temporary & permanent access	LS	VARIES	1	\$10,000	1	\$8,000
Erosion and sediment control	LS	VARIES	1	\$10,000	1	\$11,000
Clearing and grubbing	AC	\$3,500	0.3	\$1,085	0.2	\$651
Subtotal - Miscellaneous Site Work				\$26,085		\$24,651
Earthwork						
Excavation and stockpile, soil	CY	\$6.00	1,051	\$6,309	1,253	\$7,517
Excavation and stockpile, rock	CY	\$15.00	701	\$10,514	835	\$12,528
Backfill (imported material)	CY	\$30.00	682	\$20,447	816	\$24,494
Backfill (native material)	CY	\$8.00	810	\$6,481	969	\$7,748
Waste/Disposal of excess material	CY	\$5.00	942	\$4,711	1,120	\$5,598
Subtotal - Earthwork				\$48,462		\$57,886
Pump Station and Intake Facility						
Reinforced Concrete Structure	LS	\$40,000	1	\$40,000	1	\$40,000
Vertical Turbine or Centrifugal Pumps - 150-160 HP, VFD	EA	\$120,000	2	\$240,000	2	\$240,000
3-Phase Power Extension	LS	VARIES	1	\$100,000	1	\$100,000
Electrical and Controls	LS	\$80,000	1	\$80,000	1	\$80,000
Piping, Valves, Meter and Appurtenances	LS	\$40,000	1	\$40,000	1	\$40,000
Subtotal - Pump Station and Intake Facility				\$500,000		\$500,000
Delivery Pipeline						
20" CL 52 D.I. Pipe	LF	\$115.00	2,250	\$258,750	2,700	\$310,500
Fittings and Appurtenances	LF	\$15.00	2,250	\$33,750	2,700	\$40,500
Deadman Road Pavement Repair	LS	VARIES	1	\$25,000	1	\$30,000
Subtotal - Delivery Pipeline				\$317,500		\$381,000
Outlet Structure						
Reinforced Concrete Outlet Structure	LS	\$5,000.00	1	\$5,000	1	\$5,000
Subtotal - Outlet Structure				\$5,000		\$5,000
Construction Subtotal				\$897,000		\$969,000
Mobilization / Demobilization	10.0%			\$89,700		\$96,900
Contingency	30.0%			\$296,010		\$319,770
Engineering, Permitting and Administration	20.0%			\$197,340		\$213,180
Sales Tax	8.0%			\$118,404		\$127,908
Allowance for Land Acquisition	LS	\$50,000.00	1	\$50,000	1	\$50,000
Total Project Cost				\$1,648,000		\$1,777,000

Peshastin Irrigation District Pump Exchange Appraisal Study	D. Rice
Opinion of Probable Operating Costs - Pumping From PID Canal to IID Division 3A Canal	20-Sep-12
Design Flow Rate = 10 CFS	

			ALIGN	IMENT 1	ALIGN	IMENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST
Annual Operations and Maintenance Cost ¹				\$11,920		\$16,120
Pumping Power Costs ²						
Monthly Basic Charge (3-Phase Power)	/EA/MO	\$14.50	1	\$14.50	1	\$14.50
Monthly Demand Charge	/HP/MO	\$3.52	320	\$1,126.40	305	\$1,073.60
Energy Charge						
2-Week Annual Pumping Duration	kWH	\$0.0165	80,210	\$1,323.46	76,450	\$1,261.43
4-Week Annual Pumping Duration	kWH	\$0.0165	160,420	\$2,646.93	152,900	\$2,522.85
6-Week Annual Pumping Duration	kWH	\$0.0165	240,630	\$3,970.39	229,350	\$3,784.28
8-Week Annual Pumping Duration	kWH	\$0.0165	320,840	\$5,293.85	305,800	\$5,045.71
Total Annual Pumping Costs						
2-Week Annual Pumping Duration				\$7,028		\$6,702
4-Week Annual Pumping Duration				\$8,351		\$7,963
6-Week Annual Pumping Duration				\$9,675		\$9,225
8-Week Annual Pumping Duration				\$10,998		\$10,486
Total Annual Operating Costs ³						
2-Week Annual Pumping Duration				\$18,900		\$22,800
4-Week Annual Pumping Duration				\$20,300		\$24,100
6-Week Annual Pumping Duration				\$21,600		\$25,300
8-Week Annual Pumping Duration				\$22,900		\$26,600

Notes:

1) Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance, 2) Pumping power costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service), and are applied May through June, in 2012 dollars. 3) Rounded to nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal StudyD. RiceOpinion of Probable Costs - Pumping From PID Canal to IID Division 3A Canal20-Sep-12Design Flow Rate = 20 CFSCFS

			ALIGN	IMENT 1	ALIGN	IMENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST
Micellaneous Site Work						
Diversion and care of water	LS	VARIES	1	\$5,000	1	\$5,000
Temporary & permanent access	LS	VARIES	1	\$10,000	1	\$8,000
Erosion and sediment control	LS	VARIES	1	\$10,000	1	\$11,000
Clearing and grubbing	AC	\$3,500	0.3	\$1,085	0.2	\$651
Subtotal - Miscellaneous Site Work				\$26,085		\$24,651
Earthwork						
Excavation and stockpile, soil	CY	\$6.00	1,501	\$9,007	1,789	\$10,733
Excavation and stockpile, rock	CY	\$15.00	1,001	\$15,011	1,193	\$17,889
Backfill (imported material)	CY	\$30.00	970	\$29,107	1,162	\$34,870
Backfill (native material)	CY	\$8.00	998	\$7,985	1,193	\$9,541
Waste/Disposal of excess material	CY	\$5.00	1,504	\$7,519	1,789	\$8,944
Subtotal - Earthwork				\$68,628		\$81,977
Pump Station and Intake Facility						
Reinforced Concrete Structure	LS	\$60,000	1	\$60,000	1	\$60,000
Vertical Turbine or Centrifugal Pumps - 200-210 HP, VFD	EA	\$160,000	3	\$480,000	3	\$480,000
3-Phase Power Extension	LS	VARIES	1	\$100,000	1	\$100,000
Electrical and Controls	LS	\$130,000	1	\$130,000	1	\$130,000
Piping, Valves, Meter and Appurtenances	LS	\$60,000	1	\$60,000	1	\$60,000
Subtotal - Pump Station and Intake Facility				\$830,000		\$830,000
Delivery Pipeline						
30" CL 52 D.I. Pipe	LF	\$160.00	2,250	\$360,000	2,700	\$432,000
Fittings and Appurtenances	LF	\$17.00	2,250	\$38,250	2,700	\$45,900
Deadman Road Pavement Repair	LS	VARIES	1	\$25,000	1	\$30,000
Subtotal - Delivery Pipeline				\$423,250		\$507,900
Outlet Structure						
Reinforced Concrete Outlet Structure	LS	\$6,000.00	1	\$6,000	1	\$6,000
Subtotal - Outlet Structure				\$6,000		\$6,000
Construction Subtotal				\$1,354,000		\$1,451,000
Mobilization / Demobilization	10.0%			\$135,400		\$145,100
Contingency	30.0%			\$446,820		\$478,830
Engineering, Permitting and Administration	20.0%			\$297,880		\$319,220
Sales Tax	8.0%			\$178,728		\$191,532
Allowance for Land Acquisition	LS	\$50,000.00	1	\$50,000	1	\$50,000
Total Project Cost				\$2,463,000		\$2,636,000

Peshastin Irrigation District Pump Exchange Appraisal Study	D. Rice
Opinion of Probable Operating Costs - Pumping From PID Canal to IID Division 3A Canal	20-Sep-12
Design Flow Rate = 20 CFS	

			ALIGN	IMENT 1	ALIGN	MENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST
Annual Operations and Maintenance Cost ¹				\$14,220		\$19,320
Pumping Power Costs ²						
Basic Charge (3-Phase Power)	/EA/MO	\$14.50	1	\$14.50	1	\$14.50
Monthly Demand Charge	/HP/MO	\$3.52	620	\$2,182.40	590	\$2,076.80
Energy Charge						
2-Week Annual Pumping Duration	kWH	\$0.0165	155,407	\$2,564.21	147,887	\$2,440.14
4-Week Annual Pumping Duration	kWH	\$0.0165	310,813	\$5,128.42	295,774	\$4,880.27
6-Week Annual Pumping Duration	kWH	\$0.0165	466,220	\$7,692.63	443,661	\$7,320.41
8-Week Annual Pumping Duration	kWH	\$0.0165	621,627	\$10,256.84	591,548	\$9,760.54
Total Annual Pumping Costs						
2-Week Annual Pumping Duration				\$13,549		\$12,897
4-Week Annual Pumping Duration				\$16,113		\$15,337
6-Week Annual Pumping Duration				\$18,677		\$17,777
8-Week Annual Pumping Duration				\$21,241		\$20,217
Total Annual Operating Costs ³						
2-Week Annual Pumping Duration				\$27,800		\$32,200
4-Week Annual Pumping Duration				\$30,300		\$34,700
6-Week Annual Pumping Duration				\$32,900		\$37,100
8-Week Annual Pumping Duration				\$35,500		\$39,500

Notes:

Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance,
 Pumping power costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service), and are applied May through June, in 2012 dollars.
 Rounded to nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study Opinion of Probable Costs - Pumping From PID Canal to IID Division 3A Canal Design Flow Rate = 40 CFS

			ALIGN	IMENT 1	ALIGNMENT 5			
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST		
Micellaneous Site Work								
Diversion and care of water	LS	VARIES	1	\$5,000	1	\$5,000		
Temporary & permanent access	LS	VARIES	1	\$10,000	1	\$8,000		
Erosion and sediment control	LS	VARIES	1	\$10,000	1	\$11,000		
Clearing and grubbing	AC	\$3,500	0.3	\$1,085	0.2	\$651		
Subtotal - Miscellaneous Site Work				\$26,085		\$24,651		
Earthwork								
Excavation and stockpile, soil	CY	\$6.00	1,817	\$10,902	2,164	\$12,987		
Excavation and stockpile, rock	CY	\$15.00	1,211	\$18,170	1,443	\$21,645		
Backfill (imported material)	CY	\$30.00	1,158	\$34,736	1,387	\$41,617		
Backfill (native material)	CY	\$8.00	1,115	\$8,919	1,331	\$10,650		
Waste/Disposal of excess material	CY	\$5.00	1,913	\$9,567	2,276	\$11,381		
Subtotal - Earthwork				\$82,296		\$98,279		
Pump Station and Intake Facility								
Reinforced Concrete Structure	LS	\$80,000	1	\$80,000	1	\$80,000		
Vertical Turbine or Centrifugal Pumps - 420-440 HP, VFD	EA	\$240,000	3	\$720,000	3	\$720,000		
3-Phase Power Extension	LS	VARIES	1	\$100,000	1	\$100,000		
Electrical and Controls	LS	\$190,000	1	\$190,000	1	\$190,000		
Piping, Valves, Meter and Appurtenances	LS	\$110,000	1	\$110,000	1	\$110,000		
Subtotal - Pump Station and Intake Facility				\$1,200,000		\$1,200,000		
Delivery Pipeline								
36" CL 52 D.I. Pipe	LF	\$210.00	2,250	\$472,500	2,700	\$567,000		
Fittings and Appurtenances	LF	\$23.00	2,250	\$51,750	2,700	\$62,100		
U.S. Highway 2 Crossing	LS	VARIES	0	\$0	1	\$420,000		
Subtotal - Delivery Pipeline				\$524,250		\$1,049,100		
Outlet Structure								
Reinforced Concrete Outlet Structure	LS	\$10,000.00	1	\$10,000	1	\$10,000		
Subtotal - Outlet Structure				\$10,000		\$10,000		
Construction Subtotal				\$1,843,000		\$2,382,000		
Mobilization / Demobilization	10.0%			\$184,300		\$238,200		
Contingency	30.0%			\$608,190		\$786,060		
Engineering, Permitting and Administration	20.0%			\$405,460		\$524,040		
Sales Tax	8.0%	¢50.000.00		\$243,276		\$314,424		
Allowance for Land Acquisition	LS	\$50,000.00	1	\$50,000	1	\$50,000		
Total Project Cost				\$3,334,000		\$4,295,000		

D. Rice

20-Sep-12

Peshastin Irrigation District Pump Exchange Appraisal Study	D. Rice
Opinion of Probable Operating Costs - Pumping From PID Canal to IID Division 3A Canal	20-Sep-12
Design Flow Rate = 40 CFS	

			ALIGN	IMENT 1	ALIGN	IMENT 5
ITEM	UNIT	UNIT COST	QTY	COST	QTY	COST
Annual Operations and Maintenance Cost ¹				\$17,820		\$24,620
Pumping Power Costs ²						
Basic Charge (3-Phase Power)	/EA/MO	\$14.50	1	\$14.50	1	\$14.50
Monthly Demand Charge	/HP/MO	\$3.52	1,300	\$4,576.00	1,240	\$4,364.80
Energy Charge						
2-Week Annual Pumping Duration	kWH	\$0.0165	325,853	\$5,376.57	310,813	\$5,128.42
4-Week Annual Pumping Duration	kWH	\$0.0165	651,706	\$10,753.14	621,627	\$10,256.84
6-Week Annual Pumping Duration	kWH	\$0.0165	977,558	\$16,129.71	932,440	\$15,385.27
8-Week Annual Pumping Duration	kWH	\$0.0165	1,303,411	\$21,506.28	1,243,254	\$20,513.69
Total Annual Pumping Costs						
2-Week Annual Pumping Duration				\$28,329		\$27,025
4-Week Annual Pumping Duration				\$33,706		\$32,153
6-Week Annual Pumping Duration				\$39,082		\$37,282
8-Week Annual Pumping Duration				\$44,459		\$42,410
Total Annual Operating Costs ³						
2-Week Annual Pumping Duration				\$46,100		\$51,600
4-Week Annual Pumping Duration				\$51,500		\$56,800
6-Week Annual Pumping Duration				\$56,900		\$61,900
8-Week Annual Pumping Duration				\$62,300		\$67,000

Notes:

1) Annual Operations and Maintenance Costs include estimated salaries, benefits, transportation, maintenance, repairs, administration, insurance 2) Pumping power costs are based on Chelan PUD Electrical Rate Schedule 5 (Irrigation Service), and are applied May through June, in 2012 dollars 3) Rounded to nearest \$100.

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal Alternative 1, Design Flow Rate = 10 CFS

ASSUMPTIONS:												
Estimated Capita	al Cost:		\$1,648,000 Total Capita	l Cost								
Interest on Repl	acement F	und:	3.00%									
Rate of Inflation	:		3.00%									
Project Design L	ife:		50 Years									
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³								
Estimated Proje	ct Replacer	ment Cost:										
To Replace	25%	After Life of Project		\$1,806,169								
To Replace	50%	After Life of Project		\$3,612,339								
To Replace	100%	After Life of Project	\$1,648,000	\$7,224,677								
Disposal and Re	moval Cost											
To Replace	25%	After Life of Project		\$52,607								
To Replace	50%	After Life of Project		\$105,214								
To Replace	100%	After Life of Project	\$48,000	\$210,427								
Total Replacem	ent Cost:											
To Replace	25%	After Life of Project		\$1,858,776								

\$1,696,000

\$1,648,000

REPLACEMENT F	UND SUN	IMARY								
Annual Deposit I	Required (Assume Equal Deposit Made	Each Year):							
To Replace	25%	After Life of Project	\$16,479							
To Replace	50%	After Life of Project	\$32,958							
To Replace	100%	After Life of Project	\$65,916							
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):							
To Replace	25%	After Life of Project	\$8,734							
To Replace	50%	After Life of Project	\$17,469							
To Replace	100%	After Life of Project	\$34,938							
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):							
To Replace	25%	After Life of Project	\$17,755							
To Replace	50%	After Life of Project	\$35,510							
To Replace	100%	After Life of Project	\$71,021							
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):							
To Replace	25%	After Life of Project	\$37,176							
To Replace	50%	After Life of Project	\$74,351							
To Replace	100%	After Life of Project	\$148,702							

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:												
(PRESENT VALUE OF LONG-TERM	Replacment											
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL								
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:									
25% Replacement	\$424,000	\$596,000	\$351,398	\$1,371,398								
50% Replacement	\$848,000	\$596,000	\$351,398	\$1,795,398								
100% Replacement	\$1,696,000	\$596,000	\$351,398	\$2,643,398								
Assuming the Pumping Power Costs for a 4-week Annual Operating Duration:												
25% Replacement	\$424,000	\$596,000	\$417,571	\$1,437,571								
50% Replacement	\$848,000	\$596,000	\$417,571	\$1,861,571								
100% Replacement	\$1,696,000	\$596,000	\$417,571	\$2,709,571								
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:									
25% Replacement	\$424,000	\$596,000	\$483,745	\$1,503,745								
50% Replacement	\$848,000	\$596,000	\$483,745	\$1,927,745								
100% Replacement	\$1,696,000	\$596,000	\$483,745	\$2,775,745								
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:									
25% Replacement	\$424,000	\$596,000	\$549,918	\$1,569,918								
50% Replacement	\$848,000	\$596,000	\$549,918	\$1,993,918								
100% Replacement	\$1,696,000	\$596,000	\$549,918	\$2,841,918								

LIFE CYCLE COST	rs:
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LIFE CYCLE COSTS:																					
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

Replacement Fund (For Funding Replacement of 25% of System):

To Replace 50% After Life of Project

To Replace 100% After Life of Project

Deposits		\$8,734	\$8,996	\$9,266	\$9,544	\$9,831	\$10,126	\$10,429	\$10,742	\$11,064	\$11,396	\$11,738	\$12,090	\$12,453	\$12,827	\$13,212	\$13,608	\$14,016	\$14,437	\$14,870	
Interest		\$0	\$262	\$540	\$834	\$1,145	\$1,475	\$1,823	\$2,190	\$2,578	\$2,987	\$3,419	\$3,874	\$4,353	\$4,857	\$5,387	\$5,945	\$6,532	\$7,148	\$7,796	
End of Year Balance		\$8,734	\$17,993	\$27,799	\$38,177	\$49,153	\$60,753	\$73,005	\$85,938	\$99,580	\$113,964	\$129,121	\$145,085	\$161,891	\$179,575	\$198,173	\$217,727	\$238,275	\$259,859	\$282,525	
Replacement Fund (For Funding Replacement of	f 50% of Sys	,																			
Deposits		\$17,469	\$17,993	\$18,533	\$19,089	\$19,661	\$20,251	\$20,859	\$21,484	\$22,129	\$22,793	\$23,477	\$24,181	\$24,906	\$25,654	\$26,423	\$27,216	\$28,032	\$28,873	\$29,739	
Interest		\$0	\$524	\$1,080	\$1,668	\$2,291	\$2,949	\$3,645	\$4,380	\$5,156	\$5,975	\$6,838	\$7,747	\$8,705	\$9,713	\$10,774	\$11,890	\$13,064	\$14,296	\$15,592	
End of Year Balance		\$17,469	\$35,986	\$55,598	\$76,355	\$98,306	\$121,507	\$146,011	\$171,875	\$199,161	\$227,928	\$258,243	\$290,171	\$323,782	\$359,149	\$396,347	\$435,453	\$476,549	\$519,719	\$565,050	
Replacement Fund (For Funding Replacement of	f 100% of Sv	stem):																			
Deposits		\$34,938	\$35,986	\$37,065	\$38,177	\$39,323	\$40,502	\$41,717	\$42,969	\$44,258	\$45,586	\$46,953	\$48,362	\$49,813	\$51,307	\$52,846	\$54,432	\$56,065	\$57,747	\$59,479	
Interest		\$0	\$1,048	\$2,159	\$3,336	\$4,581	\$5,898	\$7,290	\$8,761	\$10,313	\$11,950	\$13,676	\$15,495	\$17,410	\$19,427	\$21,549	\$23,781	\$26,127	\$28,593	\$31,183	
End of Year Balance		\$34,938	\$71,971	\$111,196	\$152,709	\$196,613	\$243,014	\$292,021	\$343,751	\$398,321	\$455,856	\$516,485	\$580,342	\$647,565	\$718,299	\$792,694	\$870,906	\$953 <i>,</i> 098	\$1,039,438	\$1,130,100	
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$5,800	\$5,974	\$6,153	\$6,338	\$6,528	\$6,724	\$6,926	\$7,133	\$7,347	\$7,568	\$7,795	\$8,029	\$8,269	\$8,517	\$8,773	\$9,036	\$9,307	\$9,587	\$9,874	
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
Total O&M Expenses		\$11,920	\$12,278	\$12,646	\$13,025	\$13,416	\$13,819	\$14,233	\$14,660	\$15,100	\$15,553	\$16,019	\$16,500	\$16,995	\$17,505	\$18,030	\$18,571	\$19,128	\$19,702	\$20,293	
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$7,028	\$7,239	\$7,456	\$7,680	\$7,910	\$8,147	\$8,392	\$8,644	\$8,903	\$9,170	\$9,445	\$9,728	\$10,020	\$10,321	\$10,630	\$10,949	\$11,278	\$11,616	\$11,965	,
4-Week Annual Pumping Duration ⁸		\$8,351	\$8,602	\$8,860	\$9,126	\$9,400	\$9,682	\$9,972	\$10,271	\$10,579	\$10,897	\$11,224	\$11,560	\$11,907	\$12,264	\$12,632	\$13,011	\$13,402	\$13,804	\$14,218	;
6-Week Annual Pumping Duration ⁸		\$9,675	\$9,965	\$10,264	\$10,572	\$10,889	\$11,216	\$11,552	\$11,899	\$12,256	\$12,624	\$13,002	\$13,392	\$13,794	\$14,208	\$14,634	\$15,073	\$15,525	\$15,991	\$16,471	
8-Week Annual Pumping Duration ⁸		\$10.998	\$11,328	\$11,668	\$12,018	\$12,379	\$12,750	\$13,133	\$13,527	\$13,932	\$14,350	\$14,781	\$15,224	\$15,681	\$16,151	\$16,636	\$17,135	\$17,649	\$18,179	\$18,724	
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						_		_													
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	1

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$3,717,552

\$7,435,105

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
\$15,316	\$15,775	\$16,249	\$16,736
\$8,476	\$9,189	\$9,938	\$10,724
\$306,316	\$331,281	\$357,468	\$384,928
<i></i>	<i></i>	<i></i> ,	
\$30,632	\$31,551	\$32,497	\$33,472
\$16,951	\$18,379	\$19,877	\$21,448
\$612,633	\$662,563	\$714,937	\$769,857
4	400.000	40.000	
\$61,263	\$63,101	\$64,994	\$66,944
\$33,903	\$36,758	\$39,754	\$42,896
\$1,225,266	\$1,325,125	\$1,429,873	\$1,539,713
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$10,170	\$10,475	\$10,790	\$11,113
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$20,902	\$21,529	\$22,175	\$22,840
\$12,324	\$12,693	\$13,074	\$13,466
\$14,644	\$15,084	\$15,536	\$16,002
\$16,965	\$17,474	\$17,998	\$18,538
\$19,286	\$19,864	\$20,460	\$21,074
20	21	22	23
20	21	~~~	23

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50
			l																							
\$17,238 \$11,548	\$17,755 \$12,411	\$18,288 \$13,316	\$18,837 \$14,265	\$19,402 \$15,258	\$19,984 \$16,297	\$20,583 \$17,386	\$21,201 \$18,525	\$21,837 \$19,717	\$22,492 \$20,963	\$23,167 \$22,267	\$23,862 \$23,630	\$24,577 \$25,055	\$25,315 \$26,544	\$26,074 \$28,099	\$26,856 \$29,725	\$27,662 \$31,422	\$28,492 \$33,194	\$29,347 \$35,045	\$30,227 \$36,977	\$31,134 \$38,993	\$32,068 \$41,097	\$33,030 \$43,292	\$34,021 \$45.581	\$35,041 \$47,969	\$36,093 \$50,460	\$37,176 \$53,056
\$11,548 \$ 413,714	\$12,411 \$443,881	\$15,516 \$475,485	\$14,205 \$508,586	\$15,258 \$ 543,246	\$10,297 \$579,527	\$17,386 \$617,496	\$18,525 \$657,221	\$19,717 \$698,774	\$20,985 \$742,229	\$787,663	\$23,630 \$835,154	\$25,055 \$884,786	\$26,544 \$936,645	\$28,099 \$ 990,818	\$29,725 \$1,047,399	\$31,422 \$1,106,483	\$35,194 \$1,168,170		\$30,977 \$ 1,299,765	\$38,995 \$ 1,369,892	\$1,443,057 \$1,443,057	\$43,292 \$ 1,519,379	1 - 7	\$47,969 \$ 1,681,992	. ,	\$55,050 \$1,858,776
\$34,476	\$35,510	\$36,576	\$37,673	\$38,803	\$39,967	\$41,166	\$42,401	\$43,673	\$44,984	\$46,333	\$47,723	\$49,155	\$50,629	\$52,148	\$53,713	\$55,324	\$56,984	\$58,693	\$60,454	\$62,268	\$64,136	\$66,060	\$68,042	\$70,083	\$72,185	\$74,351
\$23,096	\$24,823	\$26,633	\$28,529	\$30,515	\$32,595	\$34,772	\$37,050	\$39,433	\$41,926	\$44,534	\$47,260	\$50,109	\$53,087	\$56,199	\$59,449	\$62,844	\$66,389	\$70,090	\$73,954	\$77,986	\$82,194	\$86,583	\$91,163	\$95,939	\$100,920	\$106,113
\$827,429	\$887,762	\$950,970	\$1,017,173	\$1,086,491	\$1,159,053	\$1,234,991	\$1,314,442	\$1,397,549	\$1,484,459	\$1,575,326	\$1,670,309	\$1,769,573	\$1,873,289	\$1,981,636	\$2,094,798	\$2,212,966	\$2,336,339	\$2,465,123	\$2,599,531	\$2,739,785	\$2,886,114	\$3,038,757	\$3,197,962	\$3,363,984	\$3,537,089	\$3,717,552
\$68,952	\$71,021	\$73,152	\$75,346	\$77,607	\$79,935	\$82,333	\$84,803	\$87,347	\$89,967	\$92,666	\$95,446	\$98,310	\$101,259	\$104,297	\$107,426	\$110,648	\$113,968	\$117,387	\$120,908	\$124,536	\$128,272	\$132,120	\$136,083	\$140,166	\$144,371	\$148,702
\$46,191	\$49,646	\$53,266	\$57,058	\$61,030	\$65,189	\$69,543	\$74,099	\$78,867	\$83,853	\$89,068	\$94,520	\$100,219	\$106,174	\$112,397	\$118,898	\$125,688	\$132,778	\$140,180	\$147,907	\$155,972	\$164,387	\$173,167	\$182,325	\$191,878	\$201,839	\$212,225
\$1,654,857	\$1,775,524	\$1,901,941	\$2,034,345	\$2,172,982	\$2,318,106	\$2,469,982	\$2,628,884	\$2,795,098	\$2,968,918	\$3,150,652	\$3,340,618	\$3,539,146	\$3,746,579	\$3,963,273	\$4,189,597	\$4,425,933	\$4,672,679	\$4,930,246	\$5,199,062	\$5,479,569	\$5,772,228	\$6,077,515	\$6,395,924	\$6,727,967	\$7,074,177	\$7,435,105
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1,948	\$2,006	\$2,066	\$2,128
\$11,447 \$1,974	\$11,790 \$2,033	\$12,144 \$2,094	\$12,508 \$2,157	\$12,883 \$2,221	\$13,270 \$2,288	\$13,668 \$2,357	\$14,078 \$2,427	\$14,500 \$2,500	\$14,935 \$2,575	\$15,384 \$2,652	\$15,845 \$2,732	\$16,320 \$2,814	\$16,810 \$2,898	\$17,314 \$2,985	\$17,834 \$3,075	\$18,369 \$3,167	\$18,920 \$3,262	\$19,487 \$3,360	\$20,072 \$3,461	\$20,674 \$3,565	\$21,294 \$3,671	\$21,933 \$3,782	\$22,591 \$3,895	\$23,269 \$4,012	\$23,967 \$4,132	\$24,686 \$4,256
\$1,574 \$0	\$2,035 \$0	\$2,054 \$0	\$2,157	\$2,221	\$2,288 \$0	\$0	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,052 \$0	\$2,732 \$0	\$2,814 \$0	\$2,858 \$0	\$2,585 \$0	\$3,075 \$0	\$3,107	\$3,202 \$0	\$3,500 \$0	\$0,401 \$0	\$3,505 \$0	\$3,071	\$3,782	\$3,855 \$0	\$4,012 \$0	\$0	\$4,230 \$0
\$23,525	\$24,231	\$24,958	\$25,707	\$26,478	\$27,272	\$28,090	\$28,933	\$29,801	\$30,695	\$31,616	\$32,564	\$33,541	\$34,547	\$35,584	\$36,651	\$37,751	\$38,883	\$40,050	\$41,251	\$42,489	\$43,764	\$45,077	\$46,429	\$47,822	\$49,256	\$50,734
\$13,870	\$14,286	\$14,715	\$15,156	\$15,611	\$16,079	\$16,562	\$17,059	\$17,570	\$18,098	\$18,641	\$19,200	\$19,776	\$20,369	\$20,980	\$21,609	\$22,258	\$22,925	\$23,613	\$24,322	\$25,051	\$25,803	\$26,577	\$27,374	\$28,195	\$29,041	\$29,913
\$16,482	\$16,977	\$17,486	\$18,011	\$18,551	\$19,107	\$19,681	\$20,271	\$20,879	\$21,506	\$22,151	\$22,815	\$23,500	\$24,205	\$24,931	\$25,679	\$26,449	\$27,243	\$28,060	\$28,902	\$29,769	\$30,662	\$31,582	\$32,529	\$33,505	\$34,510	\$35,546
\$19,094	\$19,667	\$20,257	\$20,865	\$21,491	\$22,135	\$22,800	\$23,483	\$24,188	\$24,914	\$25,661	\$26,431	\$27,224	\$28,041	\$28,882	\$29,748	\$30,641	\$31,560	\$32,507	\$33,482	\$34,486	\$35,521	\$36,587	\$37,684	\$38,815	\$39,979	\$41,178
\$21,706	\$22,357	\$23,028	\$23,719	\$24,431	\$25,163	\$25,918	\$26,696	\$27,497	\$28,322	\$29,171	\$30,046	\$30,948	\$31,876	\$32,833	\$33,818	\$34,832	\$35,877	\$36,953	\$38,062	\$39,204	\$40,380	\$41,591	\$42,839	\$44,124	\$45,448	\$46,811
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal Alternative 5, Design Flow Rate = 10 CFS

ASSUMPTIONS:				
Estimated Capit	al Cost:		\$1,777,000 Total Cap	ital Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
1				
SUMMARY REP	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replacer	ment Cost:		
To Replace	25%	After Life of Project		\$1,947,550
To Replace	50%	After Life of Project		\$3,895,100
To Replace	100%	After Life of Project	\$1,777,000	\$7,790,201
Disposal and Re	moval Cost			
To Replace	25%	After Life of Project		\$90,966
To Replace	50%	After Life of Project		\$181,932
To Replace	100%	After Life of Project	\$83,000	\$363,864

\$1,860,000

\$1,777,000

REPLACEMENT	FUND SUN	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$18,072	
To Replace	50%	After Life of Project	\$36,145	
To Replace	100%	After Life of Project	\$72,290	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$9,579	
To Replace	50%	After Life of Project	\$19,158	
To Replace	100%	After Life of Project	\$38,316	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$19,472	
To Replace	50%	After Life of Project	\$38,944	
To Replace	100%	After Life of Project	\$77,889	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$40,770	
To Replace	50%	After Life of Project	\$81,541	
To Replace	100%	After Life of Project	\$163,081	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 50th Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$465,000	\$806,000	\$335,096	\$1,606,096
50% Replacement	\$930,000	\$806,000	\$335,096	\$2,071,096
100% Replacement	\$1,860,000	\$806,000	\$335,096	\$3,001,096
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$465,000	\$806,000	\$398,168	\$1,669,168
50% Replacement	\$930,000	\$806,000	\$398,168	\$2,134,168
100% Replacement	\$1,860,000	\$806,000	\$398,168	\$3,064,168
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$465,000	\$806,000	\$461,239	\$1,732,239
50% Replacement	\$930,000	\$806,000	\$461,239	\$2,197,239
100% Replacement	\$1,860,000	\$806,000	\$461,239	\$3,127,239
Assuming the Pumping Power Costs for an 8	-week Annual	Operating Dura	ation:	
25% Replacement	\$465,000	\$806,000	\$524,310	\$1,795,310
50% Replacement	\$930,000	\$806,000	\$524,310	\$2,260,310
100% Replacement	\$1,860,000	\$806,000	\$524,310	\$3,190,310

LIFE	CYCLE	COSTS:

Year 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19	LIFE CYCLE COSTS:																				
	Year	0	1	2	3	4	5	6	7	8	9	10	12	13	14	15	16	17	18	19	

Capital Expenses:

Replacement Fund (For Funding Replacement of 25% of System):

 Total Replacement Cost:

 To Replace
 25%
 After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

Replacement i and (i or i anding replacement	10 01 23/0 01 3	scenij.																			
Deposits		\$9,579	\$9,866	\$10,162	\$10,467	\$10,781	\$11,105	\$11,438	\$11,781	\$12,134	\$12,498	\$12,873	\$13,260	\$13,657	\$14,067	\$14,489	\$14,924	\$15,371	\$15,833	\$16,308	
Interest		\$0	\$287	\$592	\$915	\$1,256	\$1,617	\$1,999	\$2,402	\$2,827	\$3,276	\$3,750	\$4,248	\$4,773	\$5,326	\$5,908	\$6,520	\$7,163	\$7,839	\$8,550	,
End of Year Balance		\$9,579	\$19,733	\$30,487	\$41,869	\$53,906	\$66,628	\$80,065	\$94,248	\$109,210	\$124,984	\$141,607	\$159,115	\$177,546	\$196,939	\$217,336	\$238,780	\$261,315	\$284,987	\$309,845	
Replacement Fund (For Funding Replacement	nt of 50% of Sy	/stem):																			
Deposits		\$19,158	\$19,733	\$20,325	\$20,934	\$21,562	\$22,209	\$22,876	\$23,562	\$24,269	\$24,997	\$25,747	\$26,519	\$27,315	\$28,134	\$28,978	\$29,848	\$30,743	\$31,665	\$32,615	
Interest		\$0	\$575	\$1,184	\$1,829	\$2,512	\$3,234	\$3,998	\$4,804	\$5,655	\$6,553	\$7,499	\$8,496	\$9,547	\$10,653	\$11,816	\$13,040	\$14,327	\$15,679	\$17,099	1
End of Year Balance		\$19,158	\$39,465	\$60,974	\$83,738	\$107,812	\$133,256	\$160,130	\$188,495	\$218,419	\$249,968	\$283,214	\$318,230	\$355,091	\$393,878	\$434,673	\$477,561	\$522,630	\$569,975	\$619,689	
Replacement Fund (For Funding Replaceme	nt of 100% of §	System):																			
Deposits		\$38,316	\$39,465	\$40,649	\$41,869	\$43,125	\$44,419	\$45,751	\$47,124	\$48,538	\$49,994	\$51,493	\$53,038	\$54,629	\$56,268	\$57,956	\$59,695	\$61,486	\$63,331	\$65,230	1
Interest		\$0	\$1,149	\$2,368	\$3,658	\$5,024	\$6,469	\$7,995	\$9,608	\$11,310	\$13,105	\$14,998	\$16,993	\$19,094	\$21,305	\$23,633	\$26,080	\$28,654	\$31,358	\$34,198	i.
End of Year Balance		\$38,316	\$78,931	\$121,948	\$167,476	\$215,625	\$266,512	\$320,259	\$376,991	\$436,838	\$499,937	\$566,428	\$636,460	\$710,183	\$787,757	\$869,346	\$955,121	\$1,045,261	\$1,139,949	\$1,239,378	
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	1
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	/
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	
Maintenance and Small Repairs ⁷		\$10,000	\$10,300	\$10,609	\$10,927	\$11,255	\$11,593	\$11,941	\$12,299	\$12,668	\$13,048	\$13,439	\$13,842	\$14,258	\$14,685	\$15,126	\$15,580	\$16,047	\$16,528	\$17,024	
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	:
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	,
Total O&M Expenses		\$16,120	\$16,604	\$17,102	\$17,615	\$18,143	\$18,687	\$19,248	\$19,826	\$20,420	\$21,033	\$21,664	\$22,314	\$22,983	\$23,673	\$24,383	\$25,114	\$25,868	\$26,644	\$27,443	
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$6,702	\$6,903	\$7,110	\$7,323	\$7,543	\$7,769	\$8,002	\$8,243	\$8,490	\$8,744	\$9,007	\$9,277	\$9,555	\$9,842	\$10,137	\$10,441	\$10,755	\$11,077	\$11,410	,
4-Week Annual Pumping Duration ⁸		\$7,963	\$8,202	\$8,448	\$8,702	\$8,963	\$9,232	\$9,509	\$9,794	\$10,088	\$10,390	\$10,702	\$11,023	\$11,354	\$11,694	\$12,045	\$12,407	\$12,779	\$13,162	\$13,557	,
6-Week Annual Pumping Duration ⁸		\$9,225	\$9,502	\$9,787	\$10,080	\$10,383	\$10,694	\$11,015	\$11,345	\$11,686	\$12,036	\$12,397	\$12,769	\$13,152	\$13,547	\$13,953	\$14,372	\$14,803	\$15,247	\$15,705	
8-Week Annual Pumping Duration ⁸		\$10,486	\$10,801	\$11,125	\$11,459	\$11,802	\$12,156	\$12,521	\$12,897	\$13,284	\$13,682	\$14,093	\$14,515	\$14,951	\$15,399	\$15,861	\$16,337	\$16,827	\$17,332	\$17,852	1
					I		I	<u> </u>	<u> </u>			<u> </u>	I	<u> </u>	<u> </u>			, ,			Г
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$2,038,516

\$4,077,033

\$8,154,065

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
¢16 707	ć17 201	ć17 930	Ć10 2F4
\$16,797 \$9,295	\$17,301 \$10,078	\$17,820 \$10,899	\$18,354 \$11,761
\$335,937	\$10,078 \$363,316	\$392,035	\$422,150
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	<i>\$</i> 303,310	<i>4352,033</i>	<i>3422,130</i>
\$33,594	\$34,601	\$35,640	\$36,709
\$18,591	\$20,156	\$21,799	\$23,522
\$671,873	\$726,631	\$784,070	\$844,300
\$67,187	\$69,203	\$71,279	\$73,417
\$37,181	\$40,312	\$43,598	\$47,044
\$1,343,747	\$1,453,262	\$1,568,139	\$1,688,601
ćr 707	ć= 000	¢C 130	ćc 222
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$17,535	\$18,061	\$18,603	\$19,161
\$1,754	\$1,806	\$1,860	\$1,916
\$0 \$28 267	\$0 \$20 115	\$0 \$20 099	\$0 \$20 999
\$28,267	\$29,115	\$29,988	\$30,888
\$11,752	\$12,104	\$12,468	\$12,842
\$13,964	\$14,383	\$14,814	\$15,259
\$16,176	\$16,661	\$17,161	\$17,676
\$18,388	\$18,939	\$19,507	\$20,093
	,,	,	
20	21	22	23

\$18,905 \$12,665 \$453,720	\$19,472 \$13,612 \$486,803	\$20,056 \$14,604 \$521,464	\$20,658 \$15,644 \$557,766	\$21,278 \$16,733 \$595,776	\$21,916 \$17,873 \$635,566	\$22,574 \$19,067 \$677,206	\$23,251 \$20,316 \$720,773	\$23,948 \$21,623 \$766,345	\$24,667 \$22,990 \$814,002	\$25,407 \$24,420 \$863,828	\$26,169 \$25,915 \$915,912	\$26,954 \$27,477 \$970,344	\$27,763 \$29,110 \$1,027,216	\$28,595 \$30,816 \$1,086,628	\$29,453 \$32,599 \$1,148,681	\$30,337 \$34,460 \$1,213,478	\$31,247 \$36,404 \$1,281,129	\$32,184 \$38,434 \$1,351,748	\$33,150 \$40,552 \$1,425,450	\$34,145 \$42,764 \$1,502,358	\$35,169 \$45,071 \$1,582,598	\$47,478	
\$37,810	\$38,944	\$40,113	\$41,316	\$42,555	\$43,832	\$45,147	\$46,501	\$47,897	\$49,333	\$50,813	\$52,338	\$53,908	\$55,525	\$57,191	\$58,907	\$60,674	\$62,494	\$64,369	\$66,300	\$68,289	\$70,338	\$72,448	
\$25,329	\$27,223	\$29,208	\$31,288	\$33,466	\$35,747	\$38,134	\$40,632	\$43,246	\$45,981	\$48,840	\$51,830	\$54,955	\$58,221	\$61,633	\$65,198	\$68,921	\$72,809	\$76,868	\$81,105	\$85,527	\$90,141	\$94,956	
\$907,439	\$973 <i>,</i> 607	\$1,042,928	\$1,115,531	\$1,191,553	\$1,271,131	\$1,354,412	\$1,441,546	\$1,532,689	\$1,628,003	\$1,727,657	\$1,831,824	\$1,940,687	\$2,054,433	\$2,173,257	\$2,297,361	\$2,426,956	\$2,562,259	\$2,703,496	\$2,850,901	\$3,004,717	\$3,165,196	\$3,332,599	\$
\$75,620	\$77,889	\$80,225	\$82,632	\$85,111	\$87,664	\$90,294	\$93,003	\$95,793	\$98,667	\$101,627	\$104,676	\$107,816	\$111,050	\$114,382	\$117,813	\$121,348	\$124,988	\$128,738	\$132,600	\$136,578	\$140,675	\$144,896	
\$50,658	\$54,446	\$58,416	\$62,576	\$66,932	\$71,493	\$76,268	\$81,265	\$86,493	\$91,961	\$97,680	\$103,659	\$109,909	\$116,441	\$123,266	\$130,395	\$137,842	\$145,617	\$153,736	\$162,210	\$171,054	\$180,283	\$189,912	
\$1,814,879	\$1,947,213	\$2,085,855	\$2,231,063	\$2,383,105	\$2,542,263	\$2,708,825	\$2,883,093	\$3,065,379	\$3,256,007	\$3,455,314	\$3,663,649	\$3,881,374	\$4,108,866	\$4,346,514	\$4,594,723	\$4,853,912	\$5,124,518	\$5,406,991	\$5,701,801	\$6,009,433	\$6,330,391	\$6,665,199	\$
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846		
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	
\$19,736	\$20,328	\$20,938	\$21,566	\$22,213	\$22,879	\$23,566	\$24,273	\$25,001	\$25,751	\$26,523	\$27,319	\$28,139	\$28,983	\$29,852	\$30,748	\$31,670	\$32,620	\$33,599	\$34,607	\$35,645	\$36,715	\$37,816	
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671		
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0		
\$31,814	\$32,769	\$33,752	\$34,764	\$35,807	\$36,881	\$37,988	\$39,127	\$40,301	\$41,510	\$42,756	\$44,038	\$45,359	\$46,720	\$48,122	\$49,566	\$51,052	\$52,584	\$54,162	\$55,786	\$57,460	\$59,184	\$60,959	
\$13,227	\$13,624	\$14,032	\$14,453	\$14,887	\$15,334	\$15,794	\$16,267	\$16,755	\$17,258	\$17,776	\$18,309	\$18,858	\$19,424	\$20,007	\$20,607	\$21,225	\$21,862	\$22,518	\$23,193	\$23,889	\$24,606	\$25,344	
\$15,716	\$16,188	\$16,673	\$17,174	\$17,689	\$18,220	\$18,766	\$19,329	\$19,909	\$20,506	\$21,121	\$21,755	\$22,408	\$23,080	\$23,772	\$24,486	\$25,220	\$25,977	\$26,756	\$27,559	\$28,386	\$29,237	\$30,114	
\$18,206	\$18,752	\$19,315	\$19,894	\$20,491	\$21,106	\$21,739	\$22,391	\$23,063	\$23,755	\$24,467	\$25,201	\$25,957	\$26,736	\$27,538	\$28,364	\$29,215	\$30,092	\$30,994	\$31,924	\$32,882	\$33,868	\$34,884	
\$20,695	\$21,316	\$21,956	\$22,614	\$23,293	\$23,992	\$24,711	\$25,453	\$26,216	\$27,003	\$27,813	\$28,647	\$29,507	\$30,392	\$31,304	\$32,243	\$33,210	\$34,206	\$35,233	\$36,290	\$37,378	\$38,500	\$39,655	
							1		1		1		1		1	1	1		1	1			-
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	1
		-																					1

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal (Cont.)

47	48	49	50
\$37,311	\$38,430	\$39,583	\$40,770
\$49,989	\$52,608	\$55,339	\$58,187
\$1,753,599	\$1,844,637	\$1,939,559	\$2,038,516
\$74,621	\$76,860	\$79,166	\$81,541
\$99,978	\$105,216	\$110,678	\$116,374
\$3,507,199	\$3,689,274	\$3,879,118	\$4,077,033
\$149,242	\$153,720	\$158,331	\$163,081
\$199,956	\$210,432	\$221,356	\$232,747
\$7,014,397	\$7,378,549	\$7,758,237	\$8,154,065
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$38,950	\$40,119	\$41,323	\$42,562
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$62,788	\$64,672	\$66,612	\$68,610
\$26,104	\$26,887	\$27,694	\$28,525
\$31,018	\$31,948	\$32,907	\$33,894
\$35,931	\$37,009	\$38,119	\$39,263
\$40,844	\$42,070	\$43,332	\$44,632
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal Alternative 1, Design Flow Rate = 20 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$2,463,000 Total Cap	ital Cost
Interest on Repl	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design L	ife:		50 Years	
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Proje	ct Replacer	ment Cost:		
To Replace	25%	After Life of Project		\$2,699,390
To Replace	50%	After Life of Project		\$5,398,780
To Replace	100%	After Life of Project	\$2,463,000	\$10,797,561
Disposal and Re	moval Cost			
To Replace	25%	After Life of Project		\$73,430

\$67,000

\$2,530,000

\$2,463,000

REPLACEMENT I	UND SUN	IMARY		
Annual Deposit I	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$24,582	
To Replace	50%	After Life of Project	\$49,165	
To Replace	100%	After Life of Project	\$98,330	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$13,030	
To Replace	50%	After Life of Project	\$26,059	
To Replace	100%	After Life of Project	\$52,118	
Deposit Require	d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$26,486	
To Replace	50%	After Life of Project	\$52,973	
To Replace	100%	After Life of Project	\$105,945	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$55,456	
To Replace	50%	After Life of Project	\$110,913	
To Replace	100%	After Life of Project	\$221,826	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 50th Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$632,500	\$711,000	\$677,436	\$2,020,936
50% Replacement	\$1,265,000	\$711,000	\$677,436	\$2,653,436
100% Replacement	\$2,530,000	\$711,000	\$677,436	\$3,918,436
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$632,500	\$711,000	\$805,646	\$2,149,146
50% Replacement	\$1,265,000	\$711,000	\$805,646	\$2,781,646
100% Replacement	\$2,530,000	\$711,000	\$805,646	\$4,046,646
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$632,500	\$711,000	\$933,857	\$2,277,357
50% Replacement	\$1,265,000	\$711,000	\$933,857	\$2,909,857
100% Replacement	\$2,530,000	\$711,000	\$933,857	\$4,174,857
Assuming the Pumping Power Costs for an 8	-week Annual	Operating Dura	ation:	
25% Replacement	\$632,500	\$711,000	\$1,062,067	\$2,405,567
50% Replacement	\$1,265,000	\$711,000	\$1,062,067	\$3,038,067
100% Replacement	\$2,530,000	\$711,000	\$1,062,067	\$4,303,067

LIFE	CYCLE	COSTS:
------	-------	--------

LIFE CICLE COSTS:																					_
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

Total Replacement Cost:

Replacement Fund (For Funding Replacement of 25% of System):	
--------------------------------------------------------------	--

To Replace 50% After Life of Project

To Replace 100% After Life of Project

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

		,-																			
Deposits		\$13,030	\$13,420	\$13,823	\$14,238	\$14,665	\$15,105	\$15,558	\$16,025	\$16,505	\$17,001	\$17,511	\$18,036	\$18,577	\$19,134	\$19,708	\$20,300	\$20,909	\$21,536	\$22,182	1
Interest		\$0	\$391	\$805	\$1,244	\$1,709	\$2,200	\$2,719	\$3,267	\$3,846	\$4,456	\$5,100	\$5,778	\$6,493	\$7,245	\$8,036	\$8,869	\$9,744	\$10,663	\$11,629	J
End of Year Balance		\$13,030	\$26,841	\$41,469	\$56,951	\$73,324	\$90,629	\$108,905	\$128,197	\$148,548	\$170,005	\$192,616	\$216,431	\$241,500	\$267,880	\$295,624	\$324,793	\$355,445	\$387,644	\$421,455	j.
Replacement Fund (For Funding Replacement of 5	50% of Syste	- /																			
Deposits		\$26,059	\$26,841	\$27,646	\$28,475	\$29,330	\$30,210	\$31,116	\$32,049	\$33,011	\$34,001	\$35,021	\$36,072	\$37,154	\$38,269	\$39,417	\$40,599	\$41,817	\$43,072	\$44,364	
Interest		\$0	\$782	\$1,610	\$2,488	\$3,417	\$4,399	\$5,438	\$6,534	\$7,692	\$8,913	\$10,200	\$11,557	\$12,986	\$14,490	\$16,073	\$17,737	\$19,488	\$21,327	\$23,259	
End of Year Balance		\$26,059	\$53,682	\$82,938	\$113,901	\$146,648	\$181,257	\$217,811	\$256,394	\$297,097	\$340,011	\$385,232	\$432,861	\$483,001	\$535,759	\$591,249	\$649,585	\$710,890	\$775,288	\$842,910)
Replacement Fund (For Funding Replacement of 1	100% of Svs	tem):																			
Deposits		\$52,118	\$53,682	\$55,292	\$56,951	\$58,659	\$60,419	\$62,232	\$64,099	\$66,022	\$68,002	\$70,042	\$72,144	\$74,308	\$76,537	\$78,833	\$81,198	\$83,634	\$86,143	\$88,727	,
Interest		\$0	\$1,564	\$3,221	\$4,976	\$6,834	\$8,799	\$10,875	\$13,069	\$15,384	\$17,826	\$20,401	\$23,114	\$25,972	\$28,980	\$32,146	\$35,475	\$38,975	\$42,653	\$46,517	
End of Year Balance		\$52,118	\$107,363	\$165,876	\$227,803	\$293,296	\$362,514	\$435,621	\$512,789	\$594,194	\$680,022	\$770,465	\$865,722	\$966,001	\$1,071,519	\$1,182,497	\$1,299,170	\$1,421,780	\$1,550,576	\$1,685,821	Ł
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	3
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	/
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	Ł
Maintenance and Small Repairs ⁷		\$8,100	\$8,343	\$8,593	\$8,851	\$9,117	\$9,390	\$9,672	\$9,962	\$10,261	\$10,569	\$10,886	\$11,212	\$11,549	\$11,895	\$12,252	\$12,620	\$12,998	\$13,388	\$13,790	J
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	2
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	J
Total O&M Expenses		\$14,220	\$14,647	\$15,086	\$15,539	\$16,005	\$16,485	\$16,979	\$17,489	\$18,013	\$18,554	\$19,110	\$19,684	\$20,274	\$20,883	\$21,509	\$22,154	\$22,819	\$23,503	\$24,209	,
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$13,549	\$13,955	\$14,374	\$14,805	\$15,249	\$15,707	\$16,178	\$16,663	\$17,163	\$17,678	\$18,208	\$18,755	\$19,317	\$19,897	\$20,494	\$21,108	\$21,742	\$22,394	\$23,066	;
4-Week Annual Pumping Duration ⁸		\$16,113	\$16,596	\$17,094	\$17.607	\$18,135	\$18,679	\$19,240	\$19,817	\$20,411	\$21,024	\$21,654	\$22,304	\$22,973	\$23,662	\$24,372	\$25,103	\$25,857	\$26,632	\$27,431	
6-Week Annual Pumping Duration ⁸		\$18,677	\$19,237	\$19,815	\$20,409	\$21,021	\$21,652	\$22,301	\$22,971	\$23,660	\$24,369	\$25,101	\$25,854	\$26,629	\$27,428	\$28,251	\$29,098	\$29,971	\$30,870	\$31,797	
8-Week Annual Pumping Duration ⁸		\$21,241	\$21,879	\$22,535	\$23,211	\$23,907	\$24,625	\$25,363	\$26,124	\$26,908	\$27,715	\$28,547	\$29,403	\$30,285	\$31,194	\$32,129	\$33,093	\$34,086	\$35,109	\$36,162	
o-week Annual Fumping Duration		<i>7</i> 21,241	<i>321,019</i>	<i>322,33</i> 5	<i>723,</i> 211	<i>723,301</i>	<i>724,023</i>	<i>723,3</i> 03	<i>720,12</i> 4	<i>320,3</i> 08	<i>321,1</i> 15	<i>₹</i> 20,347	şz3,403	<i>330,285</i>	<i>331,19</i> 4	<i>332,129</i>	<i>333,095</i>	<i>334,080</i>	\$33,109	<i>330,</i> 102	
Market Market					. [_		_								45	46	47	40	40	Т
Year	U	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	1

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$146,861

\$293,722

\$2,772,821

\$5,545,641

\$11,091,282

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
\$22,847	\$23,533	\$24,239	\$24,966
\$12,644	\$13,708	\$14,826	\$15,998
\$456,946	\$494,187	\$533,252	\$574,215
	, .		,
\$45,695	\$47,065	\$48,477	\$49,932
\$25,287	\$27,417	\$29,651	\$31,995
\$913,892	\$988,375	\$1,066,503	\$1,148,430
		4000	
\$91,389	\$94,131	\$96,955	\$99,863
\$50,575	\$54,834	\$59,302	\$63,990
\$1,827,785	\$1,976,749	\$2,133,006	\$2,296,860
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$14,203	\$14,630	\$15,068	\$15,520
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$24,935	\$25,683	\$26,453	\$27,247
\$23,758	\$24,470	\$25,205	\$25,961
\$28,254	\$29,102	\$29,975	\$30,874
\$32,750	\$33,733	\$34,745	\$35,787
\$37,247	\$38,364	\$39,515	\$40,701
20	21	22	23

\$25,715	\$26,486	\$27,281	\$28,099	\$28,942	\$29,811	\$30,705	\$31,626	\$32,575	\$33,552	\$34,559	\$35,595	\$36,663	\$37,763	\$38,896	\$40,063	\$41,265	\$42,503	\$43,778	\$45,091	\$46,444	\$47,837	\$49,272	\$50,750	\$52,273	\$53,841	\$55,456
\$17,226	\$18,515	\$19,865	\$21,279	\$22,760	\$24,312	\$25,935	\$27,634	\$29,412	\$31,272	\$33,217	\$35,250	\$37,375	\$39,596	\$41,917	\$44,341	\$46,874	\$49,518	\$52,278	\$55,160	\$58,168	\$61,306	\$64,580	\$67,996	\$71,558	\$75,273	\$79,147
\$617,156	\$662,157	\$709,303	\$758,681	\$810,384	\$864,506	\$921,146	\$980,407	\$1,042,393	\$1,107,217	\$1,174,992	\$1,245,838	\$1,319,876	\$1,397,235	\$1,478,048	\$1,562,453	\$1,650,591	\$1,742,612	\$1,838,668	\$1,938,919	\$2,043,530	\$2,152,673	\$2,266,526	\$2,385,272	\$2,509,103	\$2,638,218	\$2,772,821
\$51,430	\$52,973	\$54,562	\$56,199	\$57,885	\$59,621	\$61,410	\$63,252	\$65,150	\$67,104	\$69,117	\$71,191	\$73,326	\$75,526	\$77,792	\$80,126	\$82,530	\$85,005	\$87,556	\$90,182	\$92,888	\$95,674	\$98,545	\$101,501	\$104,546	\$107,682	\$110,913
\$34,453	\$37,029	\$39,729	\$42,558	\$45,521	\$48,623	\$51,870	\$55,269	\$58,824	\$62,544	\$66,433	\$70,500	\$74,750	\$79,193	\$83,834	\$88,683	\$93,747	\$99,035	\$104,557	\$110,320	\$116,335	\$122,612	\$129,160	\$135,992	\$143,116	\$150,546	\$158,293
\$1,234,313	\$1,324,315	\$1,418,606	\$1,517,363	\$1,620,768	\$1,729,012	\$1,842,292	\$1,960,813	\$2,084,787	\$2,214,435	\$2,349,985	\$2,491,675	\$2,639,752	\$2,794,471	\$2,956,097	\$3,124,906	\$3,301,182	\$3,485,223	\$3,677,335	\$3,877,838	\$4,087,061	\$4,305,347	\$4,533,052	\$4,770,544	\$5,018,207	\$5,276,435	\$5,545,641
\$102,859	\$105,945	\$109,124	\$112,397	\$115,769	\$119,242	\$122,819	\$126,504	\$130,299	\$134,208	\$138,234	\$142,381	\$146,653	\$151,052	\$155,584	\$160,252	\$165,059	\$170,011	\$175,111	\$180,365	\$185,775	\$191,349	\$197,089	\$203,002	\$209,092	\$215,365	\$221,826
\$68,906	\$74,059	\$79,459	\$85,116	\$91,042	\$97,246	\$103,741	\$110,538	\$117,649	\$125,087	\$132,866	\$140,999	\$149,501	\$158,385	\$167,668	\$177,366	\$187,494	\$198,071	\$209,113	\$220,640	\$232,670	\$245,224	\$258,321	\$271,983	\$286,233	\$301,092	\$316,586
\$2,468,625	\$2,648,629	\$2,837,211	\$3,034,725	\$3,241,536	\$3,458,024	\$3,684,584	\$3,921,626	\$4,169,574	\$4,428,869	\$4,699,970	\$4,983,350	\$5,279,504	\$5,588,941	\$5,912,194	\$6,249,811	\$6,602,364	\$6,970,446	\$7,354,671	\$7,755,676	\$8,174,121	\$8,610,694	\$9,066,104	\$9,541,089	\$10,036,413	\$10,552,870	\$11,091,282
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	\$12,854	\$13,239	\$13,636	\$14,046
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,455	\$5,618
\$987	\$1.016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1.214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	\$1.948	\$2,006	\$2,066	\$2,128
\$15,986	\$16,466	\$16,960	\$17,468	\$17,992	\$18,532	\$19,088	\$19,661	\$20,251	\$20,858	\$21,484	\$22,128	\$22,792	\$23,476	\$24,180	\$24,906	\$25,653	\$26,423	\$27,215	\$28,032	\$28,873	\$29,739	\$30,631	\$31,550	\$32,496	\$33,471	\$34,475
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
\$28,064	\$28,906	\$29,774	\$30,667	\$31,587	\$32,534	\$33,510	\$34,516	\$35,551	\$36,618	\$37,716	\$38,848	\$40,013	\$41,214	\$42,450	\$43,723	\$45,035	\$46,386	\$47,778	\$49,211	\$50,687	\$52,208	\$53,774	\$55,388	\$57,049	\$58,761	\$60,523
\$26,740	\$27,542	\$28,368	\$29,219	\$30,096	\$30,998	\$31,928	\$32,886	\$33,873	\$34,889	\$35,936	\$37,014	\$38,124	\$39,268	\$40,446	\$41,659	\$42,909	\$44,196	\$45,522	\$46,888	\$48,295	\$49,743	\$51,236	\$52,773	\$54,356	\$55,987	\$57,666
\$31,800	\$32,754	\$33,737	\$34,749	\$35,791	\$36,865	\$37,971	\$39,110	\$40,284	\$41,492	\$42,737	\$44,019	\$45,340	\$46,700	\$48,101	\$49,544	\$51,030	\$52,561	\$54,138	\$55,762	\$57,435	\$59,158	\$60,933	\$62,761	\$64,643	\$66,583	\$68,580
\$36,861	\$37,967	\$39,106	\$40,279	\$41,487	\$42,732	\$44,014	\$45,334	\$46,694	\$48,095	\$49,538	\$51,024	\$52,555	\$54,132	\$55,755	\$57,428	\$59,151	\$60,926	\$62,753	\$64,636	\$66,575	\$68,572	\$70,629	\$72,748	\$74,931	\$77,179	\$79,494
\$41,922	\$43,179	\$44,475	\$45,809	\$47,183	\$48,599	\$50,057	\$51,558	\$53,105	\$54,698	\$56,339	\$58,029	\$59,770	\$61,563	\$63,410	\$65,313	\$67,272	\$69,290	\$71,369	\$73,510	\$75,715	\$77,987	\$80,326	\$82,736	\$85,218	\$87,775	\$90,408
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal (Cont.)

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal Alternative 5, Design Flow Rate = 20 CFS

ASSUMPTIONS:									
Estimated Capita	al Cost:		\$2,636,000 Total Capit	tal Cost					
Interest on Repl	acement F	und:	3.00%						
Rate of Inflation	:		3.00%						
Project Design L	ife:		50 Years	50 Years					
			÷						
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³					
Estimated Proje	ct Replacer	ment Cost:							
To Replace	25%	After Life of Project		\$2,888,994					
To Replace	50%	After Life of Project		\$5,777,988					
To Replace	100%	After Life of Project	\$2,636,000	\$11,555,976					
Disposal and Re	moval Cost								
To Replace	25%	After Life of Project		\$120,557					
To Replace	50%	After Life of Project		\$241.115					

\$110,000

\$2,746,000

\$2,636,000

REPLACEMENT	FUND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$26,681	
To Replace	50%	After Life of Project	\$53,362	
To Replace	100%	After Life of Project	\$106,725	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$14,142	
To Replace	50%	After Life of Project	\$28,284	
To Replace	100%	After Life of Project	\$56,568	
Deposit Require	d at Year 2	5 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$28,748	
To Replace	50%	After Life of Project	\$57,495	
To Replace	100%	After Life of Project	\$114,990	
Deposit Require	d at Year 5	0 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$60,191	
To Replace	50%	After Life of Project	\$120,382	
To Replace	100%	After Life of Project	\$240,764	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 50th Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$686,500	\$966,000	\$644,832	\$2,297,332
50% Replacement	\$1,373,000	\$966,000	\$644,832	\$2,983,832
100% Replacement	\$2,746,000	\$966,000	\$644,832	\$4,356,832
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$686,500	\$966,000	\$766,839	\$2,419,339
50% Replacement	\$1,373,000	\$966,000	\$766,839	\$3,105,839
100% Replacement	\$2,746,000	\$966,000	\$766,839	\$4,478,839
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$686,500	\$966,000	\$888,845	\$2,541,345
50% Replacement	\$1,373,000	\$966,000	\$888,845	\$3,227,845
100% Replacement	\$2,746,000	\$966,000	\$888,845	\$4,600,845
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$686,500	\$966,000	\$1,010,852	\$2,663,352
50% Replacement	\$1,373,000	\$966,000	\$1,010,852	\$3,349,852
100% Replacement	\$2,746,000	\$966,000	\$1,010,852	\$4,722,852

LIFE	CYCLE	COSTS:
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_	LIFE CYCLE COSTS:																					
	Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

Total Replacement Cost:

Replacement Fund (For Funding Replacement of 25% of System):	
--------------------------------------------------------------	--

To Replace 100% After Life of Project

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

Deposits		\$14,142	\$14,566	\$15,003	\$15,453	\$15,917	\$16,394	\$16,886	\$17,393	\$17,915	\$18,452	\$19,006	\$19,576	\$20,163	\$20,768	\$21,391	\$22,033	\$22,694	\$23,374	\$24,076	ô
Interest		\$0	\$424	\$874	\$1,350	\$1,854	\$2,388	\$2,951	\$3,546	\$4,174	\$4,837	\$5,536	\$6,272	\$7,047	\$7,864	\$8,722	\$9,626	\$10,576	\$11,574	\$12,622	2
End of Year Balance		\$14,142	\$29,132	\$45,009	\$61,813	\$79,584	\$98,366	\$118,203	\$139,142	\$161,231	\$184,520	\$209,061	\$234,908	\$262,119	\$290,750	\$320,863	\$352,522	\$385,791	\$420,739	\$457,43	1
Replacement Fund (For Funding Replacement of 50	% of Syst	,																			
Deposits		\$28,284	\$29,132	\$30,006	\$30,906	\$31,834	\$32,789	\$33,772	\$34,786	\$35,829	\$36,904	\$38,011	\$39,151	\$40,326	\$41,536	\$42,782	\$44,065	\$45,387	\$46,749	\$48,153	
Interest		\$0	\$849	\$1,748	\$2,701	\$3,709	\$4,775	\$5,902	\$7,092	\$8,349	\$9,674	\$11,071	\$12,544	\$14,095	\$15,727	\$17,445	\$19,252	\$21,151	\$23,147	\$25,24	
End of Year Balance		\$28,284	\$58,265	\$90,019	\$123,626	\$159,168	\$196,732	\$236,406	\$278,284	\$322,462	\$369,039	\$418,122	\$469,817	\$524,237	\$581,500	\$641,727	\$705,044	\$771,582	\$841,479	\$914,874	ł
Replacement Fund (For Funding Replacement of 10	0% of Sys	stem):																			
Deposits		\$56,568	\$58,265	\$60,013	\$61,813	\$63,667	\$65,577	\$67,545	\$69,571	\$71,658	\$73,808	\$76,022	\$78,303	\$80,652	\$83,071	\$85,564	\$88,130	\$90,774	\$93,498	\$96,303	3
Interest		\$0	\$1,697	\$3,496	\$5,401	\$7,418	\$9,550	\$11,804	\$14,184	\$16,697	\$19,348	\$22,142	\$25,087	\$28,189	\$31,454	\$34,890	\$38,504	\$42,303	\$46,295	\$50,489	Э
End of Year Balance		\$56,568	\$116,529	\$180,038	\$247,252	\$318,337	\$393,464	\$472,813	\$556,568	\$644,923	\$738,079	\$836,243	\$939,633	\$1,048,474	\$1,163,000	\$1,283,454	\$1,410,088	\$1,543,165	\$1,682,957	\$1,829,748	8
Operations and Maintenance Expenses:																					
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	3
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,24	7
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$85:	1
Maintenance and Small Repairs ⁷		\$13,200	\$13,596	\$14,004	\$14,424	\$14,857	\$15,302	\$15,761	\$16,234	\$16,721	\$17,223	\$17,740	\$18,272	\$18,820	\$19,385	\$19,966	\$20,565	\$21,182	\$21,818	\$22,472	2
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,70	
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
Total O&M Expenses		\$19,320	\$19,900	\$20,497	\$21,111	\$21,745	\$22,397	\$23,069	\$23,761	\$24,474	\$25,208	\$25,964	\$26,743	\$27,546	\$28,372	\$29,223	\$30,100	\$31,003	\$31,933	\$32,893	L
Pumping Power Costs:																					
2-Week Annual Pumping Duration ⁸		\$12,897	\$13,284	\$13,682	\$14,093	\$14,515	\$14,951	\$15,399	\$15,861	\$16,337	\$16,827	\$17,332	\$17,852	\$18,388	\$18,939	\$19,507	\$20,093	\$20,695	\$21,316	\$21,956	6
4-Week Annual Pumping Duration ⁸		\$15.337	\$15.797	\$16.271	\$16.759	\$17.262	\$17.780	\$18.313	\$18.862	\$19.428	\$20,011	\$20.611	\$21.230	\$21.867	\$22.523	\$23,198	\$23.894	\$24.611	\$25,349	\$26,110	
6-Week Annual Pumping Duration ⁸		\$17,777	\$18,310	\$18,860	\$19,425	\$20,008	\$20,608	\$21,227	\$21,863	\$22,519	\$23,195	\$23,891	\$24,607	\$25,346	\$26,106	\$26,889	\$27,696	\$28,527	\$29,383	\$30,264	
8-Week Annual Pumping Duration ⁸		\$20,217	\$20,824	\$21,448	\$22,092	\$22,754	\$23,437	\$24,140	\$24,864	\$25,610	\$26,379	\$27,170	\$27,985	\$28,825	\$29,689	\$30,580	\$31,497	\$32,442	\$33,416	\$34,418	
		<i>420,217</i>	<i>\$20,024</i>	<i>721,440</i>	<i>\$22,032</i>	<i>422,1</i> 34	<i>423,437</i>	<i>724,140</i>	<i>724,004</i>	<i>423,010</i>	<i>4</i> 20,373	<i>4</i> 27,170	<i>421,303</i>	<i>420,023</i>	<i>425,005</i>	<i>4</i> 30,380	<i>Ş</i> 31, 4 37	<i>~J</i> 2, 4 42	<i>Ş</i> 33,410	<i>4</i> 34 , 41	,
Year		1	2	2	4	r	c	7		0	10	11	12	13	14	15	16	17	18	19	Т
Year 0		1	2	3	4	5	ъ	'	8	Э	10	11	12	13	14	15	16	1/	18	19	1

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$482,230

\$3,009,551

\$6,019,103

\$12,038,206

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
\$24,798	\$25,542	\$26,308	\$27,097
\$13,723	\$14,879	\$16,091	\$17,363
\$495,958	\$536,379	\$578,778	\$623,239
4 40 5 00	654.004	650.040	÷= • • • • =
\$49,596	\$51,084	\$52,616	\$54,195
\$27,446 \$991,916	\$29,757 \$1,072,757	\$32,183 \$1,157,556	\$34,727 \$1,246,478
<i>3331,310</i>	\$1,072,737	\$1,157,550	<i>31,240,478</i>
\$99,192	\$102,167	\$105,232	\$108,389
\$54,892	\$59,515	\$64,365	\$69,453
\$1,983,833	\$2,145,515	\$2,315,113	\$2,492,956
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$23,146	\$23,841	\$24,556	\$25,293
\$1,754	\$1,806	\$1,860	\$1,916
\$0	\$0	\$0	\$0
\$33,878	\$34,894	\$35,941	\$37,019
\$22,614	\$23,293	\$23,992	\$24,711
\$26,893	\$27,700	\$28,531	\$29,387
\$31,172	\$32,107	\$33,070	\$34,062
\$35,451	\$36,514	\$37,610	\$38,738
20	21	22	23

24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46
\$39,900	\$41,097	\$42,330	\$43,600	\$44,908	\$46,255	\$47,643	\$49,072	\$50,544	\$52,061	\$53,622	\$55,231	\$56,888	\$58,595	\$60,352	\$62,163	\$64,028	\$65,949	\$67,927	\$69,965	\$72,064	\$74,226	\$76,453
\$35,084	\$36,137	\$37,221	\$38,338	\$39,488	\$40,672	\$41,892	\$43,149	\$44,444	\$45,777	\$47,150	\$48,565	\$50,022	\$51,522	\$53,068	\$54,660	\$56,300	\$57,989	\$59,729	\$61,520	\$63,366	\$65,267	\$67,225
\$25,453 \$30,268	\$26,216 \$31,177	\$27,003 \$32,112	\$27,813 \$33,075	\$28,647 \$34,067	\$29,507 \$35,089	\$30,392 \$36,142	\$31,304 \$37,226	\$32,243 \$38,343	\$33,210 \$39,493	\$34,206 \$40,678	\$35,232 \$41,899	\$36,289 \$43,156	\$37,378 \$44,450	\$38,499 \$45,784	\$39,654 \$47,157	\$40,844 \$48,572	\$42,069 \$50,029	\$43,331 \$51,530	\$44,631 \$53,076	\$45,970 \$54,668	\$47,349 \$56,308	\$48,770 \$57,997
\$38,130	\$39,274	\$40,452	\$41,665	\$42,915	\$44,203	\$45,529	\$46,895	\$48,302	\$49,751	\$51,243	\$52,780	\$54,364	\$55,995	\$57,675	\$59,405	\$61,187	\$63,023	\$64,913	\$66,861	\$68,866	\$70,932	\$73,060
\$0	\$2,055 \$0	\$0	\$0	\$0	\$0	\$2,557 \$0	\$2,427 \$0	\$2,500 \$0	\$2,575 \$0	\$2,052 \$0	\$2,732 \$0	\$2,014 \$0	\$0	\$2,585 \$0	\$0,075	\$0	\$3,202 \$0	\$0,500	\$0,401	\$3,585 \$0	\$0,071	\$0,782
\$26,051 \$1,974	\$26,833 \$2,033	\$27,638 \$2,094	\$28,467 \$2,157	\$29,321 \$2,221	\$30,201 \$2,288	\$31,107 \$2,357	\$32,040 \$2,427	\$33,001 \$2,500	\$33,991 \$2,575	\$35,011 \$2,652	\$36,061 \$2,732	\$37,143 \$2,814	\$38,257 \$2,898	\$39,405 \$2,985	\$40,587 \$3,075	\$41,805 \$3,167	\$43,059 \$3,262	\$44,351 \$3,360	\$45,681 \$3,461	\$47,052 \$3,565	\$48,463 \$3,671	\$49,917 \$3,782
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479
2,679,385	\$2,874,757	\$3,079,440	\$3,293,816	\$3,518,284	\$3,753,255	\$3,999,158	\$4,256,437	\$4,525,553	\$4,806,986	\$5,101,232	\$5,408,806	\$5,730,244	\$6,066,100	\$6,416,950	\$6,783,392	\$7,166,045	\$7,565,552	\$7,982,580	\$8,417,820	\$8,871,991	\$9,345,836	\$9,840,127
\$74,789	\$80,382	\$86,243	\$92,383	\$98,814	\$105,549	\$112,598	\$119,975	\$127,693	\$135,767	\$130,030 \$144,210	\$153,037 \$153,037	\$162,264	\$171,907	\$181,983	\$192,509	\$203,502	\$214,981	\$226,967	\$239,477	\$252,535	\$266,160	\$280,375
\$111,641	\$114,990	\$118,440	\$121,993	\$125,653	\$129,423	\$133,305	\$137,304	\$141,424	\$145,666	\$150,036	\$154,537	\$159,173	\$163,949	\$168,867	\$173,933	\$179,151	\$184,526	\$190,061	\$195,763	\$201,636	\$207,685	\$213,916
1,339,693	\$1,437,379	\$1,539,720	\$1,646,908	\$1,759,142	\$1,876,627	\$1,999,579	\$2,128,218	\$2,262,777	\$2,403,493	\$2,550,616	\$2,704,403	\$2,865,122	\$3,033,050	\$3,208,475	\$3,391,696	\$3,583,022	\$3,782,776	\$3,991,290	\$4,208,910	\$4,435,995	\$4,672,918	\$4,920,063
\$55,821 \$37,394	\$57,495 \$40,191	\$59,220 \$43,121	\$60,997 \$46,192	\$62,826 \$49,407	\$64,711 \$52,774	\$66,653 \$56,299	\$68,652 \$59,987	\$70,712 \$63,847	\$72,833 \$67,883	\$75,018 \$72,105	\$77,269 \$76,518	\$79,587 \$81,132	\$81,974 \$85,954	\$84,434 \$90,992	\$86,967 \$96,254	\$89,576 \$101,751	\$92,263 \$107,491	\$95,031 \$113,483	\$97,882 \$119,739	\$100,818 \$126,267	\$103,843 \$133,080	\$106,958 \$140,188
\$669,846	\$718,689	\$769,860	\$823,454	\$879,571	\$938,314	\$999,789	\$1,064,109	\$1,131,388	\$1,201,747	\$1,275,308	\$1,352,202	\$1,432,561	\$1,516,525	\$1,604,238	\$1,695,848	\$1,791,511	\$1,891,388	\$1,995,645	\$2,104,455	\$2,217,998	\$2,336,459	\$2,460,032
\$18,697	\$20,095	\$21,561	\$23,096	\$24,704	\$26,387	\$28,149	\$29,994	\$31,923	\$33,942	\$36,052	\$38,259	\$40,566	\$42,977	\$45,496	\$48,127	\$50,875	\$53,745	\$56,742	\$59,869	\$63,134	\$66,540	\$70,094
\$27,910	\$28,748	\$29,610	\$30,498	\$31,413	\$32,356	\$33,326	\$34,326	\$35,356	\$36,417	\$37,509	\$38,634	\$39,793	\$40,987	\$42,217	\$43,483	\$44,788	\$46,131	\$47,515	\$48,941	\$50,409	\$51,921	\$53,479

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal (Cont.)

47	48	49	50
\$55,083	\$56,736	\$58,438	\$60,191
\$73,801	\$77,667	\$81,700	\$85,904
\$2,588,916	\$2,723,319	\$2,863,457	\$3,009,551
\$110,167	\$113,472	\$116,876	\$120,382
\$147,602	\$155,335	\$163,399	\$171,807
\$5,177,832	\$5,446,639	\$5,726,913	\$6,019,103

\$220,333	\$226,943	\$233,752	\$240,764
\$295,204 \$10,355,664	\$310,670 \$10,893,277	\$326,798 \$11,453,827	\$343,615 \$12,038,206
Ş10,333,00 4	\$10,053,277	Ş11, 4 33,627	<i>Ş12,038,200</i>
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$51,415	\$52,957	\$54,546	\$56,182
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$75,252	\$77,510	\$79,835	\$82,230
\$50,233	\$51,740	\$53,292	\$54,891
\$59,737	\$61,530	\$63,375	\$65,277
\$69,242	\$71,319	\$73,459	\$75,662
\$78,746	\$81,109	\$83,542	\$86,048
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal Alternative 1, Design Flow Rate = 40 CFS

ASSUMPTIONS:				
Estimated Capita	al Cost:		\$3,334,000 Total Capita	al Cost
Interest on Repla	acement F	und:	3.00%	
Rate of Inflation	:		3.00%	
Project Design Li	ife:		50 Years	
SUMMARY REPI	LACEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Project	ct Replacer	ment Cost:		
To Replace	25%	After Life of Project		\$3,653,986
To Replace	50%	After Life of Project		\$7,307,971
To Replace	100%	After Life of Project	\$3,334,000	\$14,615,943
Disposal and Rei	moval Cost	:		
To Replace	25%	After Life of Project		\$107,406
To Replace	50%	After Life of Project		\$214,811

\$98,000

\$3,432,000

UND SUM	IMARY		
Required (<i>I</i>	Assume Equal Deposit Made	Each Year):	
25%	After Life of Project	\$33,347	
50%	After Life of Project	\$66,693	
100%	After Life of Project	\$133,386	
d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
25%	After Life of Project	\$17,675	
50%	After Life of Project	\$35,350	
100%	After Life of Project	\$70,699	
d at Year 2	5 (Assume Deposits Increase	e at the Rate of Inflation):	
25%	After Life of Project	\$35,929	
50%	After Life of Project	\$71,858	
100%	After Life of Project	\$143,717	
d at Year 5	0 (Assume Deposits Increase	at the Rate of Inflation):	
25%	After Life of Project	\$75,228	
50%	After Life of Project	\$150,456	
100%	After Life of Project	\$300,911	
	Required (<i>i</i> 25% 50% 100% d at Year 1 25% 50% 100% d at Year 2 25% 50% 100% d at Year 5 25% 50%	25% After Life of Project 50% After Life of Project 100% After Life of Project dat Year 1 (Assume Deposits Increase 25% After Life of Project 50% After Life of Project 50% After Life of Project 100% After Life of Project 100% After Life of Project 100% After Life of Project 50% After Life of Project 50% After Life of Project 50% After Life of Project 100% After Life of Project 50% After Life of Project 25% After Life of Project 50% After Life of Project	Required (Assume Equal Deposit Made Each Year): 25% After Life of Project \$33,347 50% After Life of Project \$66,693 100% After Life of Project \$133,386 dat Year 1 (Assume Deposits Increase at the Rate of Inflation): 25% After Life of Project \$17,675 50% After Life of Project \$35,350 100% After Life of Project \$35,350 100% After Life of Project \$70,699 44 Year 25 (Assume Deposits Increase at the Rate of Inflation): 25% After Life of Project \$35,929 50% After Life of Project \$71,858 100% After Life of Project \$143,717 4147,717 4147,717 dat Year 50 (Assume Deposits Increase at the Rate of Inflation): 25% After Life of Project \$75,228 50% After Life of Project \$75,228 50% After Life of Project \$75,228

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 50th Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 &M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$858,000	\$891,000	\$1,416,454	\$3,165,454
50% Replacement	\$1,716,000	\$891,000	\$1,416,454	\$4,023,454
100% Replacement	\$3,432,000	\$891,000	\$1,416,454	\$5,739,454
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$858,000	\$891,000	\$1,685,282	\$3,434,282
50% Replacement	\$1,716,000	\$891,000	\$1,685,282	\$4,292,282
100% Replacement	\$3,432,000	\$891,000	\$1,685,282	\$6,008,282
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$858,000	\$891,000	\$1,954,111	\$3,703,111
50% Replacement	\$1,716,000	\$891,000	\$1,954,111	\$4,561,111
100% Replacement	\$3,432,000	\$891,000	\$1,954,111	\$6,277,111
Assuming the Pumping Power Costs for an 8	3-week Annual	Operating Dura	ation:	
25% Replacement	\$858,000	\$891,000	\$2,222,939	\$3,971,939
50% Replacement	\$1,716,000	\$891,000	\$2,222,939	\$4,829,939
100% Replacement	\$3,432,000	\$891,000	\$2,222,939	\$6,545,939

LIFE CYCLE COSTS:

Total Replacement Cost:

LIFE CICLE COSIS:																					
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

To Replace 100% After Life of Project

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

LIFE CYCLE COSTS:																								
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
Capital Expenses:	\$3,334,000																							
Replacement Fund (For Funding Replacemer	nt of 25% of Syst	em):																						
Deposits		\$17,675	\$18,205	\$18,751	\$19,314	\$19,893	\$20,490	\$21,105	\$21,738	\$22,390	\$23,062	\$23,753	\$24,466	\$25,200	\$25,956	\$26,735	\$27,537	\$28,363	\$29,214	\$30,090	\$30,993	\$31,923	\$32,880	\$33,867
Interest		\$0	\$530	\$1,092	\$1,688	\$2,318	\$2,984	\$3,688	\$4,432	\$5,217	\$6,045	\$6,918	\$7,839	\$8,808	\$9,828	\$10,902	\$12,031	\$13,218	\$14,465	\$15,775	\$17,151	\$18,596	\$20,111	\$21,701
End of Year Balance		\$17,675	\$36,410	\$56,254	\$77,255	\$99,466	\$122,940	\$147,732	\$173,902	\$201,509	\$230,616	\$261,288	\$293,593	\$327,601	\$363,385	\$401,021	\$440,588	\$482,169	\$525,848	\$571,713	\$619,857	\$670,376	\$723,367	\$778,935
Replacement Fund (For Funding Replacemer	nt of 50% of Syst	em):																						
Deposits		\$35,350	\$36,410	\$37,502	\$38,627	\$39,786	\$40,980	\$42,209	\$43,476	\$44,780	\$46,123	\$47,507	\$48,932	\$50,400	\$51,912	\$53,469	\$55,074	\$56,726	\$58,428	\$60,180	\$61,986	\$63,845	\$65,761	\$67,733
Interest		\$0	\$1,060	\$2,185	\$3,375	\$4,635	\$5,968	\$7,376	\$8,864	\$10,434	\$12,091	\$13,837	\$15,677	\$17,616	\$19,656	\$21,803	\$24,061	\$26,435	\$28,930	\$31,551	\$34,303	\$37,191	\$40,223	\$43,402
End of Year Balance		\$35,350	\$72,820	\$112,507	\$154,510	\$198,931	\$245,879	\$295,465	\$347,804	\$403,018	\$461,232	\$522,576	\$587,185	\$655,201	\$726,769	\$802,042	\$881,176	\$964,337	\$1,051,695	\$1,143,426	\$1,239,715	\$1,340,752	\$1,446,735	\$1,557,870
Replacement Fund (For Funding Replacemer	nt of 100% of Sy	stem):																						
Deposits		\$70,699	\$72,820	\$75,005	\$77,255	\$79,573	\$81,960	\$84,419	\$86,951	\$89,560	\$92,246	\$95,014	\$97,864	\$100,800	\$103,824	\$106,939	\$110,147	\$113,451	\$116,855	\$120,361	\$123,971	\$127,691	\$131,521	\$135,467
Interest		\$0	\$2,121	\$4,369	\$6,750	\$9,271	\$11,936	\$14,753	\$17,728	\$20,868	\$24,181	\$27,674	\$31,355	\$35,231	\$39,312	\$43,606	\$48,122	\$52,871	\$57,860	\$63,102	\$68,606	\$74,383	\$80,445	\$86,804
End of Year Balance		\$70,699	\$145,640	\$225,014	\$309,020	\$397,863	\$491,758	\$590,930	\$695,609	\$806,037	\$922,464	\$1,045,152	\$1,174,371	\$1,310,402	\$1,453,538	\$1,604,083	\$1,762,353	\$1,928,675	\$2,103,390	\$2,286,852	\$2,479,430	\$2,681,503	\$2,893,469	\$3,115,740
Operations and Maintenance Expenses:																								
Salaries (1/12 FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,618	\$5,787	\$5,960	\$6,139	\$6,323
Benefits ⁵		\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,247	\$2,315	\$2,384	\$2,456	\$2,529
Transportation Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$851	\$877	\$903	\$930	\$958
Maintenance and Small Repairs ⁷		\$11,700	\$12,051	\$12,413	\$12,785	\$13,168	\$13,564	\$13,970	\$14,390	\$14,821	\$15,266	\$15,724	\$16,196	\$16,681	\$17,182	\$17,697	\$18,228	\$18,775	\$19,338	\$19,918	\$20,516	\$21,132	\$21,765	\$22,418
Administration, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,702	\$1,754	\$1,806	\$1,860	\$1,916
Other		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
Total O&M Expenses		\$17,820	\$18,355	\$18,905	\$19,472	\$20,057	\$20,658	\$21,278	\$21,916	\$22,574	\$23,251	\$23,949	\$24,667	\$25,407	\$26,169	\$26,954	\$27,763	\$28,596	\$29,454	\$30,337	\$31,247	\$32,185	\$33,150	\$34,145
Pumping Power Costs:																								
2-Week Annual Pumping Duration ⁸		\$28,329	\$29,179	\$30,054	\$30,956	\$31,885	\$32,841	\$33,826	\$34,841	\$35,886	\$36,963	\$38,072	\$39,214	\$40,390	\$41,602	\$42,850	\$44,136	\$45,460	\$46,824	\$48,228	\$49,675	\$51,165	\$52,700	\$54,281
4-Week Annual Pumping Duration ⁸		\$33,706	\$34,717	\$35,758	\$36,831	\$37,936	\$39,074	\$40,246	\$41,454	\$42,697	\$43,978	\$45,298	\$46,656	\$48,056	\$49,498	\$50,983	\$52,512	\$54,088	\$55,710	\$57,382	\$59,103	\$60,876	\$62,702	\$64,583
6-Week Annual Pumping Duration ⁸		\$39,082	\$40,255	\$41,462	\$42,706	\$43,987	\$45,307	\$46,666	\$48,066	\$49,508	\$50,993	\$52,523	\$54,099	\$55,722	\$57,394	\$59,115	\$60,889	\$62,715	\$64,597	\$66,535	\$68,531	\$70,587	\$72,704	\$74,886
8-Week Annual Pumping Duration ⁸		\$44,459		\$47,166	\$48,581	\$50,039	\$51,540	\$53,086	\$54,679	\$56,319	\$58,009	\$59,749	\$61,541	\$63,388	\$65,289	\$67,248	\$69,265	\$71,343	\$73,484	\$75,688	\$77,959	\$80,298	\$82,706	\$85,188
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23

NOTES:

1) Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.

2) Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$429,623

\$3,761,391

\$7,522,783

\$15,045,565

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

Ş54,005	<i>\$33,525</i>	\$57,007	<i>\$50,117</i>	<i>\$55,201</i>	Ş40,435	941,052	<u>7</u> +2,501	Ş44,100	945,514	Ş40,075	Ş40,200	Ş45,754	<i>\$51,220</i>	<i>\$52,705</i>	Ş54,540	255,577	<i>\$57,050</i>	255,500	<i>\$01,107</i>	\$05,002	90 4 ,052	\$00,055	
\$23,368	\$25,116	\$26,947	\$28,866	\$30,875	\$32,979	\$35,182	\$37,487	\$39,898	\$42,421	\$45,059	\$47,817	\$50,700	\$53,713	\$56,861	\$60,150	\$63,585	\$67,172	\$70,917	\$74,826	\$78,906	\$83,163	\$87,604	
\$837,186	\$898,231	\$962,185	\$1,029,168	\$1,099,303	\$1,172,721	\$1,249,555	\$1,329,943	\$1,414,029	\$1,501,964	\$1,593,903	\$1,690,006	\$1,790,440	\$1,895,380	\$2,005,005	\$2,119,501	\$2,239,063	\$2,363,890	\$2,494,193	\$2,630,186	\$2,772,093	\$2,920,148	\$3,074,592	\$
\$69,765	\$71,858	\$74,014	\$76,235	\$78,522	\$80,877	\$83,304	\$85,803	\$88,377	\$91,028	\$93,759	\$96,572	\$99,469	\$102,453	\$105,527	\$108,692	\$111,953	\$115,312	\$118,771	\$122,334	\$126,004	\$129,784	\$133,678	
\$46,736	\$50,231	\$53,894	\$57,731	\$61,750	\$65,958	\$70,363	\$74,973	\$79,797	\$84,842	\$90,118	\$95,634	\$101,400	\$107,426	\$113,723	\$120,300	\$127,170	\$134,344	\$141.833	\$149,652	\$157,811	\$166,326	\$175,209	
\$1.674.372	\$1.796.461	\$1.924.370	\$2,058,335	\$2,198,607	\$2.345.443	\$2,499,109	\$2,659,885	\$2,828,059	\$3,003,929	. ,	\$3,380,012		\$3,790,760	\$4,010,010	. ,	\$4,478,125	\$4,727,781	\$4.988.385	\$5,260,371	\$5,544,187		\$6,149,183	
<i>,,,,,,,,</i> ,,,,	+_,,.	+_, ,	+_,,	+_,,	<i>,_,_,_,</i>	+-,,	+_,,	+-,,	+-,,	<i></i>	+-,,	+-,,	<i></i>	+ .,,	+ ,,	+ ,,,	+ .,. = . ,. = =	+ ,,	+-,,	<i>,.,.</i> ,.,,	<i>,,,,,,,,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,	+-,,	•
\$139,531	\$143,717	\$148,028	\$152,469	\$157,043	\$161,755	\$166,607	\$171,606	\$176,754	\$182,056	\$187,518	\$193,144	\$198,938	\$204,906	\$211,053	\$217,385	\$223,906	\$230,623	\$237,542	\$244,668	\$252,008	\$259,569	\$267,356	
\$93,472	\$100,462	\$107,788	\$115,462	\$123,500	\$131,916	\$140,727	\$149,947	\$159,593	\$169,684	\$180,236	\$191,268	\$202,801	\$214,853	\$227,446	\$240,601	\$254,340	\$268,688	\$283,667	\$299,303	\$315,622	\$332,651	\$350,418	
\$3,348,744	\$3,592,923	\$3,848,739	\$4,116,670	\$4,397,214	\$4,690,885	\$4,998,219	\$5,319,771	\$5,656,118	\$6,007,858	\$6,375,611	\$6,760,023	\$7,161,762	\$7,581,520	\$8,020,019	\$8,478,005	\$8,956,251	\$9,455,562	\$9,976,771	\$10,520,743	\$11,088,373	\$11,680,593	\$12,298,367	\$1
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	
\$2,605	\$2,683	\$2,764	\$2,847	\$2,932	\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836		
\$23,091	\$23,784	\$24,497	\$25,232	\$25,989	\$26,769	\$27,572	\$28,399	\$29,251	\$30,128	\$31,032	\$31,963	\$32,922	\$33,910	\$34,927	\$35,975	\$37,054	\$38,166	\$39,311	\$40,490	\$41,705	\$42,956	\$44,245	
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$2,42, \$0	\$0 \$0	\$0	\$2,052	\$0	\$0	\$2,050	\$0	\$0	\$0,107	\$0	\$3,500 \$0	\$0,401 \$0	\$0,505	\$0	\$0	
\$35,169	\$36,224	\$37,311	\$38,430	\$39,583	\$40,771	\$41,994	\$43,254	\$44,551	\$45,888	\$47,265	\$48,683	\$50,143	\$51,647	\$53,197	\$54,793	\$56,436	\$58,130	\$59,873	\$61.670	\$63,520	\$65,425		
\$55,105	Ş30,224	<i>\$57,</i> 511	J J0,4J0	<i>333,303</i>	<i>\$</i> 4 0,771	Ş41,554	J+J,2J+	,JJJ1	Ş 4 3,888	Ş47,203	J-0,003	Ş50,145	Ş J1, 047	<i>\$</i> 33 ,1 <i>3</i> 7	ŞJ 4 ,755	\$50,430	\$56,150	<i>433,613</i>	J 01,070	303,320	303,423	<i>907,</i> 3 00	
\$55,910	\$57,587	\$59,315	\$61,094	\$62,927	\$64,815	\$66,759	\$68,762	\$70,825	\$72,950	\$75,138	\$77,392	\$79,714	\$82,106	\$84,569	\$87,106	\$89,719	\$92,411	\$95,183	\$98,038	\$100,979	\$104,009	\$107,129	
\$66,521	\$68,517	\$70,572	\$72,689	\$74,870	\$77,116	\$79,430	\$81,812	\$84,267	\$86,795	\$89,399	\$92,081	\$94,843	\$97,688	\$100,619	\$103,638	\$106,747	\$109,949	\$113,248	\$116,645	\$120,144	\$123,749	\$127,461	
\$77,132	\$79,446	\$81,829	\$84,284	\$86,813	\$89,417	\$92,100	\$94,863	\$97,709	\$100,640	\$103,659	\$106,769	\$109,972	\$113,271	\$116,669	\$120,169	\$123,774	\$127,488	\$131,312	\$135,252	\$139,309	\$143.488	\$147,793	
					. ,												. ,		. ,		,		
\$87,743	\$90,376	\$93,087	\$95,879	\$98,756	\$101,718	\$104,770	\$107,913	\$111,151	\$114,485	\$117,920	\$121,457	\$125,101	\$128,854	\$132,720	\$136,701	\$140,802	\$145,026	\$149,377	\$153,858	\$158,474	\$163,228	\$168,125	
		1				I				I				I		<u> </u>					r		_
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	1
																							1

\$34,883 \$35,929 \$37,007 \$38,117 \$39,261 \$40,439 \$41,652 \$42,901 \$44,188 \$45,514 \$46,879 \$48,286 \$49,734 \$51,226 \$52,763 \$54,346 \$55,977 \$57,656 \$59,386 \$61,167 \$63,002 \$64,892 \$66,839

47	48	49	50
\$68,844	\$70,909	\$73,037	\$75,228
\$92,238	\$97,070	\$102,110	\$107,364
\$3,235,674	\$3,403,653	\$3,578,800	\$3,761,391
\$137,688	\$141,819	\$146,073	\$150,456
\$184,476	\$194,140	\$204,219	\$214,728
\$6,471,347	\$6,807,306	\$7,157,599	\$7,522,783
\$275,376	\$283,638	\$292,147	\$300,911
\$368,951	\$388,281	\$408,438	\$429,456
\$12,942,694	\$13,614,613	\$14,315,198	\$15,045,565
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$45,572	\$46,939	\$48,347	\$49,798
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$69,410	\$71,492	\$73,637	\$75,846
\$110,343	\$113,653	\$117,063	\$120,575
\$131,285	\$135,223	\$139,280	\$143,459
\$152,227	\$156,794	\$161,498	\$166,342
\$173,169	\$178,364	\$183,715	\$189,226
47	48	49	50

Peshastin Irrigation District Pump Exchange Appraisal Study Life Cycle Cost Analysis - Pumping From PID Canal to IID Division 3A Canal Alternative 5, Design Flow Rate = 40 CFS

ASSUMPTIONS:				
Estimated Capital	Cost:		\$4,295,000 Total Cap	ital Cost
Interest on Replace	ement Fi	und:	3.00%	
Rate of Inflation:			3.00%	
Project Design Life	:		50 Years	
-				
SUMMARY REPLAC	CEMENT	COSTS:	CURRENT COST ²	FUTURE COST ³
Estimated Project I	Replacer	ment Cost:		
To Replace	25%	After Life of Project		\$4,707,219
To Replace	50%	After Life of Project		\$9,414,438
To Replace	100%	After Life of Project	\$4,295,000	\$18,828,876
Disposal and Remo	oval Cost	:		
To Replace	25%	After Life of Project		\$168,780
To Replace	50%	After Life of Project		\$337,561
To Replace	100%	After Life of Project	\$154.000	\$675,122

\$4,449,000

\$4,295,000

REPLACEMENT I	UND SUM	IMARY		
Annual Deposit	Required (Assume Equal Deposit Made	Each Year):	
To Replace	25%	After Life of Project	\$43,228	
To Replace	50%	After Life of Project	\$86,456	
To Replace	100%	After Life of Project	\$172,913	
Deposit Require	d at Year 1	(Assume Deposits Increase	at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$22,912	
To Replace	50%	After Life of Project	\$45,825	
To Replace	100%	After Life of Project	\$91,649	
Deposit Require	d at Year 2	5 (Assume Deposits Increas	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$46,576	
To Replace	50%	After Life of Project	\$93,152	
To Replace	100%	After Life of Project	\$186,304	
Deposit Require	d at Year 5	0 (Assume Deposits Increase	e at the Rate of Inflation):	
To Replace	25%	After Life of Project	\$97,520	
To Replace	50%	After Life of Project	\$195,040	
To Replace	100%	After Life of Project	\$390,080	

Input Cells - Assumed or Given Values Input Cells - Adjust Using Goal Seek Tool to Make Account Balance at end of 5oth Year Equal to Future Value of Replacement Cost

TOTAL LONG-TERM COST SUMMARY:				
(PRESENT VALUE OF LONG-TERM	Replacment			
COSTS THROUGH 50-YEAR LIFE CYCLE)	Fund	0 & M	Power	TOTAL
Assuming the Pumping Power Costs for a 2-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,112,250	\$1,231,000	\$1,351,246	\$3,694,496
50% Replacement	\$2,224,500	\$1,231,000	\$1,351,246	\$4,806,746
100% Replacement	\$4,449,000	\$1,231,000	\$1,351,246	\$7,031,246
Assuming the Pumping Power Costs for a 4-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,112,250	\$1,231,000	\$1,607,667	\$3,950,917
50% Replacement	\$2,224,500	\$1,231,000	\$1,607,667	\$5,063,167
100% Replacement	\$4,449,000	\$1,231,000	\$1,607,667	\$7,287,667
Assuming the Pumping Power Costs for a 6-	week Annual O	perating Durat	ion:	
25% Replacement	\$1,112,250	\$1,231,000	\$1,864,088	\$4,207,338
50% Replacement	\$2,224,500	\$1,231,000	\$1,864,088	\$5,319,588
100% Replacement	\$4,449,000	\$1,231,000	\$1,864,088	\$7,544,088
Assuming the Pumping Power Costs for an 8	-week Annual	Operating Dura	ation:	
25% Replacement	\$1,112,250	\$1,231,000	\$2,120,509	\$4,463,759
50% Replacement	\$2,224,500	\$1,231,000	\$2,120,509	\$5,576,009
100% Replacement	\$4,449,000	\$1,231,000	\$2,120,509	\$7,800,509

LIFE	CYCLE	COSTS:

Total Replacement Cost:

To Replace 25% After Life of Project

To Replace 50% After Life of Project

To Replace 100% After Life of Project

LIFE CYCLE COSTS:																					
Year	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

Capital Expenses:

Deposits			\$22,912	\$23,600	\$24,308	\$25,037	\$25,788	\$26,562	\$27,359	\$28,179	\$29,025	\$29,895	\$30,792	\$31,716	\$32,668	\$33,648	\$34,657	\$35,697	\$36,768	\$37,871	\$39,00	7
Interest			\$0	\$687	\$1,416	\$2,188	\$3,004	\$3,868	\$4,781	\$5,745	\$6,763	\$7,837	\$8,969	\$10,161	\$11,418	\$12,740	\$14,132	\$15,596	\$17,134	\$18,751	\$20,45	J
End of Year Ba	alance		\$22,912	\$47,199	\$72,923	\$100,148	\$128,940	\$159,370	\$191,510	\$225,434	\$261,222	\$298,954	\$338,715	\$380,593	\$424,678	\$471,066	\$519,855	\$571,147	\$625 <i>,</i> 049	\$681,671	\$741,12	B
Replacement Fu	und (For Funding Replaceme	ent of 50% of Sy																				
Deposits			\$45,825	\$47,199	\$48,615	\$50,074	\$51,576	\$53,123	\$54,717	\$56,359	\$58,049	\$59,791	\$61,585	\$63,432	\$65,335	\$67,295	\$69,314	\$71,393	\$73,535	\$75,741	\$78,01	
Interest			\$0		\$2,832	\$4,375	\$6,009	\$7,736	\$9,562	\$11,491	\$13,526	\$15,673	\$17,937	\$20,323	\$22,836	\$25,481	\$28,264	\$31,191	\$34,269	\$37,503	\$40,90	
End of Year Ba	alance		\$45,825	\$94,399	\$145,846	\$200,296	\$257,881	\$318,740	\$383,020	\$450,869	\$522,444	\$597,908	\$677,430	\$761,185	\$849,356	\$942,132	\$1,039,710	\$1,142,294	\$1,250,098	\$1,363,342	\$1,482,25	5
Replacement Fu	und (For Funding Replaceme	ont of 100% of	System):																			
Deposits	and (For Funding Replaceme		\$91,649	\$94,399	\$97,231	\$100,148	\$103,152	\$106,247	\$109,434	\$112,717	\$116.099	\$119,582	\$123,169	\$126,864	\$130,670	\$134.590	\$138,628	\$142.787	\$147.070	\$151,482	\$156,02	7
Interest			\$0		\$5,664	\$8,751	\$12,018	\$15,473	\$19,124	\$22,981	\$27.052	\$31,347	\$35,875	\$40.646	\$45,671	\$50,961	\$56,528	\$62,383	\$68,538	\$75,006	\$81,80	
End of Year Ba	alance		\$91.649	\$188.798	\$291,693	\$400,591	\$515,761	\$637,481	\$766,039	\$901.738	\$1.044.888	\$1,195,817	\$1.354.860	\$1.522.370	\$1.698.712	\$1.884.263	\$2.079.419	\$2.284.588	\$2.500.196	\$2,726,685	\$2,964,51	
								,			. ,. ,	. , , .						. , . ,		., ,		
Operations and	Maintenance Expenses:																					
Salaries (1/12	FTE) ⁴		\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	\$5,141	\$5,296	\$5,454	\$5,61	8
Benefits ⁵			\$1,320	\$1,360	\$1,400	\$1,442	\$1,486	\$1,530	\$1,576	\$1,623	\$1,672	\$1,722	\$1,774	\$1,827	\$1,882	\$1,938	\$1,997	\$2,057	\$2,118	\$2,182	\$2,24	7
Transportation	n Costs ⁶		\$500	\$515	\$530	\$546	\$563	\$580	\$597	\$615	\$633	\$652	\$672	\$692	\$713	\$734	\$756	\$779	\$802	\$826	\$85	1
Maintenance a	and Small Repairs ⁷		\$18,500	\$19,055	\$19,627	\$20,215	\$20,822	\$21,447	\$22,090	\$22,753	\$23,435	\$24,138	\$24,862	\$25,608	\$26,377	\$27,168	\$27,983	\$28,822	\$29,687	\$30,578	\$31,49	5
Administratior	n, Insurance, Accounting		\$1,000	\$1,030	\$1,061	\$1,093	\$1,126	\$1,159	\$1,194	\$1,230	\$1,267	\$1,305	\$1,344	\$1,384	\$1,426	\$1,469	\$1,513	\$1,558	\$1,605	\$1,653	\$1,70	2
Other			\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$1	J
Total O&M Ex	kpenses		\$24,620	\$25,359	\$26,119	\$26,903	\$27,710	\$28,541	\$29,398	\$30,279	\$31,188	\$32,124	\$33,087	\$34,080	\$35,102	\$36,155	\$37,240	\$38,357	\$39,508	\$40,693	\$41,91	4
Pumping Power	Costs:																					
2-Week Annu	al Pumping Duration ⁸		\$27,025	\$27.836	\$28,671	\$29,531	\$30,417	\$31,329	\$32,269	\$33,237	\$34.234	\$35,261	\$36,319	\$37.409	\$38,531	\$39,687	\$40,878	\$42,104	\$43,367	\$44.668	\$46,00	8
	al Pumping Duration ⁸		\$32,153		\$34,111	\$35,135	\$36,189	\$37,275	\$38,393	\$39,545	\$40,731	\$41,953	\$43,211	\$44,508	\$45,843	\$47,218	\$48,635	\$50,094	\$51,597	\$53,145	\$54,73	
	al Pumping Duration ⁸		\$37,282		\$39,552	\$40,739	\$41,961	\$43,220	\$44,516	\$45,852	\$47,227	\$48,644	\$50,104	\$51,607	\$53,155	\$54,750	\$56,392	\$58,084	\$59,826	\$61,621	\$63,47	
						. ,								. ,								
8-Week Annua	al Pumping Duration ⁸		\$42,410	\$43,682	\$44,993	\$46,343	\$47,733	\$49,165	\$50,640	\$52,159	\$53,724	\$55,336	\$56,996	\$58,706	\$60,467	\$62,281	\$64,149	\$66,074	\$68,056	\$70,098	\$72,20	L
																						Т
Year		0	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	

NOTES:

Total Field Cost is from Engineer's Opinion of Probable Costs, includes construction costs and contingency.
 Current Cost is equal to the Engineer's opinion of the probable cost of the project at beginning of project life (2012 dollars) plus the current estimated cost of disposal and removal.

\$4,875,999

\$9,751,999

\$19,503,998

3) Future cost is value or the project cost at end of life cycle of the project, or the current cost inflated at the rate shown through the life cycle of the project.

4) Salaries assumes salary for 1/12 full-time equivalent (FTE) to help manage/operate the pump station, or one person for about 8 hours per week during irrigation season.

5) Benefits assumes benefits = salaries X 40%.

6) Allowance for trips to and from pump station.

7) Estimated in the first year as 0.3% of the capacital cost of the pump station, rounded to the nearest \$100.

20	21	22	23
\$40,177	\$41,382	\$42,624	\$43,902
\$22,234	\$24,106	\$26,071	\$28,132
\$803,539	\$869,027	\$937,722	\$1,009,756
<i>Q</i> QQQQQ	<i>\$665,62</i>	<i>\$567,7</i> -	<i>_</i> ,,
\$80,354	\$82,765	\$85,247	\$87,805
\$44,468	\$48,212	\$52,142	\$56,263
\$1,607,078	\$1,738,055	\$1,875,444	\$2,019,512
\$160,708	\$165,529	\$170,495	\$175,610
\$88,935	\$96,425	\$104,283	\$112,527
\$3,214,156	\$3,476,109	\$3,750,887	\$4,039,024
ćr 707	¢5.060	¢c 120	66 222
\$5,787	\$5,960	\$6,139	\$6,323
\$2,315	\$2,384	\$2,456	\$2,529
\$877	\$903	\$930	\$958
\$32,440	\$33,413	\$34,415	\$35,448
\$1,754	\$1,806	\$1,860	\$1,916
\$0 642 171	\$0 \$44,466	\$0 \$45 800	\$0 \$47,174
\$43,171	\$44,400	\$45,800	\$47,174
\$47,388	\$48,810	\$50,274	\$51,783
\$56,381	\$58,073	\$59,815	\$61,609
\$65,374	\$67,335	\$69,355	\$71,436
\$74,367	\$76,598	\$78,895	\$81,262
20	21	22	23

\$15) 22 0	<i>\$</i> 10,570	<i>\$17,575</i>	<i>\(\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</i>	<i>\$56,655</i>	Ç52) 122	<i>ç</i> 55,555 i	<i>\$55,61</i>	\$37, 2 05	<i>\$55,661</i>	<i>\$00),11</i>	¢0 ≥ ,55 i	\$01, 11 E	<i>\$66,166</i>	<i>\$66,555</i>	<i>\$10,131</i>	¢72,501	<i>\$7.1)7.1</i>	<i>ç, 0,505</i>	¢, 5,255	<i>\$01)071</i>	\$0 I)122	\$00)0 is	
\$30,293	\$32,558	\$34,932	\$37,419	\$40,024	\$42,752	\$45,607	\$48,595	\$51,721	\$54,991	\$58,411	\$61,987	\$65,724	\$69,630	\$73,711	\$77,974	\$82,427	\$87,077	\$91,931	\$96,999	\$102,288	\$107,806	\$113,564	
\$1,085,268	\$1,164,402	\$1,247,308	\$1,334,139	\$1,425,059	\$1,520,232	\$1,619,834	\$1,724,043	\$1,833,047	\$1,947,040	\$2,066,222	\$2,190,803	\$2,320,999	\$2,457,036	\$2,599,145	\$2,747,570	\$2,902,561	\$3,064,379	\$3,233,294	\$3,409,585	\$3,593,544	\$3,785,472	\$3,985,681	\$
\$90,439	\$93,152	\$95,947	\$98,825	\$101,790	\$104,844	\$107,989	\$111,229	\$114,565	\$118,002	\$121,542	\$125,189	\$128,944	\$132,813	\$136,797	\$140,901	\$145,128	\$149,482	\$153,966	\$158,585	\$163,343	\$168,243	\$173,290	
\$60,585	\$65,116	\$69,864	\$74,838	\$80,048	\$85,504	\$91,214	\$97,190	\$103,443	\$109,983	\$116,822	\$123,973	\$131,448	\$139,260	\$147,422	\$155,949	\$164,854	\$174,154	\$183,863	\$193,998	\$204,575	\$215,613	\$227,128	
\$2,170,536	\$2,328,805	\$2,494,615	\$2,668,279	\$2,850,117	\$3,040,464	\$3,239,667	\$3,448,086	\$3,666,094	\$3,894,079	\$4,132,444	\$4,381,606	\$4,641,999	\$4,914,071	\$5,198,290	\$5,495,140	\$5,805,122	\$6,128,758	\$6,466,587	\$6,819,170	\$7,187,088	\$7,570,944	\$7,971,363	\$
\$180,878	\$186,304	\$191,893	\$197,650	\$203,580	\$209,687	\$215,978	\$222,457	\$229,131	\$236,005	\$243,085	\$250,377	\$257,889	\$265,625	\$273,594	\$281,802	\$290,256	\$298,964	\$307,933	\$317,171	\$326,686	\$336,486	\$346,581	
\$121,171	\$130,232	\$139,728	\$149,677	\$160,097	\$171,007	\$182,428	\$194,380	\$206,885	\$219,966	\$233,645	\$247,947	\$262,896	\$278,520	\$294,844	\$311,897	\$329,708	\$348,307	\$367,725	\$387,995	\$409,150	\$431,225	\$454,257	
\$4,341,072	\$4,657,609	\$4,989,231	\$5,336,558	\$5,700,235	\$6,080,929	\$6,479,334	\$6,896,172	\$7,332,188	\$7,788,158	\$8,264,888	\$8,763,212	\$9,283,997	\$9,828,142	\$10,396,581	\$10,990,280	\$11,610,245	\$12,257,516	\$12,933,174	\$13,638,340	\$14,374,176	\$15,141,888	\$15,942,726	Ş1
\$6,513	\$6,708	\$6,909	\$7,117	\$7,330	\$7,550	\$7,777	\$8,010	\$8,250	\$8,498	\$8,753	\$9,015	\$9,286	\$9,564	\$9,851	\$10,147	\$10,451	\$10,765	\$11,088	\$11,420	\$11,763	\$12,116	\$12,479	
. ,	\$2,683	\$2,764	\$2.847		\$3,020	\$3,111	\$3,204	\$3,300	\$3,399	\$3,501	\$3,606	\$3,714	\$3,826	\$3,940	\$4,059	\$4,180	\$4,306	\$4,435	\$4,568	\$4,705	\$4,846	\$4,992	
\$2,605	. ,		1 /-	\$2,932			. ,				. ,	. ,						. ,	. ,	. ,	. ,		
\$987	\$1,016	\$1,047	\$1,078	\$1,111	\$1,144	\$1,178	\$1,214	\$1,250	\$1,288	\$1,326	\$1,366	\$1,407	\$1,449	\$1,493	\$1,537	\$1,584	\$1,631	\$1,680	\$1,730	\$1,782	\$1,836	\$1,891	
\$36,511	\$37,607	\$38,735	\$39,897	\$41,094	\$42,327	\$43,596	\$44,904	\$46,251	\$47,639	\$49,068	\$50,540	\$52,056	\$53,618	\$55,227	\$56,883	\$58,590	\$60,348	\$62,158	\$64,023	\$65,944	\$67,922	\$69,960	
\$1,974	\$2,033	\$2,094	\$2,157	\$2,221	\$2,288	\$2,357	\$2,427	\$2,500	\$2,575	\$2,652	\$2,732	\$2,814	\$2,898	\$2,985	\$3,075	\$3,167	\$3,262	\$3,360	\$3,461	\$3,565	\$3,671	\$3,782	
\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	
\$48,590	\$50,047	\$51,549	\$53,095	\$54,688	\$56,329	\$58,019	\$59,759	\$61,552	\$63,399	\$65,300	\$67,260	\$69,277	\$71,356	\$73,496	\$75,701	\$77,972	\$80,311	\$82,721	\$85,202	\$87,758	\$90,391	\$93,103	
\$53,336	\$54,936	\$56,584	\$58,282	\$60,030	\$61,831	\$63,686	\$65,597	\$67,564	\$69,591	\$71,679	\$73,830	\$76,044	\$78,326	\$80,676	\$83,096	\$85,589	\$88,156	\$90,801	\$93,525	\$96,331	\$99,221	\$102,197	
\$63,457	\$65,361	\$67,322	\$69,342	\$71,422	\$73,565	\$75,771	\$78,045	\$80,386	\$82,798	\$85,281	\$87,840	\$90,475	\$93,189	\$95,985	\$98,865	\$101,831	\$104,885	\$108,032	\$111,273	\$114,611	\$118,049	\$121,591	
\$73,579	\$75,786	\$78,060	\$80,402	\$82,814	\$85,298	\$87,857	\$90,493	\$93,207	\$96,004	\$98,884	\$101,850	\$104,906	\$108,053	\$111,295	\$114,633	\$118,072	\$121,615	\$125,263	\$129,021	\$132,891	\$136,878	\$140,985	
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\$83,700	\$86,211	\$88,798	\$91,461	\$94,205	\$97,031	\$99,942	\$102,941	\$106,029	\$109,210	\$112,486	\$115,861	\$119,336	\$122,917	\$126,604	\$130,402	\$134,314	\$138,344	\$142,494	\$146,769	\$151,172	\$155,707	\$160,378	
24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	Í.
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\$45,220 \$46,576 \$47,973 \$49,413 \$50,895 \$52,422 \$53,994 \$55,614 \$57,283 \$59,001 \$60,771 \$62,594 \$64,472 \$66,406 \$68,399 \$70,451 \$72,564 \$74,741 \$76,983 \$79,293 \$81,671 \$84,122 \$86,645

47	48	49	50
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\$89,245	\$91,922	\$94,680	\$97,520
\$119,570	\$125,835	\$132,368	\$139,179
\$4,194,496	\$4,412,253	\$4,639,300	\$4,875,999
\$178,489	\$183,844	\$189,359	\$195,040
\$239,141	\$251,670	\$264,735	\$278,358
\$8,388,993	\$8,824,507	\$9,278,601	\$9,751,999
\$356,978	\$367,688	\$378,718	\$390,080
\$478,282	\$503,340	\$529,470	\$556,716
\$16,777,986	\$17,649,013	\$18,557,202	\$19,503,998
\$12,854	\$13,239	\$13,636	\$14,046
\$5,141	\$5,296	\$5,455	\$5,618
\$1,948	\$2,006	\$2,066	\$2,128
\$72,058	\$74,220	\$76,447	\$78,740
\$3,895	\$4,012	\$4,132	\$4,256
\$0	\$0	\$0	\$0
\$95,896	\$98,773	\$101,736	\$104,788
\$105,263	\$108,421	\$111,674	\$115,024
\$125,239	\$128,996	\$132,866	\$136,852
\$145,214	\$149,571	\$154,058	\$158,679
\$165,190	\$170,145	\$175,250	\$180,507
47	48	49	50