U.S. Fish and Wildlife Service

Icicle Creek Instream Flow and Fish Habitat Analysis for the Leavenworth National Fish Hatchery ** Final **

U.S. Fish and Wildlife Service Columbia River Fisheries Program Office Vancouver, WA 98683

On the cover: The Icicle Creek historical channel in October 2011 with 150 cfs flowing down it. This section of the creek is about half way between structures 2 and 5 and is typical of the study site. Note the undercut banks with overhanging vegetation which is excellent rearing fish habitat but there is a lack of alluvium both in the stream channel and higher up along the banks. Photograph by Joe Skalicky.

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Icicle Creek Instream Flow and Fish Habitat Analysis for the Leavenworth National Fish Hatchery

Final 401 CWA Certification Order No. 7192 for the Leavenworth National Fish Hatchery on Icicle Creek, Chelan County, Washington

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Disclaimers

The findings and conclusions in this report are those of the authors and do not necessarily represent the views of the U.S. Fish and Wildlife Service.

The mention of trade names or commercial products in this report does not constitute endorsement or recommendation for use by the federal government.

Executive Summary

The purpose of this study was to characterize the relationship between streamflow and fish habitat in the Icicle Creek historical channel, Washington, upstream from the Leavenworth National Fish Hatchery. The creek exhibits a very complex set of fish habitats including many islands, back channels and overhanging banks. Peak stream flow occurs during late spring and low flows occur during late summer and fall. Fish species selected for habitat assessment included: coho, spring/summer Chinook, steelhead/rainbow trout, bull trout, Westslope cutthroat, mountain whitefish Pacific lamprey and suckers.

The method used for the habitat assessment incorporated output from a two-dimensional hydrodynamic model (River2D) and a unique GIS cell-based habitat modeling approach. The method was reviewed and approved by the Washington State Department of Ecology. This method is based on the premise that stream dwelling fish prefer a certain range of depths, velocities, substrates and cover types, depending on the species and life stage, and that the availability of these preferred habitat conditions varies with streamflow. Weighted Usable Area (WUA) is the primary product of PHABSIM and the primary results produced for the GIS cellbased habitat modeling approach. Weighted usable area is an index of habitat availability or quantity and quality for the selected species/life stage at each simulated flow. Weighted usable area was calculated for a range of streamflows between 20 and 1500 cfs. Graphs and tables of WUA versus flow are presented for each life stage and species of interest. In addition to WUA, estimates of high quality usable (UA) area were produced and compared to WUA. UA is a value index of habitat because it quantifies high quality habitat whereas WUA quantities all levels of habitat including large amounts of low quality habitat that may rarely be used by fish. This technical information can be used by the relevant stakeholders and managers along with other site specific hydrological and biological information as the basis for instream flow recommendations in the Icicle Creek historical channel.

An instream flow and fish habitat analysis cannot by itself determine the instream flow required by a given fish species. The WUA graphs only show whether an increase or decrease in streamflow will increase or decrease the quantity and quality of fish habitat. The study's predicted fish habitat versus streamflow results have to be interpreted by knowledgeable biologists and others to arrive at an instream flow regime that satisfies applicable laws.

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Introduction

On January 7, 2010 the Washington Department of Ecology (Ecology) issued Order number 7192, in the matter of granting a Water Quality Certification to the U.S. Fish and Wildlife Service (USFWS) for the Leavenworth National Fish Hatchery (Leavenworth NFH). The certification requires implementation of an instream flow study to aid in determining the effect of hatchery operations on fish habitat. The main purpose of this study was to evaluate fish habitat as a function of streamflow using the Weighted Usable Area (WUA) index to produce estimates of habitat quantity and quality. This report describes the Leavenworth NFH's requirement to evaluate fish habitat with an instream flow study as required in the Clean Water Act (CWA) Certification Order No. 7192. Prior to implementing this study, a detailed study plan was submitted to Ecology for approval. The study plan was approved by Ecology and included comments from the Leavenworth NFH, Washington Department of Fish and Wildlife (WDFW) and The USFWS Mid-Columbia Fishery Resource Office and Ecology. A companion report submitted to Ecology details fish passage evaluations in Icicle Creek and is not discussed here, but the results of this instream flow study supported some components of the passage evaluations.

Project Goal

The overall goal of the Icicle Creek instream flow study was to quantify fish habitat as a function of streamflow in the Icicle Creek historical channel (hereafter referred to as historical channel) for the assessed fish species and lifestages; to determine streamflows required to maintain channel structure, complexity, and physical habitat; and to provide guidance regarding the integration of the target species habitat needs for the Icicle Creek historical channel hydrograph configuration. With the exception of streamflow in cubic feet/second (cfs) and River Mile (RM) all measurement units are metric including calculations of habitat unless otherwise noted.

Objectives

- 1) Produce species/lifestage specific habitat flow relationships using a two-dimensional (2D) hydrodynamic model and a GIS cell-based habitat model.
- 2) Produce spatially explicit maps depicting the distribution of the primary habitat variables for representative stream flows.
- 3) Produce tabular and graphic results that quantify species/lifestage specific habitat for streamflows from 20 to 1,500 cfs and the corresponding incremental gains or losses over a range of flows.
- 4) Estimate flushing flows, channel maintenance flows, and channel forming flows for the Icicle Creek historical channel.
- 5) Integrate species-specific habitat-flow relationships to accommodate the habitat needs for multiple target fish species/lifestages that may occur simultaneously in the Icicle Creek historical channel.

Project Description

The Leavenworth NFH is located in North Central Washington adjacent to Icicle Creek at river mile (RM) 3.0 and is two miles south of Leavenworth, Washington. In the 1930's, the Leavenworth NFH was authorized by Congress as mitigation for fish losses associated with the construction and operation of Grand Coulee Dam. Leavenworth NFH withdraws surface water from Icicle Creek at RM 4.5, utilizes it for fish production at the hatchery, and returns it to Icicle Creek at RM 2.8 [\(Figure 1\)](#page-13-2). The hatchery annually produces 1.2 million juvenile spring Chinook salmon and provides acclimation facilities for coho salmon. These salmon contribute to commercial, sport, and tribal in-river and ocean fisheries alike.

Figure 1. **Project overview depicting the location of Leavenworth NFH, the hatchery intake, the Icicle Creek historical channel, the Icicle Creek hatchery channel and Structures 2 and 5.**

Instream Flow Study Reach Description

The portion of Icicle Creek evaluated with an instream flow study is known as the Icicle Creek historical channel and extends approximately one mile from RM 2.8 to 3.8. Approximately half of the historical channel was modeled in the center section of the historical channel. The downstream model boundary requires a robust rating curve that is not hydraulically affected by other flow parameters (Icicle Creek – hatchery channel spillway or Wenatchee River backwater),

islands, and/or artificial control structures (Structure 5). As such, the boundary is upstream from the confluence of the Icicle Creek hatchery channel (hereafter referred to as hatchery channel) and Structure 5. The reach modeled is depicted in [Figure 2](#page-14-1) and is the portion highlighted in blue. To address an unexpected funding shortfall in Fiscal Year 2012, the top 400 m (~1/4 mile) of the study site near Structure 2 was omitted from the hydrodynamic model; however estimates of fish habitat are likely valid for this area since the fundamental channel morphology and hydrodynamics appeared to be similar.

Figure 2. Overview of the instream flow study site. The section of the Icicle Creek historical channel modeled is outlined in blue and the black bars depict the upstream and downstream boundaries of the instream flow study reach.

Leavenworth National Fish Hatchery History

In 1939, a series of small control structures were built in the historical channel to function as an actual instream hatchery and to assist with the capture of migrating anadromous salmon for hatchery broodstock. A separate channel (hatchery channel) was also built adjacent to the Icicle Creek historical channel (Figure 2) to control flows between the two channels for hatchery operations. This regulation of streamflow in addition to Icicle Creek being a very high sediment load stream induced sediment deposition in the historical channel and led to subsequent colonization of the stream channel and banks by riparian plants (Lorang 2005). The historical channel was used for fish production from the 1940's to the late 1970's, and seasonally, as recently as 2005. Some of the small structures have since been removed but two structures remain at the terminal ends of the Icicle Creek historical channel, Structures 2 and 5 (Figure 2). Due to the streamflow limitations through Structure 2, the historical channel has not benefited from the channel forming flows and undercut banks are far more extensive than they otherwise might be (Jim Craig, pers. comm. 2010). Historic and new sedimentation from upstream mass wasting and the sediment deposits resulting from the old control structures are still present. The Icicle Creek historical channel still appears to be in flux.

Hydrology

The Icicle Creek drainage is located on the eastern flanks of the Cascade Mountain Range and the drainage basin encompasses an area of approximately 50 hectares or 193 square miles. Icicle Creek is a high elevation drainage with 14 glaciers, 102 lakes, and 85 tributaries. Icicle Creek is a tributary of the Wenatchee River. The hydrology is primarily driven by snowmelt, and peak flows as measured by the USGS Gage #12458000 (Icicle Creek above Snow Creek near Leavenworth, WA) occur during late spring, while low flows occur during late summer, fall, and winter [\(Figure 3\)](#page-16-0). Extremes for the period of record range from a minimum of 44 cubic feet per second (cfs) to a maximum of 19,800 cfs, and the mean annual flow is 624 cfs. Mean monthly flow statistics which are a different statistic than the 50% exceedance flows are depicted in [Table](#page-16-1) [1.](#page-16-1) The USGS gage at RM 5.8 is located above all major points of diversion. Icicle Creek streamflows below the USGS gage are altered by water diversions which reduce downstream flows. The City of Leavenworth and the Icicle-Peshastin Irrigation District divert water above the Snow Lakes trailhead (RM 5.7), and Leavenworth NFH and the Cascade Orchard Irrigation Company divert water below the trailhead (RM 4.5). These irrigation diversions can remove up to 48% and 79% of the mean monthly August and September streamflows, respectively (Mullan *et al.* 1992). To assure adequate water for the Leavenworth NFH, a supplementary water supply was developed in the Snow Lakes Basin (Nada, Upper and Lower Snow Lakes, about seven miles upstream from Leavenworth NFH. Without the water release of approximately 50 cfs from the Snow Lakes Basin, from late July through early October, some downstream reaches of Icicle Creek could potentially go dry in low snow pack years.

Figure 3. **Exceedance flows as measured at USGS Gage #12458000 on Icicle Creek near Leavenworth, WA for an average, wet, and dry year for the period of record (1936 – 2012). Mean annual flow is 624 cfs.**

Exceedance Flows at USGS Gage #12458000 on Icicle Creek

Hydrodynamic Modeling

Hydrodynamic Model Introduction and Overview

The use of two-dimensional hydrodynamic models has gained wide use and acceptance in fisheries and instream flow assessments (Tharme 2003, Stewart et al. 2005, Mingelbier et al. 2008, Hatten et al. 2008, Lee et al. 2010, Waddle 2010, Ban et al. 2011). Two-dimensional flow models describe flow dynamics in two horizontal vectors whereas a one-dimensional model describes them in only one. Neither model calculates any difference in vertical conditions thus they are termed "depth-averaged" models.

For the historical channel instream flow assessment, the River2D hydrodynamic model (Ghanem et al. 1996, Steffler and Blackburn 2002) was used to simulate continuous surfaces of hydrologic parameters throughout the study site. The only parameters output from the model for habitat modeling were depth and velocity magnitude. River2D is a two dimensional (2D), depth averaged, finite element hydrodynamic model. The model and documentation are available at: [http://www.river2d.ualberta.ca/.](http://www.river2d.ualberta.ca/) As with other 2D models, River2D uses three governing equations to solve for three unknowns; depth and mass flux in both the x and y directions. As well, the model has three basic assumptions.

- *1. The vertical pressure distribution is hydrostatic. This can potentially limit the accuracy of the model in areas of steep slopes and rapid changes of bed slopes. In general, bed features of horizontal size less than about 10 depths (typically dune formations) will not be modeled accurately. (No significant dune formations were observed in Icicle Creek.)*
- *2. The distributions of horizontal velocities over the depth are essentially constant (depthaveraged).*
- *3. Wind and Coriolis forces are assumed negligible. These forces are only significant to very large bodies of water, the historical channel not being one of them.*

Fundamental Concepts

Conservation of Mass. Mass conservation is the principal that the inflow of fluid to any point in the model matches outflow. This is evidenced by summing the mass flux in the x and y directions and setting the total mass flux equal to the change in depth over a smaller time increment. As such, if inflow is greater than outflow over a small time frame, the depth increases. If inflow equals outflow, the depth is unchanged, and so on. This approach is used in hydrodynamic models to allow simulation of unsteady flow conditions based on varying inflow and outflow.

Conservation of x- and y-direction momentum. A major contribution of 2D flow models is the ability to represent physical forces acting on the fluid. Changes to the momentum in River2D are represented as a sum of forces. The forces include shear stresses, gravitation force and friction forces. The great advantage of this is improved representation in rivers is evidenced by divided flow situations (islands) when compared to transect-based models. This is one the reasons River2D was chosen in the Icicle Creek historical channel over the PHABSIM method.

Frictional Forces. Friction in River2D is represented by a continuous surface or "skin" which is constructed directly from effective bed roughness height. Effective roughness height is used because it tends to remain constant over a wider flow range than other measures of roughness including Manning's n and it can be approximated from dominant bed material.

The ability of River2D to accurately model supercritical flow and edge wetting is an additional advantage over transect based modeling. In the event that the Icicle Creek historical channel has supercritical flows, the model will accurately simulate them. River2D uses a Petrov-Galerkin upwinding formulation to solve the flow-field. With this feature, the model can represent situations where upstream flow conditions limit the water surface at a downstream point. This enables the model to accurately simulate hydraulic conditions over sills, steep bars and other conditions that could possibly be present in the historical channel.

The historical channel study site has many side channels that are only wet at specific streamflows. This is a difficult process for numerical models and River2D has a unique and robust method of estimating this. The depth of flow is a dependent variable and is not known in advance when performing a two-dimensional flow simulation. As such, the horizontal range of the water coverage is therefore unknown. Additionally, significant computational difficulties are encountered when the depth is very shallow or it is dry at part of the modeled area. Various methods have been proposed to deal with this "edge wetting" problem. For example, some models simply neglect or drop out partially wet edge elements; others declare edge elements to be porous. The River2D model handles these occurrences by incorporating a simplified ground water model with the surface water model. In these wet/dry areas, the model changes the surface flow equations to groundwater flow equations. This allows a mesh element to have some nodes that are under surface water using the open-channel flow equation of mass conservation and some that are under the land surface using a sub-surface representation for mass conservation. A continuous free surface with positive (above ground) and negative (below ground) depths is calculated. This unique approach allows calculations to carry on without changing or updating the boundary conditions as water levels fluctuate.

Icicle Creek Hydrodynamic Modeling – Methods & Results

Both the Methods and Results for hydrodynamic modeling are presented here for reader *continuity and they are precursors to the subsequent habitat assessment.*

Hydrodynamic modeling in the Icicle historical channel was comprised of the following steps:

- *1. Develop a digital elevation model (DEM) of the Icicle Creek historical channel study site*
- *2. Collect hydrologic boundary data (paired inflow discharge and outflow WSE's)*
- *3. Collect representative roughness data*
- *4. Construct Computational Meshes*
- *5. Calibrate and Validate the hydrodynamic model*
- *6. Simulate unmeasured flows*

1. Digital Elevation Model Development

Two dimensional hydrodynamic models require a digital elevation model (DEM) of the stream channel to construct computational meshes and simulate streamflows. For the Icicle Creek historical channel instream flow assessment all geographic data including the DEM, were collected in or adjusted to a common projection and coordinate system, *Lambert Conformal Conic and Washington State-plane North*, respectively. In addition, the *Horizontal Datum, North American Datum of 1983 (NAD83)* as well as the *Vertical Datum, North American Vertical Datum of 1988 (NAVD88)* were used.

Preexisting topographic information consisting of LiDAR data collected in 2006 (Watershed Sciences 2006) was initially evaluated for potential use but found to have too much error. LiDAR for the Icicle Creek area was flown during the month of October, 2006 with a reported point density of ≥8 points per square meter. The vegetative cover in this area ranges from flat, grassy banks along the creek, to steep, highly vegetated slopes. Surveying with RTK and a total station was conducted during October, 2011. Total station points, accurate within ± 5 mm were compared with the LiDAR points classified as bare earth rather than the tops of vegetation. Both sets of points were loaded into a GIS and the Near tool was used to determine the closest LiDAR point to each total station point. The first 50 pairs of points, representing the smallest distance (≤25 mm) between a LiDAR and a total station point, were compared. The difference in elevation values ranged from 1 mm to 1.7 m. The vegetation cover for each point as well as the location along the creek was noted. There is no consistent variation of elevation values given vegetation or location. Due to the error and inconsistencies, the 2006 LiDAR data was not used. The collection of new LiDAR data was planned but inclimate weather preempted data collection.

Up to four survey grade Real Time Kinematic (RTK) GPS instruments and a single auto-tracking total station were used to collect topographic and bathymetric data in historical channel [\(Figure](#page-20-0) [4\)](#page-20-0). The RTK accuracy is approximately 3 cm with good satellite geometry. Data was collected along natural stream and channel breaks defining the geometry of the stream channel. Data collection occurred on the weeks of October $18th$, November 1st in 2011 and again the week of September $17th$ 2012. The September date was the first opportunity to collect deep water

bathymetry which is only accessible at the lowest of streamflows in the Icicle Creek historical channel. A total of 4,988 georeferenced data points were collected in historical channel study site [\(Figure 5\)](#page-20-1). At each point, X, Y and Z geographic positions were recorded as well measures of substrate and cover.

Figure 4. Survey grade RTK GPS (left) and total station (right) used to collect topographic data of the Icicle Creek historical channel.

Figure 5. Topographic data collected in the study site comprised of 4,988 points used to generate a DEM.

Using USGS rating curve standards, staff from the USFWS, Water Resources Division and Leavenworth NFH jointly collected hydrologic data in the historical channel in conjunction with other hydrologic evaluations related to hatchery operations. River2D requires two input boundary conditions for hydrodynamic simulation at a given discharge. These conditions include an inflow discharge at the upstream boundary and the corresponding downstream water surface elevation. Standard practice is to develop a rating curve so that all flow conditions between the lowest and highest flow can be simulated with the required data pairs (streamflow and water surface elevation). [Table 2](#page-21-1) depicts the relationship derived between streamflow at the upstream boundary and water surface elevation at the downstream boundary. In addition, the relationship between streamflow and the upstream boundary water surface elevation (WSE) was derived to initiate each model run. The model calculates this automatically but an accurate estimate will result in faster model solution times.

Table 2. – Relationship between streamflow and water surface elevations used for hydrodynamic modeling in the Icicle Creek historical Channel. In the table below the column header cms stands for cubic meters per second.

	Discharge	Water Surface Elevations (m)						
cfs	cms	Downstream Boundary	Upstream Boundary					
20	0.566	340.331	342.475					
30	0.850	340.361	342.492					
40	1.133	340.385	342.509					
50	1.416	340.406	342.526					
60	1.699	340.425	342.543					
70	1.982	340.443	342.559					
80	2.265	340.461	342.576					
90	2.549	340.477	342.592					
100	2.832	340.493	342.608					
120	3.398	340.525	342.640					
140	3.964	340.553	342.671					
160	4.531	340.579	342.702					
180	5.097	340.606	342.732					
200	5.663	340.631	342.762					
250	7.079	340.690	342.833					
300	8.495	340.746	342.901					
350	9.911	340.798	342.965					
400	11.327	340.848	343.026					
450	12.743	340.895	343.083					
500	14.158	340.942	343.136					
550	15.574	340.986	343.186					
600	16.990	341.030	343.232					
650	18.406	341.070	343.274					
700	19.822	341.112	343.313					
750	21.238	341.151	343.349					

3. Collect Representative Roughness Data

Frictional bed forces within a moving body of water have a direct effect on the fluids moving past them. Large boulders will slow water down more than small pebbles due their greater height into the water column (roughness height). River 2D requires a skin or layer of roughness heights to accurately estimate hydrodynamic conditions. Measurement values of substrate size and their associated roughness heights were used to characterize roughness throughout the model domain. Substrate was mapped among classes matching WDFW's generic substrate codes (WDFW and WDOE, April 1, UPDATED 2013 publication). The field effort occurred in conjunction with the topographic and bathymetric mapping. In total 4,988 data points describing roughness (substrate size) were collected and subsequently mapped in a GIS. A GIS algorithm was used to interpolate substrate values in-between data points collected in the field. Interpolation provided a continuous surface of substrate values [\(Figure 6\)](#page-22-1). The average particle size for each dominate substrate class mapped was used to generate the roughness values input into River2D. Where required, codes values for vegetation were also used to infer roughness values [\(Figure 7\)](#page-23-1).

Figure 6. Continuous surface of substrate values interpolated in a GIS from point data collected in the field.

Figure 7. Continuous surface of cover values interpolated in a GIS from point data collected in the field.

4. Computational Mesh Construction

The computational mesh is defined by the nodes laying at the intersection of each element and is the numerical framework for which all the hydrodynamic computations both occur and are produced. In 2D hydrodynamic modeling, there is a trade-off between the density of nodes in the computational mesh, the required accuracy to represent the study site, and the time required to arrive at a solution for a single discharge. Generally, to obtain the best fit to the main channel and other significant or complex habitat areas, the mesh density will vary among locations and channel configurations [\(Figure 8\)](#page-24-1). It is desirable to have a minimum of 8 to 10 nodes across channels carrying significant amounts of water to ensure the model can adequately convey flow downstream without calculating too much of the flow at any one node. The upstream and downstream model boundaries usually need to be subdivided into 20 or more nodes to ensure that no node carries too much of the computational burden. Some sites in the historical channel have numerous side channels or large boulders and it was necessary to increase the mesh/node density to capture and adequately represent the natural complexity.

Figure 8. **Computational mesh from River2D depicting varying node densities across the stream channel. The elements are the legs of each triangle defining the mesh between each of the nodes. This section of Icicle Creek is in the lower third of the study site.**

Three unique and distinct meshes were constructed for the low, medium and high flows, respectively. [Figure 8](#page-24-1) above depicts the mesh that was constructed for the medium flows. All three meshes were built with a dual mesh density composed of 1.0 and 2.0 meter spacing. However, areas with complex bathymetry had densities as fine as 0.125 m. A 2.0 m node spacing was only used for dry areas. The number of computational nodes ranged from 41,847 to 75,819 and elements from 82,382 to 149,168.

5. Model Calibration and Validation

When compared to the real world all models contain some amount of error. In hydrodynamic modeling, this error can arise from assumptions built into the model itself, but predominantly, errors arise from misrepresentations of the stream channel (DEM). Most error results from an under-representation of the stream bathymetry, bed interpolation related errors, and/or actual errors in bathymetry measurement. Additional "false" error can arise if changes in the stream channel including scour and aggradation occur between mapping of the stream channel and the collection of model calibration and validation data. In 2D hydrodynamic modeling, the general

calibration process consists of calibrating the model to three separate and bounding conditions; a low, average, and high flow condition. This is done by comparing and validating empirical field measurements of water surface elevation, velocity, and depth to the corresponding modeled calibration flows. In practice, calibration to a longitudinal profile of water surface elevations (upstream to downstream) is the most accurate calibration method and the technique used in the evaluation (Terry Waddle, USGS – Fort Collins, personal communication). Fundamentally, if water surface elevations are accurate then so will the depths and the resulting water velocity magnitudes. For the depth and velocity comparisons, data was collected along 4 cross sections, perpendicular to streamflow [\(Figure 9\)](#page-25-0). Depth and velocity data were used primarily for validation.

Observations of water surface elevations were collected from upstream to downstream at each of the three calibration flows using RTK GPS. As previously stated the RTK GPS error is generally +/- 3cm but can occasionally be more if the satellite geometry is poor. For example on the spike on the WSE plot [Figure 12](#page-27-0) is likely additionally RTK GPS error given that is area of the historical channel is pooled and has extremely flat water surface elevations. [Figure 9](#page-25-0) depicts where observations of Water Surface Elevation (WSE) were collected for the medium calibration flow along longitudinal or upstream to downstream "cross sections." Data point locations were restricted by site access and satellite availability (i.e. not under trees) since RTK GPS was used. The locations of the low and high flow WSE observations are in similar but different locations. For each of these data collection efforts, it is imperative that the calibration data is collected at a steady stream flow throughout the study site to compare to the steady state calibration flows modeled.

Figure 9. Location of cross sections used to collect observations of depth and velocity at a low, medium and high flow and the point locations of WSE at the medium flow only. Point locations at the low and high flows were collected but are not depicted on this figure.

Like many other models, roughness values are used to adjust the model output to more closely match observed conditions. In practice this is a balancing act given that an adjustment of water surface elevation will have a direct effect on velocities and depth. For the historical channel, we ran each of the three calibration flows to steady state convergence and then incrementally adjusted roughness values for each specific calibration flow to bound the error between observed and simulated water surface profiles [\(Figure 10,](#page-26-0) [Figure 11](#page-26-1) and [Figure 12\)](#page-27-0).

Figure 10. Observed vs. simulated water surface elevations at 42 cfs (low calibration flow) for four potential roughness calibration flows. WSE_Kx6 represented the best overall fit and balance upstream to downstream.

Observed vs. Simulated Water Surface Elevations at 234 cfs

Figure 11. Observed vs. simulated water surface elevations at 234 cfs (medium calibration flow) for four potential roughness calibration flows. WSE_Kx6 represented the best overall fit and balance upstream to downstream.

Figure 12. Observed vs. simulated water surface elevations at 1,220 cfs (high calibration flow) for three potential roughness calibration flows. WSE_Kx9 represented the best overall fit and balance upstream to downstream. The spike in the lower left of the plot is likely the result of higher than normal RTK GPS error.

Observed vs. simulated velocities and depths were also compared for further validation to determine if additional adjustment of roughness values was warranted [\(Table 3\)](#page-28-1). If additional adjustment was warranted, we further adjusted roughness values to accomplish the best fit for matching both simulated water surface elevations and velocities to the empirical data.

Observations of water surface elevations were collected at 42, 234 and 1,220 cfs with 80, 57 and 30 data points collected respectively. The number of data points collected was based on: observed water surface elevation variability in the field, GPS fix availability and site access. Site access was significantly restricted at the 1,220 cfs streamflow. Survey grade RTK GPS instruments were used to collect water surface elevations and an Acoustic Doppler Current Profiler (ADCP) as well as a held meter was used to collect the velocity profiles perpendicular to the flow. Depth and velocity observations were collected along four cross sections spaced throughout the reach for each of the low, medium and high calibration flows. After each calibration flow was adjusted to the best fit, production modeling was conducted and model error for simulated depths and velocities relative to measured (observed) depths and velocities was assessed and reported as the mean absolute error MAE [\(Table 3\)](#page-28-1). [Table 4](#page-29-0) depicts the roughness adjustment factors used for all of the production modeling. Each adjustment factor i.e. kx6 denotes the multiplication factor used to adjust roughness. In this example a multiplier of 6 was used.

Table 3. Comparison between measured and simulated water surface elevation, velocity and depth at each of the low, medium and high calibration flows. Values in bold are those with the least mean absolute error and the highlighted rows represent the roughness calibration factor used to model the unmeasured flows. Water surface elevations were collected along longitudinal cross sections and velocity and depth were collected along cross sections perpendicular to streamflow.

Mean Absolute Error (m/s) - **Velocity**

Mean Absolute Error (m) - **Depth**

* Fines and a moving bed precluded the processing and spatial orientation of ADCP data collected at the high flows for velocity and depth.

6. Simulation of Unmeasured Flows

Once the model was calibrated to the best fit, simulation of unmeasured flows was conducted. This was completed by adjusting the boundary conditions (discharge and water surface elevation) of the nearest calibration flow to that of the unmodeled flow conditions and running the model to solution. This process was repeated until all unmeasured flows were simulated. Streamflows ranging from 20 to 1,500 cfs were successfully simulated [\(Table 4\)](#page-29-0) including the resulting depths and velocity magnitudes for subsequent habitat modeling. [Table 4](#page-29-0) also depicts which roughness values were used for the production runs based on the calibration process. [Figure 13](#page-30-0) and [Figure 14](#page-31-0) depict simulated depths and velocity magnitudes at 20, 300 and 1,000 cfs.

Table 4. Streamflows modeled and the associated roughness calibration factors used in the Icicle Creek historical channel. Roughness calibration factors are italicized.

						Streamflows (cfs) and the Associated Roughness Calibration Adjustment Factors	
20	Kx6	140	Kx6	550	Kx6	1,050	Kx6
30	Kx6	160	Kx6	600	Kx6	1,100	Kx6
40	Kx6	180	Kx6	650	Kx6	1,150	Kx6
50	Kx6	200	Kx6	700	Kx6	1,200	Kx6
60	Kx6	250	Kx6	750	Kx6	1,250	Kx6
70	Kx6	300	Kx6	800	Kx9	1,300	Kx9
80	Kx6	350	Kx6	850	Kx9	1,350	Kx9
90	Kx6	400	Kx6	900	Kx9	1,400	Kx9
100	Kx6	450	Kx6	950	Kx9	1,450	Kx9
120	Kx6	500	Kx6	1,000	Kx9	1,500	Kx9

Streamflows (cfs) and the Associated Roughness Calibration Adjustment Factors

Figure 13. Simulated depths at 20, 300 and 1,000 cfs in the Icicle Creek historical channel.

 \Box Meters

250

62.5 125

Figure 14. Simulated velocity magnitudes at 20, 300 and 1,000 cfs in the Icicle Creek historical channel.

Habitat Modeling Methods

Integration of the results of hydrodynamic modeling and mapped physical parameter distribution (substrate and cover) with habitat preference or suitability criteria for the fish species/lifestages of interest was required to develop the relationship between streamflow and the amount, quality, and distribution of physical fish habitat.

Icicle Creek Fish Species/Lifestages and Periodicity

Fish species/lifestages that were evaluated for the Icicle Creek historical channel instream flow study were discussed and agreed upon by staff from the Leavenworth NFH, Mid-Columbia River Fisheries Resource Office, Columbia River Fisheries Program Office, WDFW and WDOE. [Table 5](#page-32-2) lists the species/lifestages selected for assessment. The various lifestages for each species generally occur in the historical channel during specific time periods. These time periods [\(Table 5\)](#page-32-2) were the focal point for physical conditions and habitat estimates for each species/lifestage.

Species	Life-Stage	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Bull Trout	Adult-rearing												
	Juvenile rearing												
Steelhead /	Adult spawning												
Rainbow	Juvenile rearing												
Coho	Adult spawning												
Summer Chinook	Adult spawning												
	Juvenile rearing												
Spring Chinook	Adult spawning												
	Juvenile rearing												
	Adult spawning												
Mountain													
Whitefish	Adult rearing												
	Juvenile rearing												
Largescale &	Adult spawning												
Bridgelip Suckers	Adult rearing												
	Juvenile rearing												
Westslope	Adult spawning												
Cutthroat	rearing												
Pacific Lamprey	Adult spawning												

Table 5. Fish species/lifestages and periodicity for habitat assessment in the Icicle Creek historical channel.

Habitat Suitability Criteria

Habitat suitability criteria (HSC) that define the suitability (on a scale of 0 to 1) of physical and hydraulic factors such as water depth and velocity, substrate, cover and slope can be developed in many forms ranging from frequency distributions of habitat use for each parameter, to complex models using combinations of parameters to predict the probability of habitat use. The WDFW and WDOE have compiled habitat preference curves for a wide range of species and lifestages in the publication, Washington State Instream Flow Study Guidelines (WDFW and WDOE, April 1, UPDATED 2013 publication) that consist of observations of fish use relative to parameter availability. This approach accurately describes selection of specific conditions, or preference for those conditions compared to simple frequency analysis of field observations (habitat utilization curves). By accounting for both habitat use and habitat availability, the resulting curves tend to be much less site-specific than utilization curves (Bovee 1986, Bovee and Zuboy *eds*. 1988).

The State of Washington's most recent fish preference curves HSC for the historical channel habitat assessment were used for the cell based habitat assessment. The criteria used were from the 2013 Guidelines but the curve sets do not include curves for Pacific lamprey or suckers. As such curves used in other instream flow studies were reviewed and used for adult lamprey and suckers. For largescale and bridgelip suckers Murdoch et al. (2005) indicated that suckers use the same habitat as rainbow trout and no differences were found in the water depth and water velocity used by rainbow trout and bridgelip sucker for spawning. For adult lamprey, criteria from a flow study in the Yuba River, Yuba County Water Agency (2012) were used. These criteria also compared well with data collected by Stone (2006) in Cedar Creek in Southwest Washington State. Criteria for juvenile lamprey were not available. For whitefish there are no substrate criteria listed in the State's publication so substrate curves from a flow study in the Spokane River were used (EES Consulting. 2007).

Physical Parameters

Characterization of the component physical parameters and their spatial distribution is the primary task leading to an evaluation of habitat suitability. While some physical parameters remain fixed in space, others vary with streamflow. Depth and velocity are the primary variable parameters. Other parameters include substrate and cover. Depth and velocity were produced from River2D and substrate and cover were mapped in the field with survey grade RTK GPS. Dominant, sub-dominant and % dominant substrates are consistent with WDFW's preference curves [\(Table 6\)](#page-34-0). All of these parameters were exported into ArcGIS for subsequent habitat assessment and quantification as individual habitat grids using cell based modeling and map algebra. The cell size of each final habitat grid was one square meter.

Substrate and Cover

We modeled new raster data layers with the Euclidean allocation process in our GIS using the 4,988 vector-based substrate/cover data points. Each raster is a continuous cell-based model of the data for each of the four criteria used. The Euclidean algorithm records the identity of the

closest source point for each cell in the new raster. A distance is then calculated from the source point to the center of each of the surrounding cells without values. The algorithm proceeds as follows. For each cell, the distance is calculated to each source cell by calculating the hypotenuse with the x-max and y-max as the other two legs of the triangle. This calculation derives the true Euclidean, not cell, distance. The shortest distance to a source is determined and the value is assigned to the cell location on the output grid. That is to say, the cells were assigned characteristics of the nearest point with empirical data. At the end of the process, a continuous and complete surface was produced. This process was completed for each of the substrate coding systems (rearing and spawning), for cover codes, and for the percent fines layer, resulting in four separate grids.

Substrate		Size		Spawning		Rearing		Holding	
Code	Description	(inch)	salmon	steelhead	resident trout	bull trout	fry	juv.	adult
	silt, clay, or organic		0.00	0.00	0.00	0.00	0.10	0.10	0.10
2	sand		0.00	0.00	0.00	0.00	0.10	0.10	0.10
3	sm gravel	$0.1 - 0.5$	0.30	0.50	0.80	1.00	0.10	0.10	0.10
$\overline{4}$	med gravel	$0.5 - 1.5$	1.00	1.00	1.00	1.00	1.00	0.30	0.30
5	lrg gravel	$1.5 - 3.0$	1.00	1.00	0.80	1.00	1.00	0.30	0.30
6	sm cobble	$3.0 - 6.0$	1.00	1.00	0.50	0.70	1.00	0.50	0.30
7	lrg cobble	$6.0 - 12.0$	0.50	0.30	0.00	0.70	1.00	0.70	0.30
8	boulder	>12.0	0.00	0.00	0.00	0.00	1.00	1.00	1.00
9	bedrock	NA	0.00	0.00	0.00	0.00	0.10	0.30	0.30

Table 6. **Generic cover and substrate codes with preference values.**

GIS Cell-Based Habitat Modeling

GIS cell based modeling was used to compute WUA and UA for all species and lifestages. The River2D hydrodynamic model was used to simulate continuous cell-based surfaces of depth and velocity for a range of streamflows from 20 to 1500 cfs. One River2D output file (CDG file) was produced for each flow for a total of 40 raw output files in standard ASCII text format. Ten cfs increments were modeled for flows from 20-100 cfs, 20 cfs increments were modeled for flows from 100-200 cfs, and 50 cfs increments were modeled for flows from 200-1500 cfs. ArcGIS was used to process the River2D modeled output files as well as rasters of substrate size and cover type to quantify fish habitat for each species and lifestage. A raster or grid is much like a checker board that has equal sized cells arranged in rows and columns [\(Figure 15\)](#page-35-1). A cell size of one meter by one meter was used for this analysis. To process the many files required to quantify habitat, detailed scripts were written to automate the process.

Figure 15. Example of how a GIS cell-based model might look where each layer represents a different habitat variable (depth, velocity and substrate) used to calculate fish habitat.

The cell based modeling process can be viewed as having two steps. The first step is processing the depth and velocity data which is computed once for each flow [\(Figure 16\)](#page-35-2). The second step integrates the habitat preference criteria with the depth, velocity, substrate, and cover data to produce a combined suitability index (CSI) for each cell, species, life stage, and streamflow. The first step required creating a custom script to import the River2D output into an ArcGIS point shapefile which is the basic format used by ArcGIS. For each point in the shapefile there is a database record that contains the depth and velocity values at that location. A TIN (triangulated irregular network) was created for depth and another for velocity, so that a continuous surface can be created for each habitat metric. This facilitates interpolation between neighboring points. Next, each TIN was converted to a raster so it could be combined with the substrate and cover rasters during the second step of the habitat modeling process.

Figure 16. Flow diagram depicting the first step in converting the raw River2d output files.

The second step of the process involves using referenced depth and velocity preference files to convert each raster value into the corresponding preference for depth and velocity for each species and life stage [\(Figure 17\)](#page-36-0). The preference rasters resulted in values from 0 to 1 with 1 being the most preferred. The same was done for substrate and cover data resulting in a value between 0 and 1. Substrate was used for spawning life stages and cover was used for rearing life stages. In locations where cover was absent, substrate characteristics were substituted. After the preference rasters were created for depth, velocity, substrate, and/or cover, all three rasters were multiplied together to create a combined suitability index (CSI) raster with values ranging from 0 to 1 with 1 being the most suitable habitat. The CSI grids were created for each species and life stage for each flow. After all the grids were created, another script was written to summarize the results for each species, life stage, and streamflow. The summary statistics include total weighted usable area (WUA) and high quality UA which was defined as a CSI value $> = 0.6$. Unless otherwise noted in this report, UA stands for high quality usable area. For the habitat assessment, estimates of both WUA and UA were successfully produced. UA is valuable index as it can distinguish habitat distribution from WUA, which can equate large areas of low quality habitat with small areas of high quality habitat (Beecher et al. 2002).

Figure 17. Flow diagram depicting the second step of the process which involves using depth and velocity preference files to convert each raster value into the corresponding preference for depth and velocity for each species and life stage.

Channel Maintenance Flows

Channel maintenance flows are comprised of higher streamflows that generally occur at a lower frequency in a natural, unaltered hydrograph, but are important for maintaining the geomorphology and physical channel structure and form which supports the ecological function of the stream network. These lower frequency, higher magnitude flows maintain the basic physical characteristics that comprise physical habitat for the biological community. They provide functions important for stream habitat such as channel flushing, sediment transport, wood recruitment, and maintenance of riparian and floodplain habitat (Wald 2009). Instream flow recommendations for high flows should include high flow pulses and flushing flows for inchannel functions, channel maintenance flows for in-channel and riparian functions, and channel forming flows for side-channel and floodplain functions (Wald 2009).

Channel maintenance flows are typically derived using either of two basic methods. Analysis of empirical streamflow data from gaging stations can provide statistics such as mean annual discharge and streamflow frequency, duration, and recurrence interval. These statistics have been used in a number of different methodologies for developing channel maintenance flows (e.g. Tennant 1975, Wesche and Rechard 1980, Orsborn 1982, Rosgen 1982). The second basic method is based on the relationship between hydraulic forces and the physical characteristics of the stream channel and existing substrate, or sediment. It consists of determining the force (velocities and streamflow) required to mobilize and entrain various sediment sizes.

For streams in the State of Washington, Wald (2009) recommends three different levels of streamflow for maintaining channel function and floodplain processes, creating and maintaining physical habitat, and facilitating fish migration and flushing fines from the stream channel for maintenance of spawning and rearing habitat. Wald's recommendations include the following specific guidance (Wald 2009):

Flushing flows

Flushing flows to improve gravel quality for spawning and incubation habitat provide the greatest benefit when they occur at the beginning of spawning seasons. Flushing flows in the fall remove organic matter and fines that accumulate during the summer. Flushing flows in the spring provide migration flows while they reduce the amount of fines in spawning gravels. Wald (2009) recommends preserving or providing the mean annual discharge as a flushing flow for 6 to 12 hours duration during specified seasons and at intervals of at least 2 per year if not provided naturally.

Channel maintenance flows

Channel maintenance flows for activating geomorphic processes are greater in magnitude and duration than flows necessary for initiation of bedload movement. Wald (2009) recommends preserving or providing the 2-year frequency peak flow or 200% of mean annual discharge for at least 24 hours duration at specified seasons as a channel maintenance flow at intervals of 2 years if not provided naturally. Release rates should be controlled according to specified ramping rates (Hunter 1992).

Channel forming flows

The author recommends preserving or providing the 10-year frequency peak flow for at least 24 hours duration at specified seasons as a channel forming flow at intervals of 10 years if not provided naturally. Release rates should be controlled according to specified ramping rates (Hunter 1992).

For the assessment of channel maintenance flows, Wald's (2009) guidelines were used for developing the Icicle Creek historical channel: channel flushing, channel maintenance and channel forming flows in the historical channel. *Flushing, channel maintenance, and channel forming flow* recommendations were developed from analysis of the hydrograph at the USGS Gage #12458000, Icicle Creek above Snow Creek near Leavenworth, Washington. This included an assessment accounting for the portion of streamflows that are diverted away from Icicle Creek proper at upstream locations, hence the difference.

Habitat Assessment Results

Physical Parameters

Substrate and Cover

We used point substrate and cover data collected in the field to model continuous raster surfaces for the study site [\(Figure 18\)](#page-39-3) and [\(Figure 19\)](#page-39-4). Point vector data were converted in the GIS to grids (rasters) with a 1.0 m cell size. We used a Euclidean allocation process to assign values to the cells. We collected a total of 4,988 data points for Icicle Creek. Points were collected along shorelines up to bankfull where accessible and in-river. At every point, dominant substrate, subdominant substrate, percent fines, and cover data were recorded.

Figure 18. Results of the Euclidean allocation process (interpolation) for substrate.

Figure 19. Results of the Euclidean allocation process (interpolation) for cover.

As shown in [Table 7](#page-40-1) and [Table 8](#page-40-2) the dominant substrates were primarily the smaller size classes (fines to small gravels). The largest portion of cover was open water with no cover, followed by short grass that was very common along shorelines and on islands. Combined, fines including Sand, Silt/Clay/Organic comprised 70.4 % by area of the mapped within the stream channel. Substrates comprised of gravel and cobble accounted for 26.2 percent by area of the mapped stream channel.

Code	Class	Area (Sq. Meters)	Percent		
	Silt/Clay/Organic	15,106	31.9%		
$\overline{2}$	Sand	18,236	38.5%		
3	Small Gravel	5,309	11.2%		
$\overline{4}$	Medium Gravel	6.6%			
5	Large Gravel	1,836	3.9%		
6	Small Cobble	692	1.5%		
7	Large Cobble	1,449	3.1%		
8	Boulder	1,170	2.5%		
9	Bedrock	453	1.0%		
	Total:	47.379			

Table 7. Dominant substrate distribution by total area

Table 8. Cover distribution by total area.

Weighted Usable Area (WUA) and high quality Usable Area (UA) Estimates

With the use of GIS cell based modeling, estimates of spawning Weighted Usable Area (WUA) and high quality (UA) were made for eight species/lifestages of fish for the Icicle Creek historical channel These estimates are based on the individual cell CSI values which can mapped in a GIS to determine the special locations of all habitat values. A graphical example of what the actual calculated habitat looks like is depicted for both spawning and rearing steelhead habitats at 20, 300 and 1000 cfs in [Figure 20](#page-41-0) and [Figure](#page-42-0) 21, respectively.

Figure 20. Plotted steelhead spawning habitat individual cell CSI values for 20, 300 and 1,000 cfs.

Figure 21. Plotted juvenile steelhead rearing habitat, individual cell CSI values for 20, 300 and 1,000 cfs.

Both WUA and UA calculated for the Icicle Creek historical channel incorporates the hydraulic variables of depth, velocity, substrate and cover with the specific habitat needs of species [\(Figure](#page-43-0) [22](#page-43-0) and [Figure 23\)](#page-44-0). Adult bull have not been documented spawning in the historical channel (Jim Craig, personal communication) and generally spawn at higher elevations and are thus not included in the assessment. With the exception of juvenile lamprey for which no criteria were available, estimates of rearing WUA were made for eight species for the relevant life stages [\(Figure 23\)](#page-44-0) using 1,800 GIS generated grids. The WUA output from the GIS is expressed as flow per 1,000 feet (305 m) of lineal stream, for each species and life stage of concern and is an index of available habitat.

Figure 22. Estimates of spawning WUA for 8 species of fish in the Icicle Creek historical channel. Chinook criteria for both the spring and summer races are identical.

Figure 23. Estimates of rearing WUA for 8 species of fish in the Icicle Creek historical channel. Chinook criteria for both the spring and summer races are identical.

[Table 11](#page-46-0) and [Table 12](#page-47-0) depict Icicle Creek historical channel flow vs. spawning WUA per 1,000 feet (305 m) of lineal stream channel and by percent of peak for each specific species, respectively. The blue bars are for visual reference relating flow to the amount of WUA. Likewise green bars in [Table 13](#page-48-0) and [Table 16](#page-51-0) relate flow to the amount UA. [Table 14](#page-49-0) and [Table 15](#page-50-0) depict the Icicle Creek historical channel flow vs. rearing WUA tables for WUA per 1,000 feet (305 m) of lineal stream channel and by percent of peak species specific habitat, respectively. In the WUA and UA tables, two columns of flow are listed including the modeled flow running through Structure 2 into the Icicle Creek historical channel and the corresponding USGS flow. This is presented for management purposed but should be viewed with some caution. For example, if the streamflow out Snow and Nada Lake is actively managed (opened) or if Structure 2 is closed the relationship may change. The relationship is based on data collected between 10/05/2010 - 11/30/2012 at Structure 2 and the USGS gage 12458000. Three separate relationships were required and are presented in [Table 9](#page-45-0) and the results are presented in [Table 10.](#page-45-1)

Table 9. Data Range, R Squared and equation for the relationship between USGS Gage 12458000 and Structure 2.

Range	USGS Range	r^2	Equation
	88-950	[r=0.944169]	y = 0.26799 * (x + 12.8967)^ 1.174500
2	950-3600		$[r=0.976185]$ $y = 32.4955 * (x - 231.914)^{0} 0.497566$
	3600-5810	[r=0.947703]	$y = 1623.88 * (x - 3220.6)^0 0.0265487$

Regression line x: USGS 1245800/ Icicle Cr. above Snow Cr nr Leavenworth WA

Regression line y: historical channel at Structure 2

Table 10 Relationship between streamflow at the USGS Gage 12458000 at RM 5.8 and Icicle Creek historical channel Structure 2 (S2) at RM 3.8. Both tables below use the same relationship. Data for the relationship collected: 10/05/2010 - 11/30/2012. R^2 for flows (88-959 cfs) = 0.944 and from (950-3600 cfs) = 0.976.

Table 11. Modeled spawning adult WUA per 1,000 feet (305 m) of lineal stream channel. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

	Flows (cfs)	Adult Spawning									
USGS	Structure 2	Coho	Chinook	Steelhead	Rainbow	Cutthroat	Whitefish	Lamprey	Sucker		
na	20	56%	17%	13%	49%	95%	4%	90%	49%		
na	30	73%	32%	25%	70%	100%	I 7%	99%	70%		
na	40	84%	47%	38%	83%	95%	\blacksquare 10%	100%	83%		
na	50	91%	60%	50%	91%	89%	13%	99%	91%		
88	60	96%	71%	61%	96%	83%	ı 16%	96%	96%		
101	70	98%	79%	70%	98%	79%	19%	93%	98%		
115	80	100%	87%	78%	99%	76%	23%	91%	99%		
129	90	100%	92%	84%	100%	73%	26%	88%	100%		
142	100	100%	96%	89%	99%	70%	29%	85%	99%		
168	120	98%	100%	95%	95%	65%	B6%	79%	95%		
193	140	94%	100%	99%	92%	60%	42%	74%	92%		
218	160	90%	100%	100%	90%	56%	49%	68%	90%		
243	180	86%	99%	99%	86%	54%	55%	64%	86%		
266	200	82%	98%	97%	83%	51%	62%	59%	83%		
325	250	76%	94%	88%	76%	44%	75%	50%	76%		
382	300	70%	91%	77%	71%	36%	85%	42%	71%		
437	350	66%	87%	66%	66%	29%	92%	35%	66%		
491	400	62%	82%	59%	60%	25%	97%	31%	60%		
544	450	59%	79%	53%	54%	20%	99%	27%	54%		
596	500	56%	75%	50%	48%	17%	100%	23%	48%		
648	550	53%	72%	47%	42%	15%	99%	20%	42%		
699	600	50%	68%	44%	36%	13%	98%	17%	36%		
749	650	47%	64%	41%	32%	I 11%	96%	15%	32%		
799	700	45%	59%	39%	28%	I 10%	94%	13%	28%		
848	750	43%	54%	36%	24%	Ш 9%	92%	11%	24%		
896	800	41%	49%	33%	21%	8%	90%	\blacksquare 10%	21%		
944	850	39%	45%	30%	19%	I 7%	88%	Π 9%	19%		
1,024	900	37%	41%	27%	ı 16%	6%	85%	L 8%	16%		
1,115	950	35%	38%	25%	П 15%	5%	83%	I 7%	п 15%		
1,215	1,000	34%	36%	23%	H 13%	5%	81%	I 6%	П 13%		
1,315	1,050	32%	34%	21%	П 12%	4%	79%	5%	I 12%		
1,425	1,100	31%	33%	20%	\Box 11%	4%	77%	5%	Π 11%		
1,525	1,150	29%	32%	19%	I 10%	4%	74%	4%	I 10%		
1,640	1,200	28%	31%	18%	\blacksquare 10%	4%	72%	4%	I 10%		
1,760	1,250	27%	29%	٠ 16%	\blacksquare 9%	3%	70%	4%	I 9%		
1,890	1,300	26%	29%	U 15%	\Box 9%	3%	68%	4%	П 9%		
2,020	1,350	24%	27%	14%	\Box 8%	3%	65%	3%	\Box 8%		
2,150	1,400	23%	26%	I 13%	7%	3%	62%	3%	I 7%		
2,300	1,450	22%	26%	ı 12%	I 7%	2%	60%	2%	I 7%		
2,440	1,500	21%	25%	٠ 12%	7%	2%	57%	2%	7%		

Table 12. Modeled spawning adult WUA as percent of peak. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

Table 13 Modeled spawning adult UA for the Icicle Creek historical channel modeled. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

Table 14. Modeled rearing WUA per 1,000 feet (305 m) of lineal stream channel. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

	Flows (cfs)	Chinook	Steelhead	Rainbow	Cutthroat	Whitefish	Whitefish	Sucker	Bull Trout
USGS	Structure 2	Juvenile	Juvenile	Winter Rearing	Juvenile	Juvenile	Adult	Adult & Juv.	Adult & Juv.
na	20	43%	30%	30%	49%	1%	3%	82%	48%
na	30	58%	39%	B9%	63%	3%	4%	95%	62%
na	40	70%	48%	48%	72%	4%	6%	99%	71%
na	50	78%	55%	55%	79%	6%	П 8%	100%	76%
88	60	84%	61%	61%	85%	8%	Π 10%	99%	81%
101	70	89%	67%	67%	89%	П 10%	Ш 12%	97%	85%
115	80	92%	72%	72%	92%	٠ 13%	٠ 14%	95%	88%
129	90	95%	76%	76%	95%	Ū 15%	П 16%	92%	91%
142	100	97%	79%	79%	97%	18%	19%	90%	93%
168	120	99%	85%	85%	99%	24%	23%	84%	96%
193	140	100%	90%	90%	100%	29%	28%	78%	99%
218	160	99%	93%	93%	100%	35%	33%	72%	100%
243	180	98%	96%	96%	99%	40%	37%	68%	100%
266	200	96%	98%	98%	97%	45%	42%	64%	100%
325	250	91%	100%	100%	93%	57%	52%	59%	97%
382	300	85%	99%	99%	87%	66%	60%	54%	93%
437	350	80%	97%	97%	81%	74%	67%	51%	89%
491	400	78%	97%	97%	79%	80%	73%	47%	87%
544	450	75%	96%	96%	75%	85%	77%	43%	83%
596	500	72%	94%	94%	71%	88%	81%	39%	79%
648	550	69%	92%	92%	68%	92%	85%	34%	76%
699	600	66%	89%	89%	64%	94%	88%	32%	72%
749	650	64%	88%	88%	61%	97%	90%	30%	70%
799	700	63%	87%	87%	60%	98%	93%	28%	69%
848	750	60%	84%	84%	56%	99%	94%	27%	65%
896	800	58%	82%	82%	54%	100%	96%	26%	62%
944	850	56%	81%	81%	52%	100%	97%	26%	60%
1,024	900	55%	79%	79%	51%	100%	98%	26%	59%
1,115	950	54%	79%	79%	50%	100%	99%	24%	58%
1,215	1,000	53%	78%	78%	49%	100%	100%	24%	57%
1,315	1,050	52%	76%	76%	47%	99%	100%	22%	55%
1,425	1,100	51%	75%	75%	46%	98%	100%	21%	54%
1,525	1,150	50%	74%	74%	45%	97%	100%	21%	54%
1,640	1,200	50%	73%	73%	45%	95%	99%	22%	54%
1,760	1,250	50%	72%	72%	44%	94%	99%	21%	53%
1,890	1,300	49%	71%	71%	44%	93%	98%	21%	53%
2,020	1,350	49%	70%	70%	44%	91%	98%	20%	53%
2,150	1,400	49%	69%	69%	44%	89%	97%	20%	53%
2,300	1,450	48%	69%	69%	44%	88%	96%	20%	52%
2,440	1,500	47%	68%	68%	43%	87%	96%	19%	52%

Table 15. Modeled rearing WUA as percent of peak. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

Table 16. Modeled rearing UA for the Icicle Creek historical channel. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

Integration of Species-Specific Habitat-Flow Relationships

To integrate species specific habitat flow relationships in order to accommodate the habitat needs for multiple target fish species and lifestages that may occur simultaneously in the Icicle Creek historical channel, tables depicting the spawners and habitat needs by month were produced (Appendix A). Additionally the 90 to 10% exceedance flows are highlighted. These are the flows that occur during each month that spawning occurs. With this data managers can easily see which species is spawning in what month, how much habitat is available throughout all flows and how much habitat is typically available for the specific month. The exact exceedance flows are listed at the bottom of each table. Both the Icicle Creek historical channel flows and the USGS flows are listed for reference as well. This assessment was not conducted for rearing stages since most of them are present all year. However, the tables of rearing habitat and flow data are still available in the report.

Channel Maintenance Flows

Using Wald's (2009) guidelines for developing the Icicle Creek historical channel: channel flushing, channel maintenance and channel forming flows, the results of the assessment are presented in [Table 17.](#page-52-2) *Flushing, channel maintenance, and channel forming flow* recommendations were developed from analysis of the hydrograph at the USGS Gage #12458000, Icicle Creek above Snow Creek near Leavenworth, Washington. This assessment inherently accounts for the average apportion of flows that are diverted away from the historical channel at upstream locations, hence the different between the two. See [Table 9](#page-45-0) and [Table 10](#page-45-1) for additional data regarding the relations between the USGS and Icicle Creek historical channel streamflows. From the relationship it appears that the maximum amount of water that can flow through Structure 2 is just over 2,000 cfs. As such the channel forming flow of 7,930 cfs listed in [Table 17](#page-52-2) cannot occur in the historical channel because most of the streamflow passes down the hatchery channel.

Table 17. Recommended Channel: Flushing, Maintenance and Forming flows (cfs) and the associated USGS gage flows that would facilitate the recommended flows in the historical channel.

Discussion

WUA is only an index and it should not be confused with the actual physical area within a stream (Payne 2003). It can only be an index because the estimated "area" is multiplied by the unit-less habitat suitability attributes. As well, the units of WUA are not standard or transferable because they are derived from a range of habitat suitability criteria and selectable combinations of variables (Mathur el al. 1985). The WUA graphs only show whether an increase or decrease in streamflow will increase or decrease the quantity of fish habitat. The study's predicted fish habitat versus streamflow results have to be interpreted by knowledgeable biologists and others to arrive at an instream flow regime that satisfies applicable laws and interests.

The hydrodynamic modeling was rather straightforward but the complexity of the site did required extra effort and attention to detail. An iterative process of running the River2D model, reviewing computational issues and then refining the bed and mesh files to provide higher computational resolution in problem areas was often employed and required considerably more time and attention to detail than a "typical" 2D modeling effort.

Model calibration and validation to water surface elevation was generally near the error of our survey grade GPS of 3.0 cm. We observed 3.3 and 3.6 cm of error at the low and medium calibration flows. Greater error was observed at the highest calibration flow of 1,220 cfs but we feel that layers of fines that were deposited between bathymetric data collection and calibration data collected are the primary cause. Most of the fines observed in the field were deposited at higher riverbank elevations [\(Figure 24\)](#page-58-0) and undoubtedly had an effect on the comparison of measured vs. observed water surface elevation. These fines were not present on our initial field visits. As such we chose not to adjust the roughness values to unrealistic measures. When we collected low flow depth and velocity observations along 4 cross sections in September of 2012 Structure 2 was closed and water was water was observed seeping from the hatchery channel to the lower elevation historical channel. The net result was an increase in stream flow due to the accretion of seepage. As a result, we used an average stream flow (42 cfs) for the low flow calibration assessment. In reality the flow was lower at the top of the historic channel and higher in the bottom section. This likely resulted in additional error. The lack of empirical depths and velocities due to entrained fines and moving bed in the ADCP files also limits the confidence in our high flow calibration but considering the deposition of fines we are satisfied with the results.

Overall we believe the model has done an excellent job and most of the "error" is due to the observed deposition fines between the collection of model bathymetry and the collection of model calibration data. As such, we are not concerned with the error levels in observed vs. simulated depths and velocities for which they were only collected at 4 cross sections for validation. Additional changes to the stream bathymetry were also observed on our final field trip. Some banks and sloughed in and even some small islands had completely disappeared. There is little doubt that Icicle Creek historical channel is a state of geomorphic flux.

Using GIS cell-based modeling and scripts to compute WUA and UA was also the best approach. With this technique we were able to incorporate precisely mapped field features including complex cover habitat running longitudinally up islands including undercut banks and overhanging vegetation which would have been difficult to capture with cross sections unless a very large number of cross sections were used. As well, we could easily produce other variants of usable habitat such as the high quality estimates of usable area (UA). High quality UA has

been shown to be strongly associated with the distribution of juvenile steelhead (Beecher et al. 1993, 1995). As such and for added management purposes we can also compare estimates of WUA and UA as percent of peak habitat [\(Table 18](#page-54-0) to [Table 21\)](#page-57-0). While the peaks are similar for the spawning estimates [\(Table 18](#page-54-0) and [Table 19\)](#page-55-0), larger differences exist between the rearing estimates [\(Table 20](#page-56-0) and [Table 21\)](#page-57-0). Since WUA often includes large amounts of low quality habitat that is rarely used, UA could be considered for implementation when significant differences are observed between WUA and UA.

Table 18. Spawning habitat results comparing predicted WUA and UA as a percent of peak for Coho, Chinook, Steelhead and Rainbow Trout. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

	Adult Spawning												
	Flows (cfs)	Coho			Chinook	Steelhead			Rainbow				
USGS	Structure 2	WUA	UA	WUA	UA	WUA	UA	WUA	UA				
na	20	56%	33%	17%	2%	13%	2%	49%	19%				
na	30	73%	57%	32%	10%	25%	12%	70%	46%				
na	40	84%	76%	47%	25%	38%	24%	83%	74%				
na	50	91%	90%	60%	35%	50%	38%	91%	79%				
88	60	96%	98%	71%	49%	61%	50%	96%	80%				
101	70	98%	100%	79%	66%	70%	61%	98%	86%				
115	80	100%	100%	87%	78%	78%	71%	99%	83%				
129	90	100%	98%	92%	94%	84%	80%	100%	89%				
142	100	100%	95%	96%	99%	89%	87%	99%	100%				
168	120	98%	89%	100%	100%	95%	96%	95%	100%				
193	140	94%	77%	100%	90%	99%	99%	92%	75%				
218	160	90%	67%	100%	80%	100%	100%	90%	81%				
243	180	86%	57%	99%	70%	99%	99%	86%	70%				
266	200	82%	49%	98%	67%	97%	93%	83%	57%				
325	250	76%	39%	94%	61%	88%	77%	76%	55%				
382	300	70%	35%	91%	50%	77%	57%	71%	48%				
437	350	66%	31%	87%	39%	66%	43%	66%	60%				
491	400	62%	29%	82%	35%	59%	36%	60%	48%				
544	450	59%	26%	79%	36%	53%	32%	54%	36%				
596	500	56%	24%	75%	43%	50%	28%	48%	26%				
648	550	53%	22%	72%	43%	47%	27%	42%	23%				
699	600	50%	21%	68%	41%	44%	26%	B6%	23%				
749	650	47%	18%	64%	37%	41%	24%	32%	15%				
799	700	45%	16%	59%	32%	39%	24%	28%	Π 12%				
848	750	43%	14%	54%	26%	B6%	23%	24%	П 10%				
896	800	41%	U 11%	49%	19%	33%	19%	21%	П 11%				
944	850	39%	U 9%	45%	13%	30%	17%	19%	L 13%				
1,024	900	B7%	I 6%	41%	I 8%	27%	ı 16%	16%	П 13%				
1,115	950	35%	5%	38%	5%	25%	ı 13%	15%	П 11%				
1,215	1,000	34%	4%	B6%	5%	23%	П 11%	13%	Π 9%				
1,315	1,050	32%	3%	34%	5%	21%	U 10%	L 12%	Π 7%				
1,425	1,100	31%	2%	33%	4%	20%	I 9%	U 11%	4%				
1,525	1,150	29%	2%	32%	3%	19%	I 8%	10%	1%				
1,640	1,200	28%	2%	31%	3%	18%	I 7%	10%	0%				
1,760	1,250	27%	1%	29%	2%	16%	5%	9%	0%				
1,890	1,300	26%	1%	29%	1%	ı 15%	4%	9%	0%				
2,020	1,350	24%	1%	27%	1%	U 14%	2%	8%	1%				
2,150	1,400	23%	1%	26%	1%	ı 13%	2%	7%	0%				
2,300	1,450	22%	1%	26%	2%	Ш 12%	2%	7%	0%				
2,440	1,500	21%	1%	25%	2%	12%	2%	7%	0%				

Table 19. Spawning habitat results comparing predicted WUA and UA as a percent of peak for Cutthroat, Whitefish, Lamprey and Sucker. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

Table 20. Rearing habitat results comparing predicted WUA and UA as a percent of peak for Chinook Steelhead Rainbow and Cutthroat. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

	Flows (cfs) Whitefish Juvenile			Whitefish Adult	Sucker Adult & Juv.		Bull Trout Adult & Juv.		
USGS	Structure 2	WUA	UA	WUA	UA	WUA	UA	WUA	UA
na	20	1%	0%	3%	0%	82%	23%	48%	26%
na	30	3%	0%	4%	0%	95%	43%	62%	47%
na	40	4%	0%	6%	0%	99%	58%	71%	64%
na	50	6%	0%	8%	0%	100%	66%	76%	67%
88	60	8%	0%	10%	0%	99%	77%	81%	73%
101	70	10%	0%	12%	0%	97%	85%	85%	78%
115	80	13%	0%	14%	0%	95%	92%	88%	80%
129	90	15%	1%	16%	1%	92%	96%	91%	85%
142	100	18%	2%	19%	1%	90%	97%	93%	85%
168	120	24%	4%	23%	3%	84%	98%	96%	88%
193	140	29%	7%	28%	4%	78%	100%	99%	92%
218	160	35%	9%	33%	6%	72%	98%	100%	99%
243	180	40%	12%	37%	9%	68%	96%	100%	100%
266	200	45%	16%	42%	12%	64%	91%	100%	95%
325	250	57%	27%	52%	19%	59%	81%	97%	94%
382	300	66%	40%	60%	25%	54%	69%	93%	84%
437	350	74%	50%	67%	32%	51%	64%	89%	82%
491	400	80%	56%	73%	39%	47%	57%	87%	95%
544	450	85%	63%	77%	45%	43%	48%	83%	90%
596	500	88%	70%	81%	51%	39%	41%	79%	79%
648	550	92%	80%	85%	59%	34%	34%	76%	68%
699	600	94%	86%	88%	65%	32%	30%	72%	62%
749	650	97%	93%	90%	71%	30%	30%	70%	61%
799	700	98%	96%	93%	74%	28%	26%	69%	59%
848	750	99%	98%	94%	78%	27%	26%	65%	56%
896	800	100%	99%	96%	81%	26%	25%	62%	50%
944	850	100%	100%	97%	85%	26%	24%	60%	53%
1,024	900	100%	99%	98%	87%	26%	24%	59%	54%
1,115	950	100%	98%	99%	90%	24%	18%	58%	51%
1,215	1,000	100%	96%	100%	93%	24%	17%	57%	51%
1,315	1,050	99%	98%	100%	94%	22%	16%	55%	48%
1,425	1,100	98%	99%	100%	95%	21%	\Box 12%	54%	44%
1,525	1,150	97%	99%	100%	96%	21%	I 11%	54%	39%
1,640	1,200	95%	96%	99%	96%	22%	11%	54%	44%
1,760	1,250	94%	96%	99%	98%	21%	U 11%	53%	46%
1,890	1,300	93%	92%	98%	99%	21%	\blacksquare 10%	53%	51%
2,020	1,350	91%	90%	98%	99%	20%	I 10%	53%	51%
2,150	1,400	89%	88%	97%	99%	20%	10%	53%	52%
2,300	1,450	88%	88%	96%	100%	20%	П 9%	52%	52%
2,440	1,500	87%	85%	96%	100%	19%	10%	52%	49%

Table 21. Rearing habitat results comparing predicted WUA and UA as a percent of peak for Whitefish, Sucker and Bulltrout. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

Perhaps the biggest surprise was the apparent reduction in available spawning habitat based on the large amounts of new fines deposited and mapped throughout the study site [\(Figure 24\)](#page-58-0). Staff observed and documented large deposits of fine and coarse sand between the start of the field effort in October of 2011 and the end of field work in September 2012. It is likely to conclude that the amount of predicted WUA for spawning will increase as these fines decrease. However, the overall proportion of habitat may remain similar across streamflows. Depositional events from upstream mass wasting events have likely occurred in the basin since time immemorial. The frequency and proportion of which that have historical deposited in the historical channel is not known.

Figure 24. Coarse and fine sand deposits in the Icicle Creek historical channel.

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Appendix A

Appendix A1 – WUA Percent of Peak, for April Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

April Exceedance flows at Structure 2 (90-10%) 270 to 873 cfs and are highlighted in the shaded area.

Appendix A2 – WUA Percent of Peak, for May Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

May exceedance flows at Structure 2 (90-10%) 959 to 1490 cfs and are highlighted in the shaded area.

Appendix A3 – WUA Percent of Peak, for June Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

June Exceedance flows at Structure 2 (90-10%) 994 to 1610 cfs and are highlighted in the shaded area.

Appendix A4 – WUA Percent of Peak, for July Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

July Exceedance flows at Structure 2 (90-10%) 288 to 1118 cfs and are highlighted in the shaded area.

Appendix A5 – WUA Percent of Peak, for August Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

August Exceedance flows at Structure 2 (90-10%) 106 to 346 cfs and are highlighted in the shaded area.

Appendix A6 – WUA Percent of Peak, for September Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

September Exceedance flows at Structure 2 (90-10%) 68 to 177 cfs and are highlighted in the shaded area.

Appendix A7 – WUA Percent of Peak, for October Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

October Exceedance flows at Structure 2 (90-10%) 59 to 348 cfs and are highlighted in the shaded area.

Appendix A8 – WUA Percent of Peak, for November and December Spawners. The Stucture 2 flow is the actual flow modeled for the Icicle Creek historical channel.

November Exceedance flows at Structure 2 (90-10%) 98 to 768 cfs and are highlighted in the shaded area.

December Exceedance flows at Structure 2 (90-10%) 103 to 517 cfs and are highlighted in the shaded area.

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November 2013